REFERENCE DRAWINGS

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Appendix B

Reference Drawings

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ALBERTA WEAK POST AND STRONG POST W-BEAM GUARDRAIL

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Alberta Weak Post and Strong Post W-Beam Guardrail

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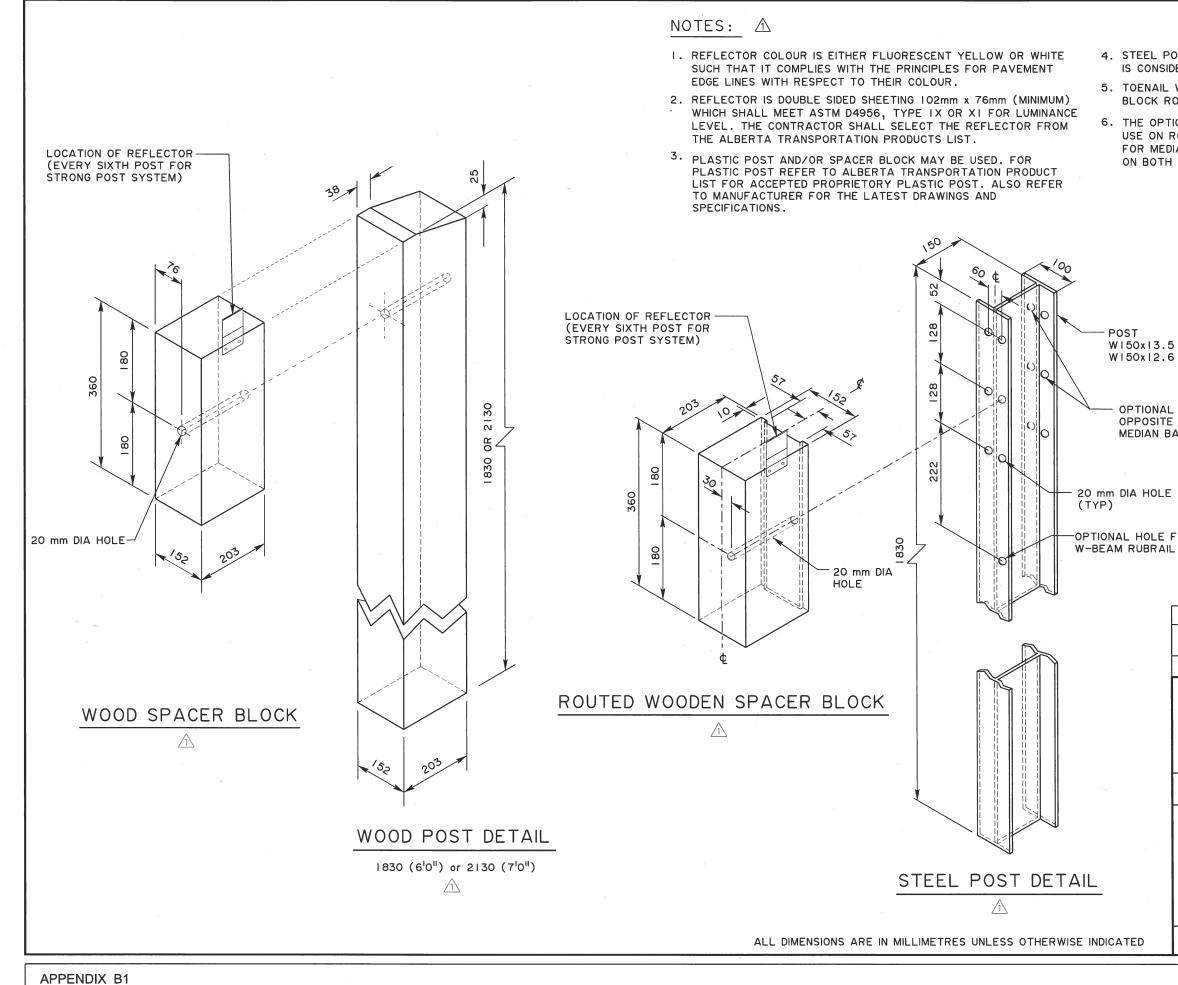
Appendix B1

Alberta Weak Post and Strong Post W-Beam Guardrail

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4. STEEL POST WI50x12.6 IS AN ACCEPTABLE ALTERNATIVE THAT IS CONSIDERED EQUIVALENT TO THE WI50x13.5

5. TOENAIL WITH ONE 89 mm LONG GALVANIZED NAIL TO PREVENT BLOCK ROTATION OF WOODEN SPACER BLOCK.

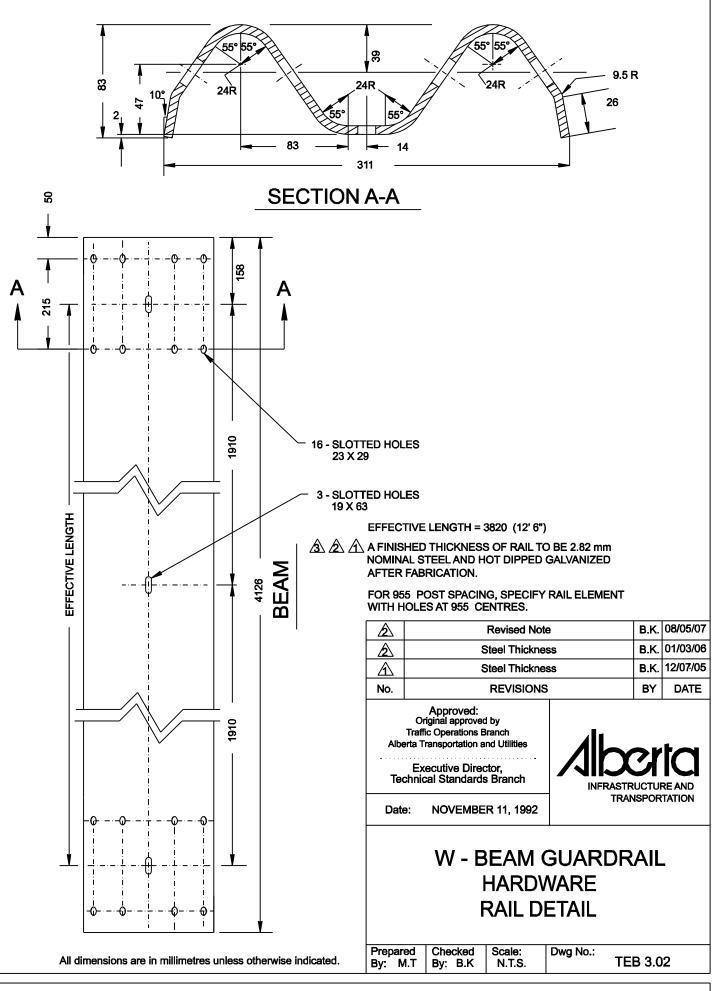
6. THE OPTIONAL HOLES IN THE FLANGE ARE ACCEPTABLE FOR USE ON ROADSIDE AND MEDIAN APPLICATIONS BUT REQUIRED FOR MEDIAN APPLICATIONS WHERE RAILS ARE INSTALLED ON BOTH SIDES.

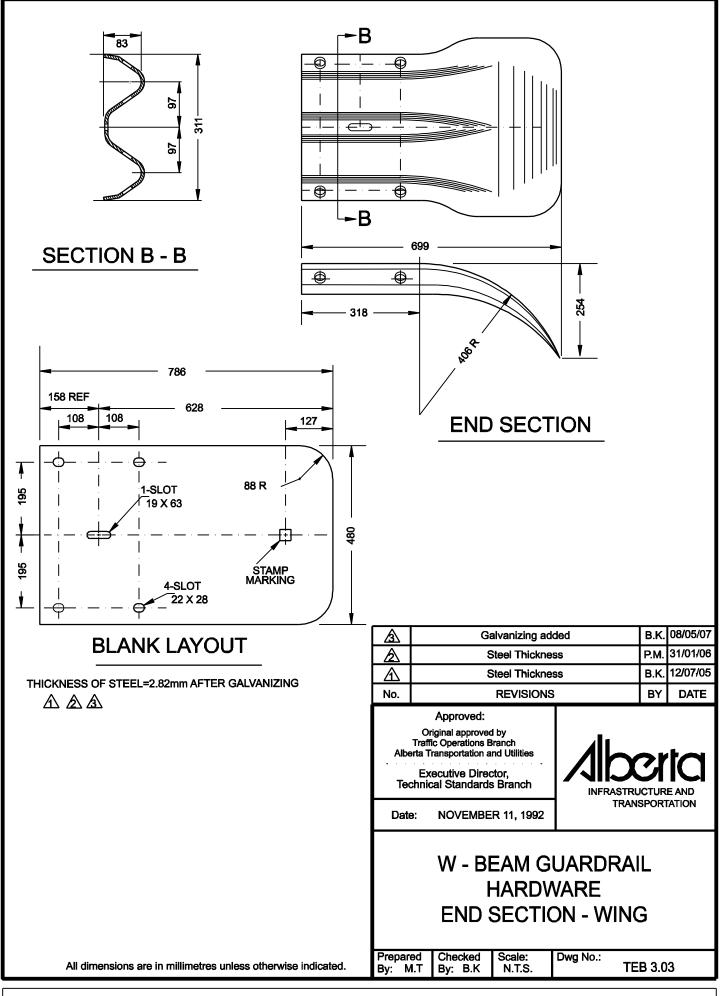
W150x13.5 OR W150x12.6

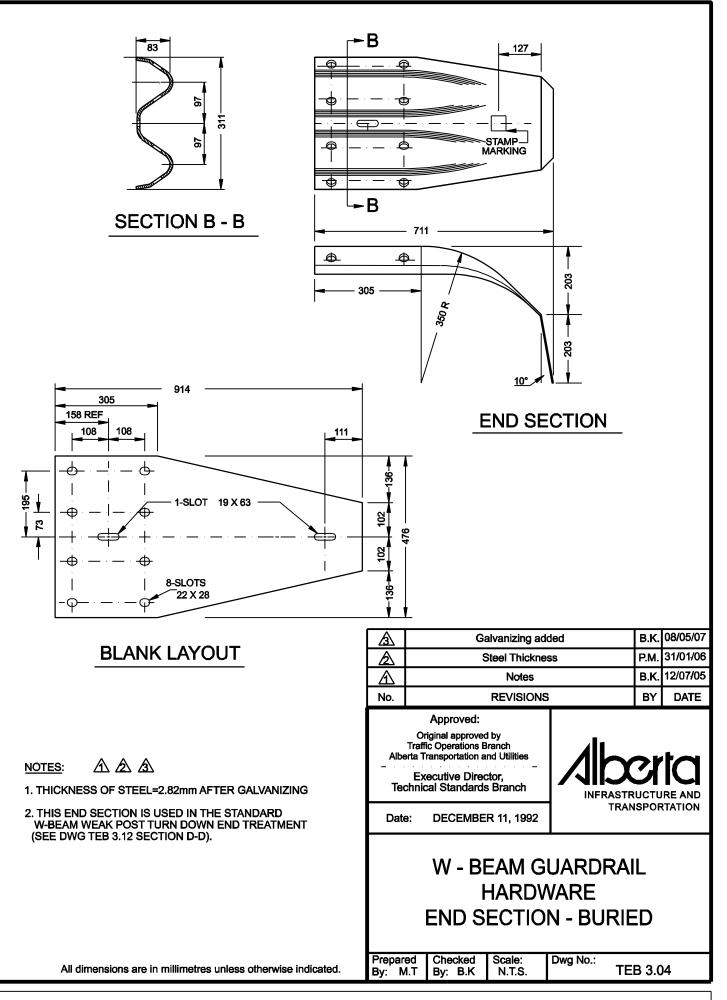
> OPTIONAL HOLES IN OPPOSITE FLANGE FOR MEDIAN BARRIER APPLICATIONS

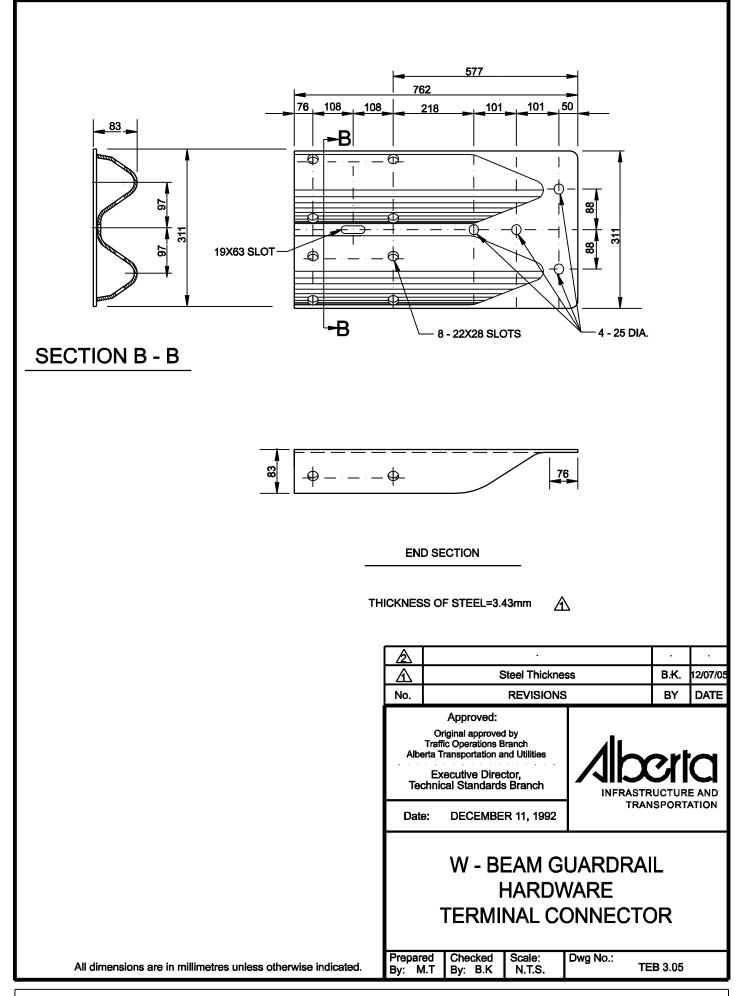
OPTIONAL HOLE FOR

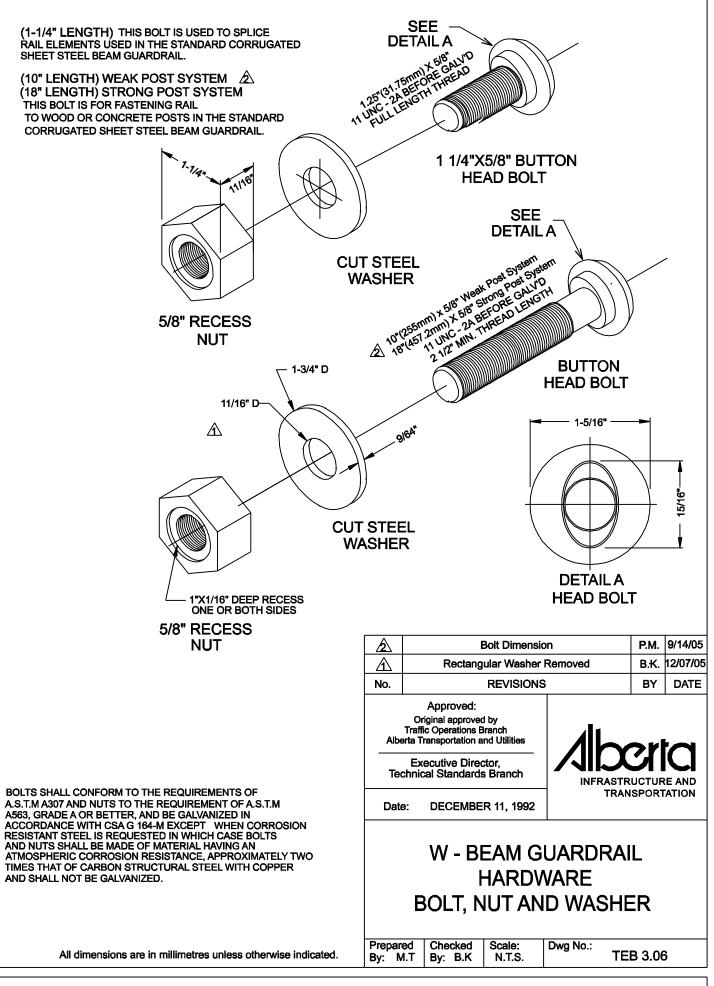
					2	
	REDRAWN FROM TEB 3.01, JULY 2009, REVISION 3. BOLT HOLE DIMENSION AND NOTE 2 REVISED. STEEL DETAIL AND NOTES 3 - 6 ADDED			PM	11 MAR 2016	
No.			REVISIONS		BY	DATE
	Approved:					
Gr	Dullianson Executive Director, Technical Standards Branch					
	Date: MARCH 2016		Transportation			
STRONG POST W - BEAM GUARDRAIL HARDWARE WOOD SPACER BLOCK AND POST						
Prepar By:GE		Checked By: PM	Scale: N.T.S.	-	TEB	3.01
					H-A	PP-B1-1

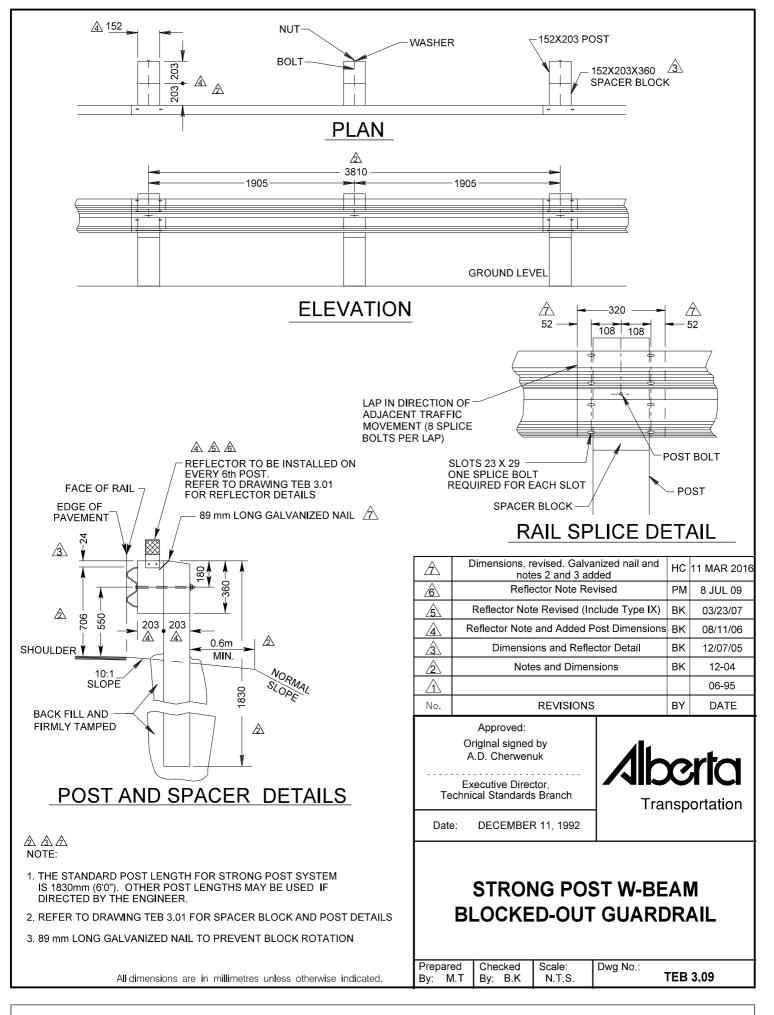


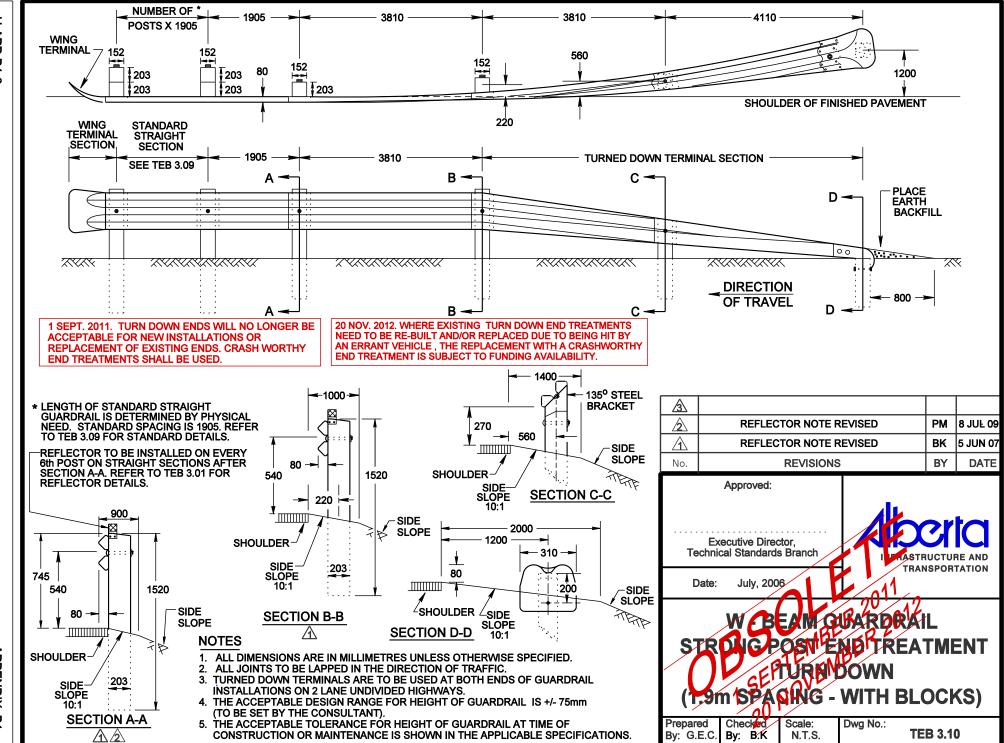




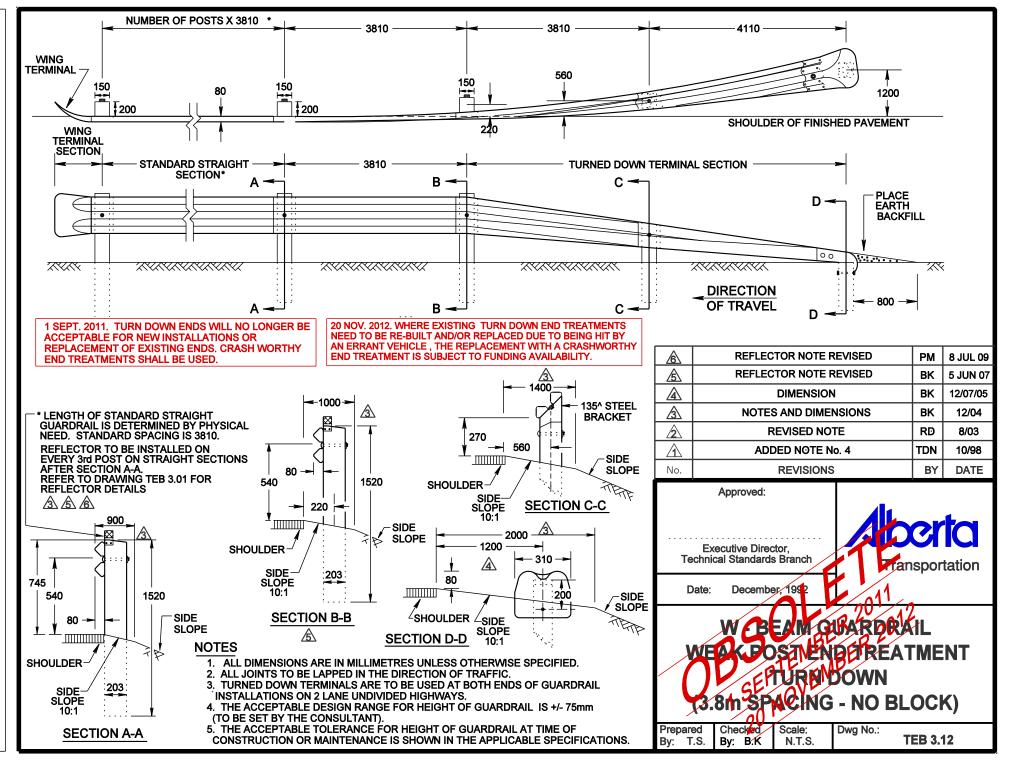


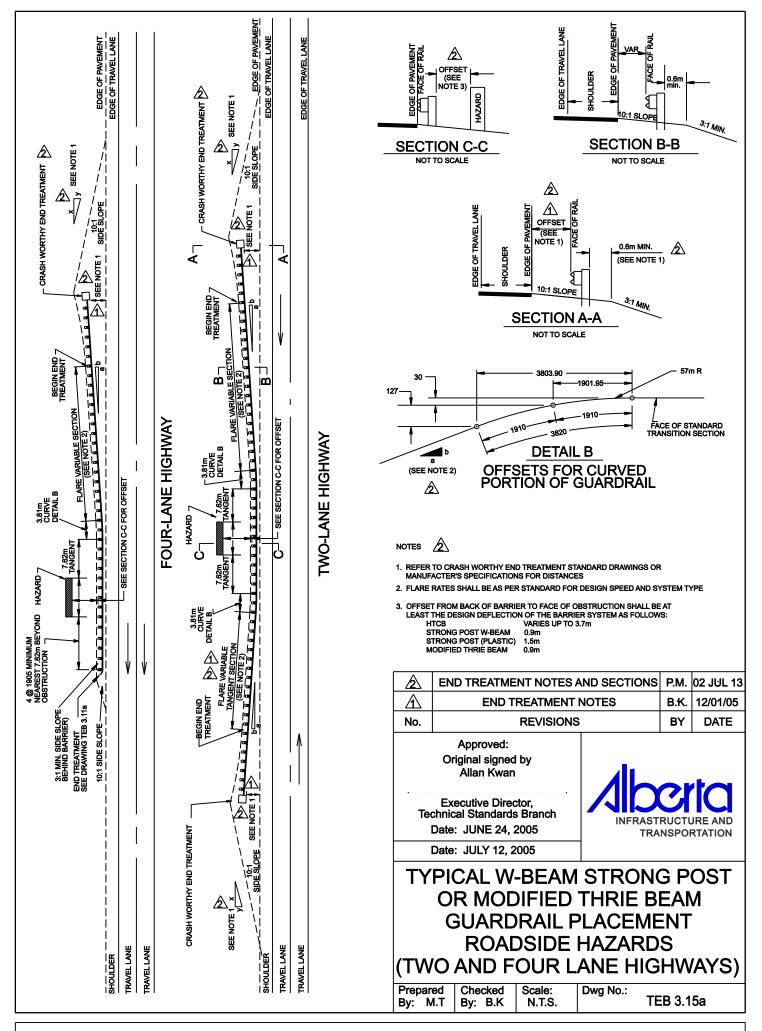


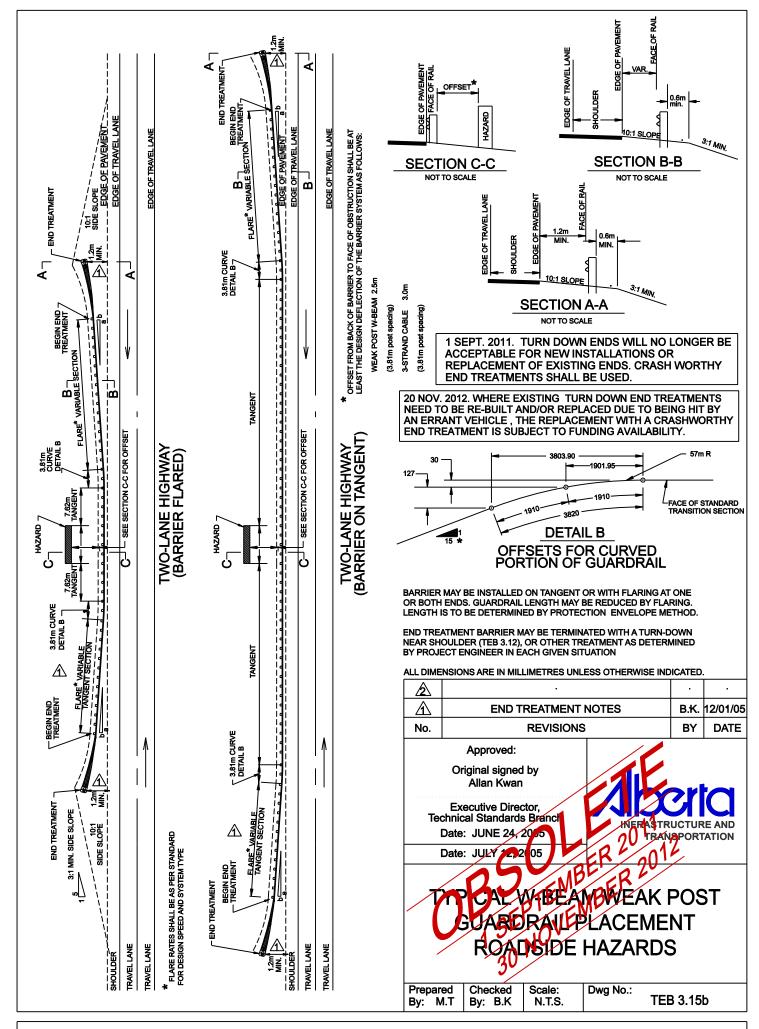




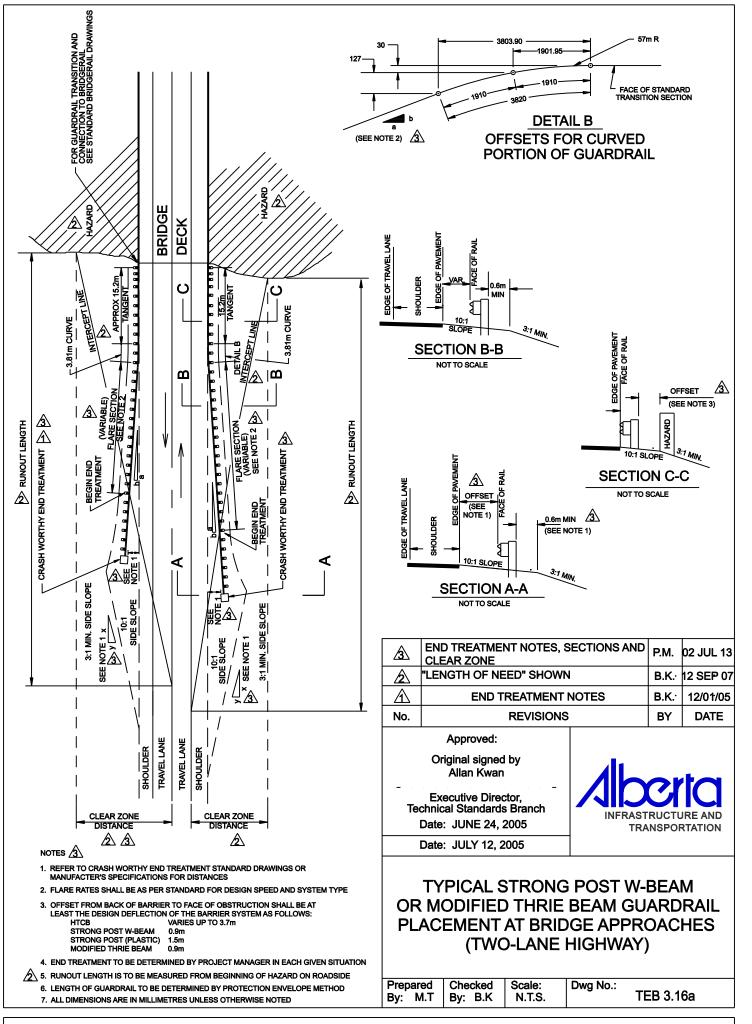


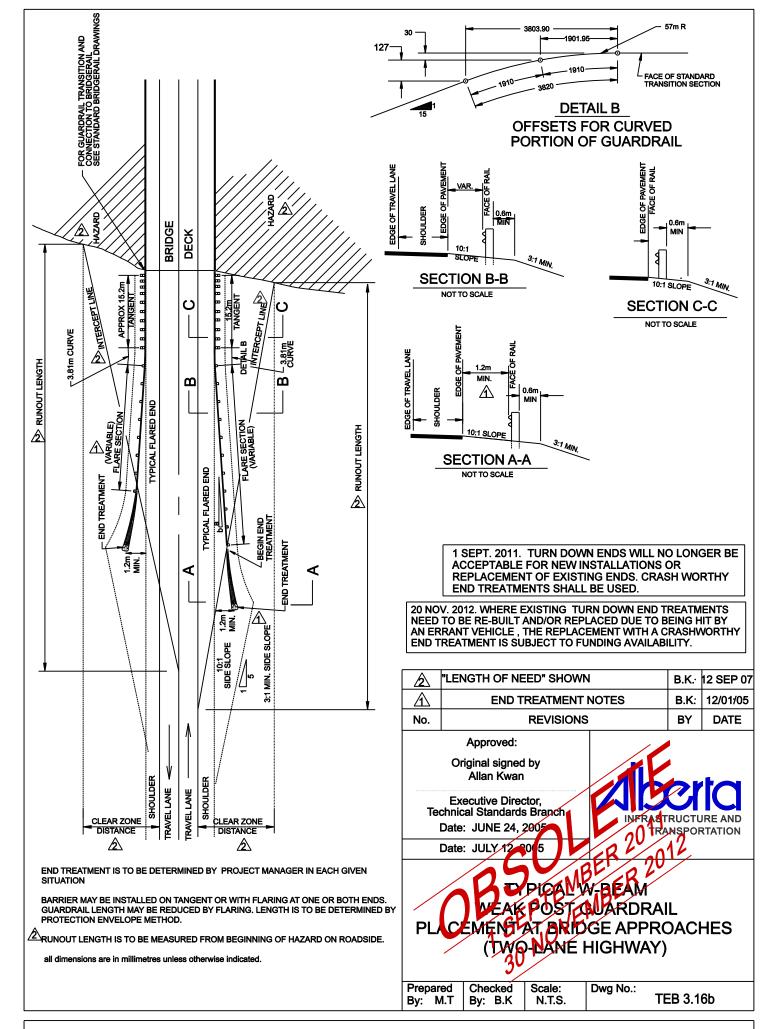




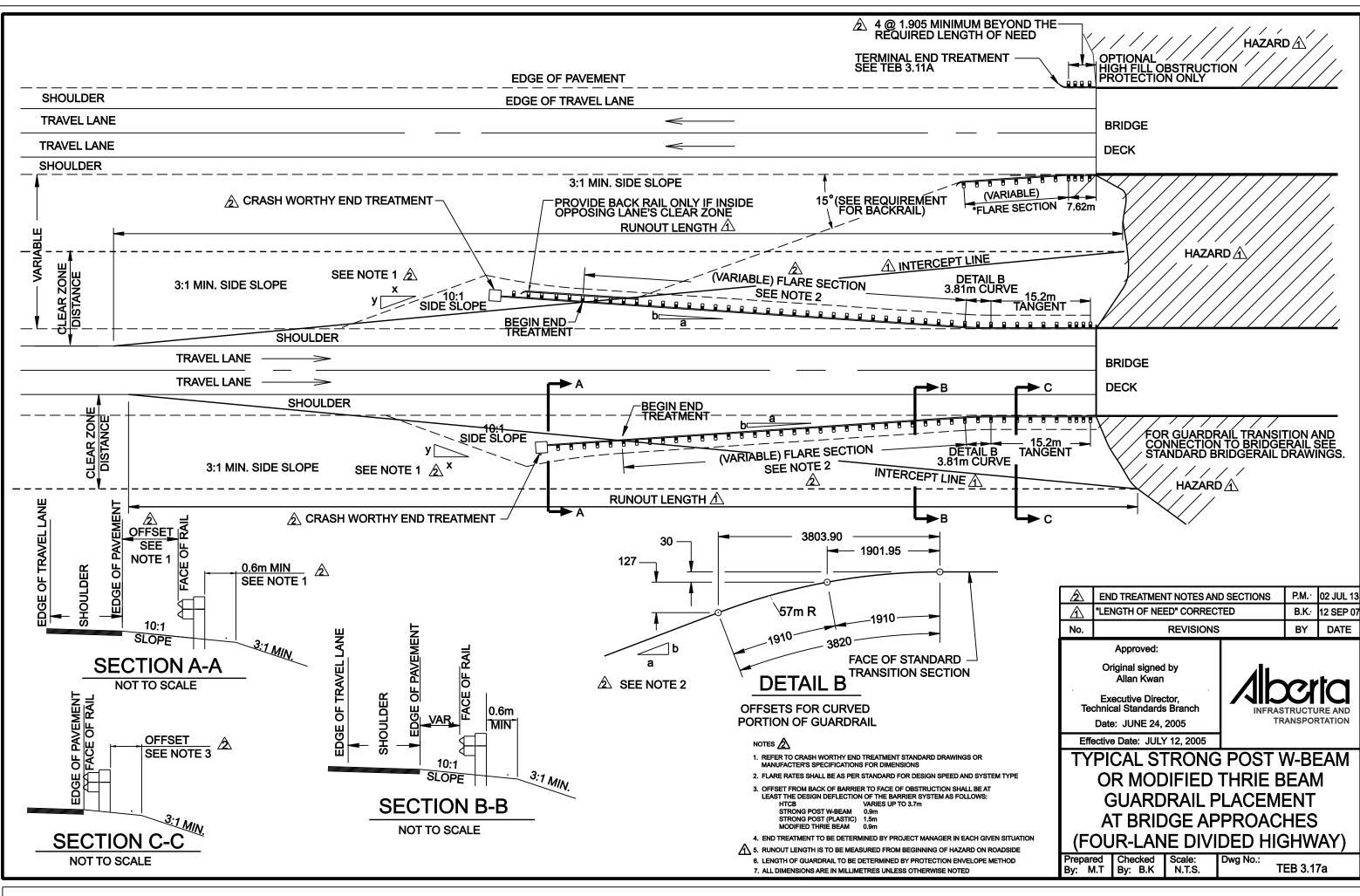


APPENDIX B1 ...\Revisions 2013\teb3 015b.dgn 6/27/2013 1:26:30 PM





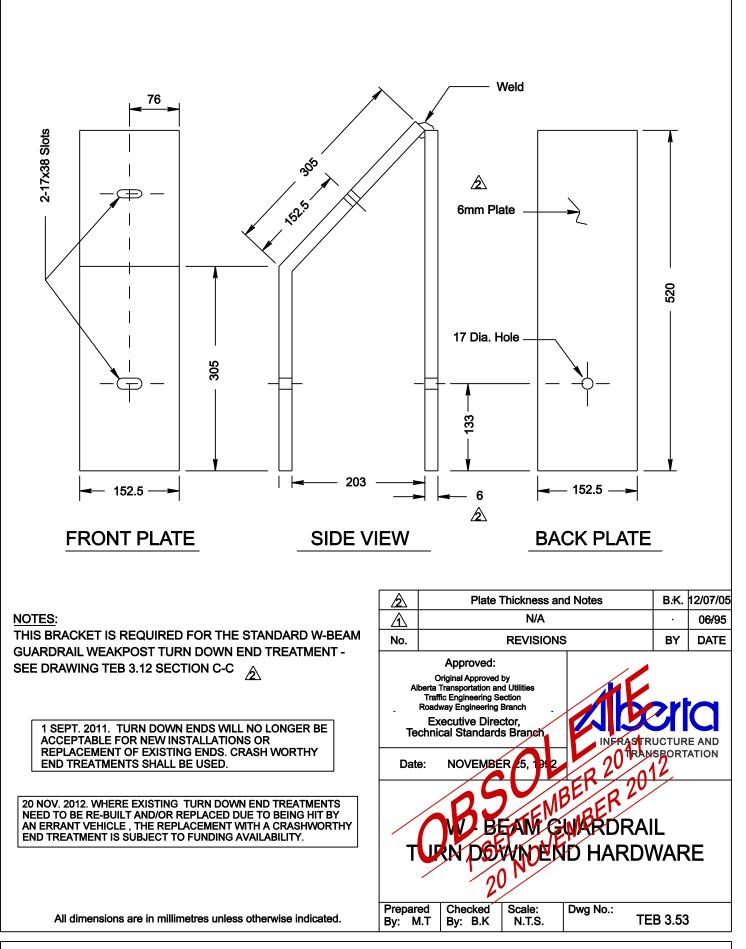
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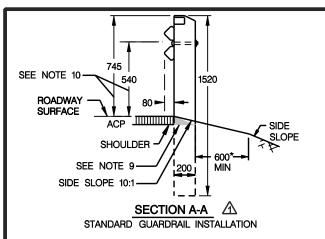


APPENDIX B1

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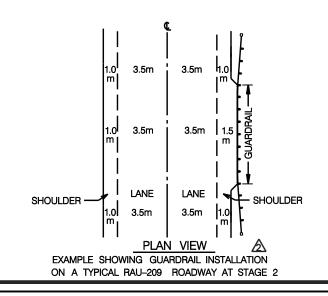
	IEDIAN BARRIER
	WING END (SEE DRAWING TEB 3.11a FOR DETAILS)
TRAVEL LANE TRAVEL LANE SHOULDER MEDIAN MEDIAN TRAVEL LANE TRAVEL LANE TRAVEL LANE SHOULDER TRAVEL LANE TRAVEL LANE SHOULDER TRAVEL LANE TRAVEL LANE SHOULDER TRAVEL LANE TRAVEL LANE SHOULDER TRAVEL LANE SHOULDER TRAVEL LANE TRAVEL LANE SHOULDER TRAVEL LANE SHOULDER SH	AND AND AND AND AND AND AND AND
CONTINUOUS M	EDIAN BARRIER
SHOULDER TRAVEL LANE SHOULDER HAZARD HAZARD TRAVEL LANE HAZARD	a * a * b MEDIAN DOUBLE RAIL ONLY IF INSIDE CLEAR ZONE
 * FLARE RATE AS PER STANDARD FOR DESIGN SPEED. CONSIDER ONLY WHERE MEDIAN WIDTH SUFFICIENT TO PROVIDE 8m MINIMUM FROM OPPOSING TRAVEL LANE TO BACK SIDE OF TERMINAL. FOR NARROW MEDIAN, IMPACT SYSTEMS ARE REQUIRED. THE LENGTH OF NEED SHALL BE BASED ON THE PROTECTION ENVELOPE. CLEARANCE BETWEEN GUARDRAIL AND OBSTRUCTION: STRONG POST (WOOD AND STEEL POSTS) 0.9m THRIE BEAM 0.9m STRONG POST (PLASTIC POSTS) 1.5m 	No. REVISIONS BY DAT Approved: Original signed by Original signed by Allan Kwan Executive Director, Technical Standards Branch Date: JUNE 24, 2005 Effective Date: JULY 12, 2005
THRIE BEAM 0.9m STRONG POST (PLASTIC POSTS) 1.5m DATA FOR 15m CURVE D=3830' R=225.0m SR=7.508m CL=15.0m END TREATMENT IS TO BE DETERMINED BY PROJECT MANAGER TO EACH GIVEN SITUATION.	TYPICAL STRONG POST W-BEAM OR MODIFIED THRIE BEA GUARDRAIL PLACEMENT FOR MEDIAN HAZARDS
All dimensions are in millimetres unless otherwise indicated.	Prepared Checked Scale: Dwg No.: By: M.T By: B.K N.T.S. TEB 3.18a

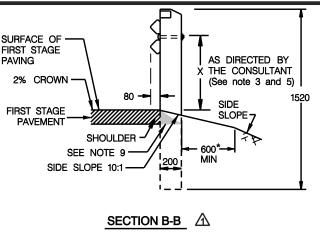


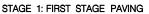


NOTES:

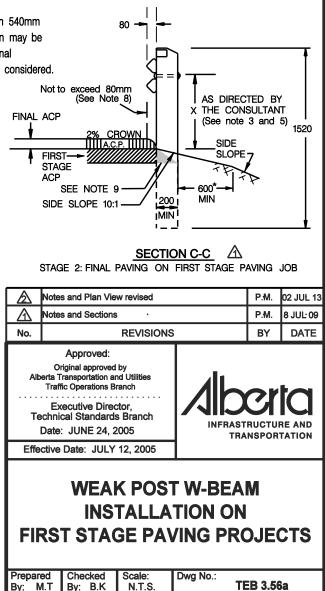
- Sections B–B and C–C show the special installation of guardrail that is suggested on projects where final paving is anticipated within 10 years.
- The standard height of guardrail from the road surface to the centre bolt is 540mm (Section A-A). The acceptable design range for height of guardrail installation is from 465mm to 615mm i.e. 6 +/-75mm
- 3. The guardrail elevation 'x' is to be set by the consultant. An elevation between 540mm and 615mm is normally chosen for First Stage Paving projects. The elevation may be selected based on an estimate of the Final Pavement thickness. Where the final pavement thickness is not known, the highest permissible elevation should be considered.
- The acceptable tolerance for height of guardrail at time of construction or maintenance is shown in the applicable specification.
- By installing guardrail at the highest permissible elevation (Section B–B), the final pavement can be placed without the guardrail being removed or adjusted (Section C–C).
- 6. All dimensions are in millimetres unless otherwise noted.
- 7. Drawing is not to scale.
- Pavement drop-off line must not be more than 80mm from the guardrail post line. This is to prevent snagging.
- 9. Insatllation of guardrail system normally requires post to be installed through ACP and GBC layers.
- This Installation method will also apply to other barrier systems. The height tolerances will vary depending in the barrier system.



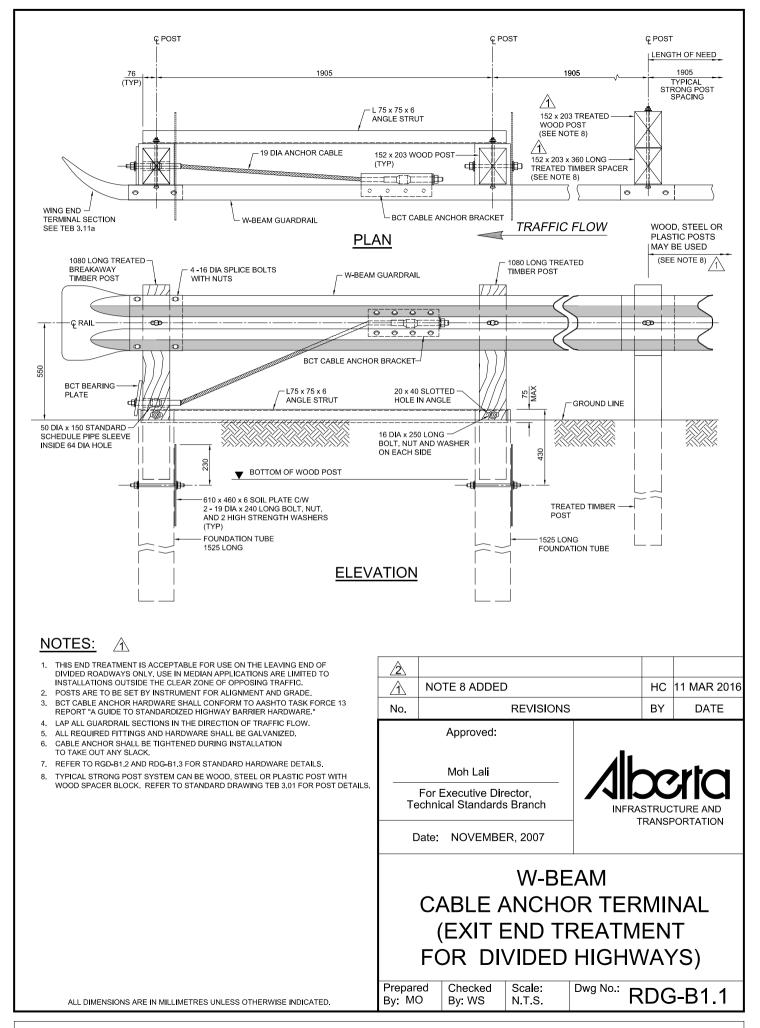


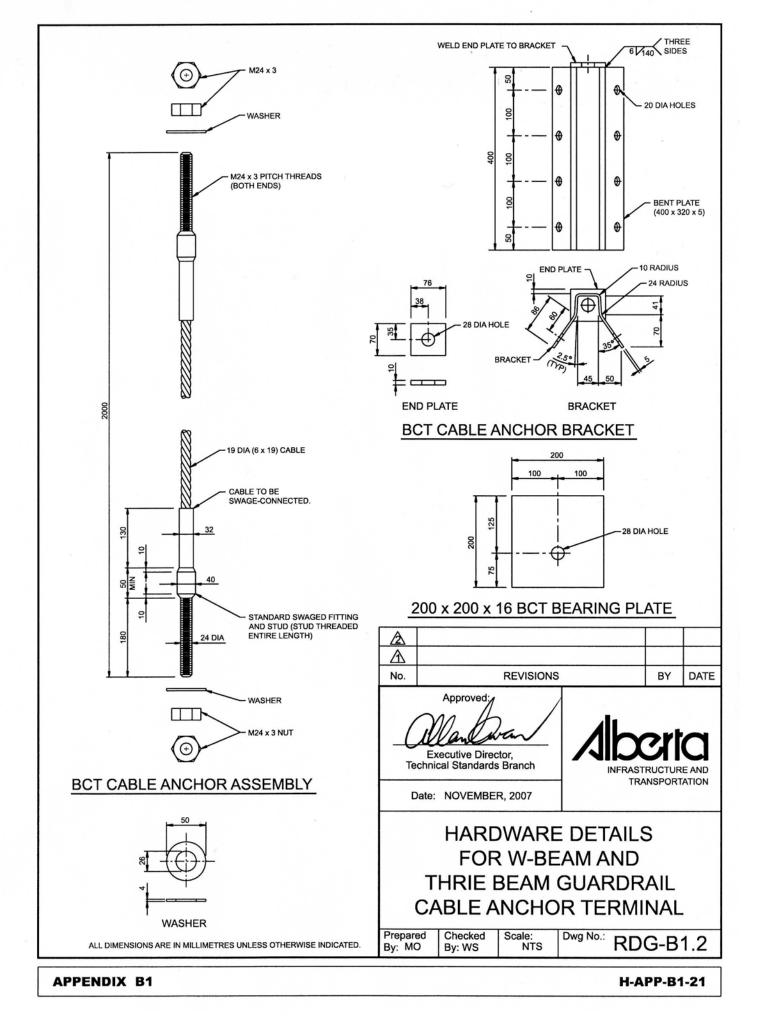


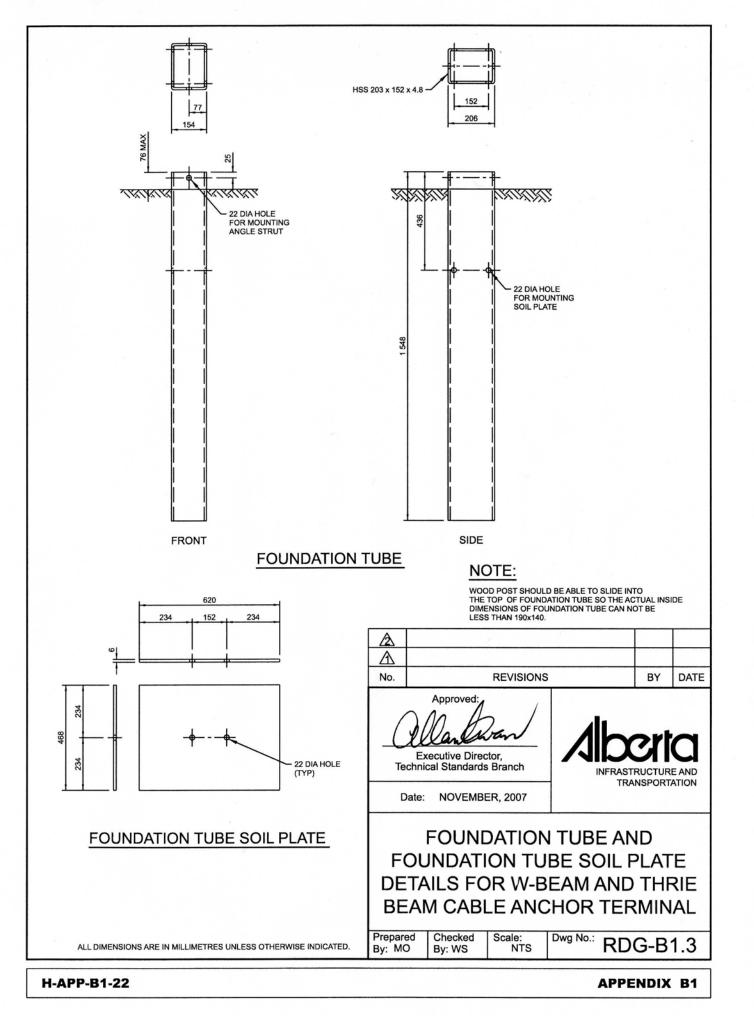
* The 600mm dimension may be exceeded if required by the consultant based on barrier type, hazard severity, embankment characteristics, etc.

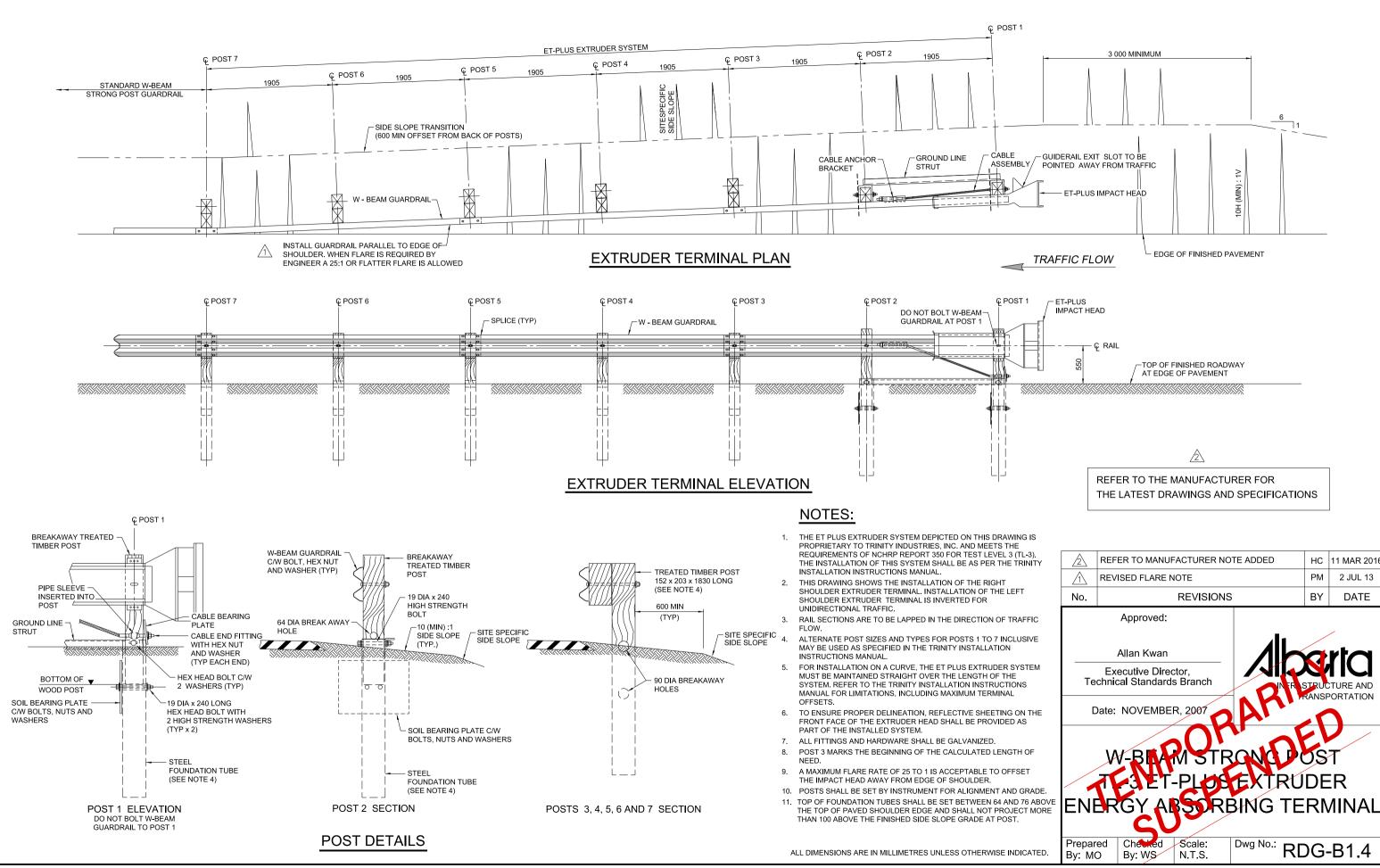


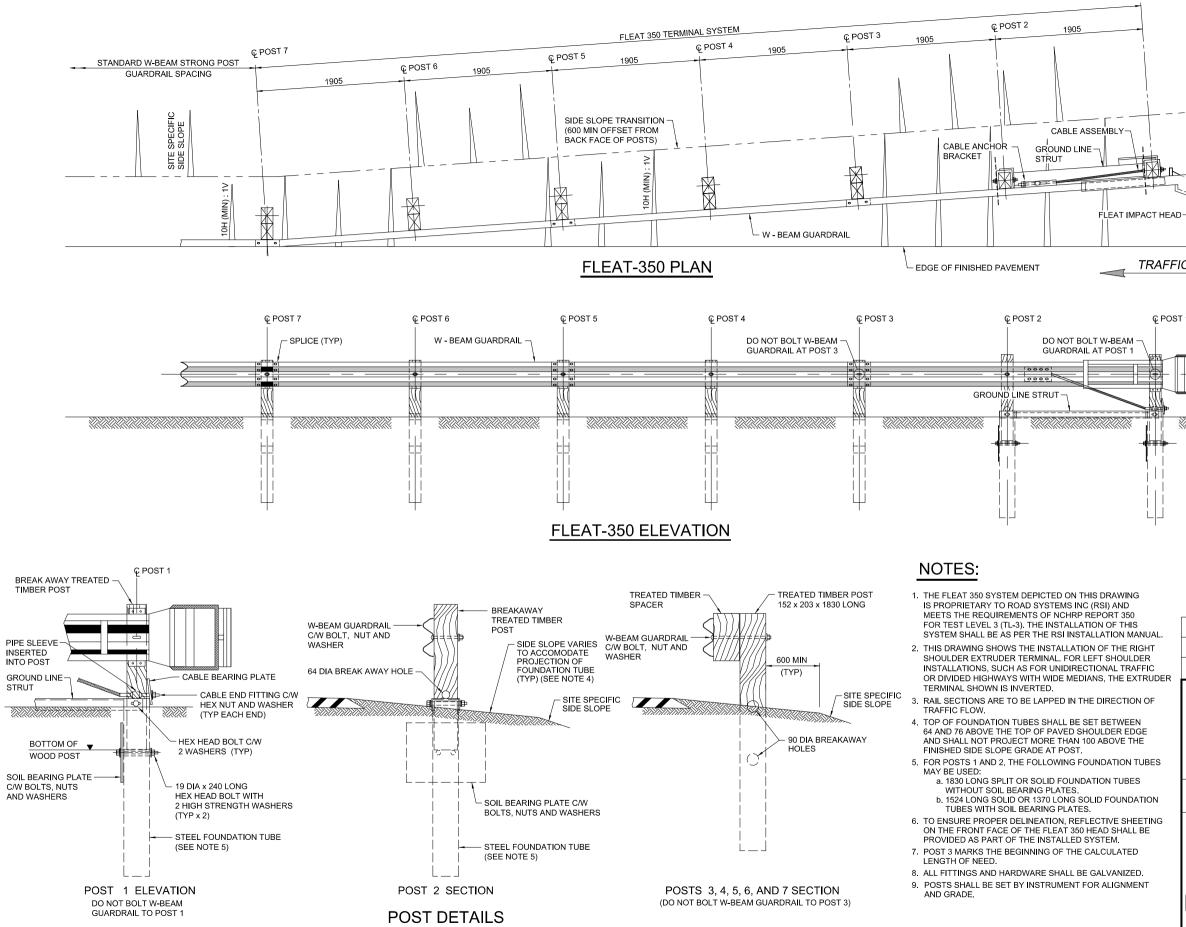
APPENDIX B1







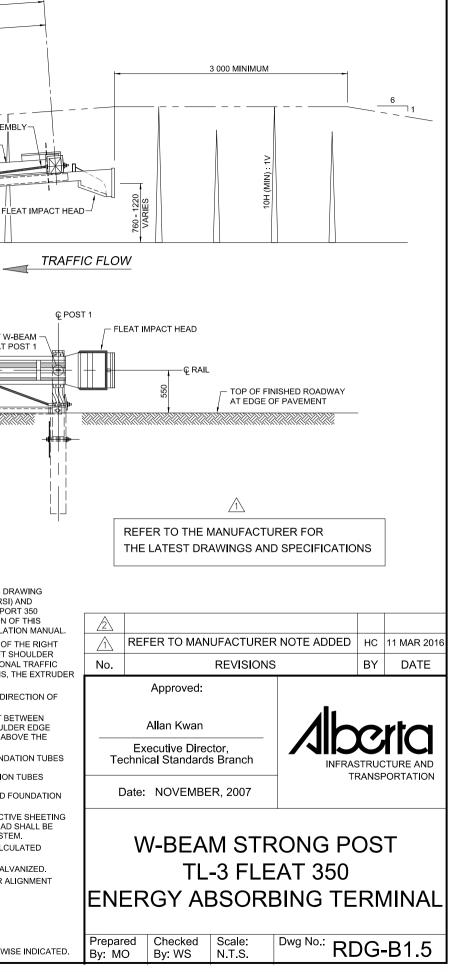


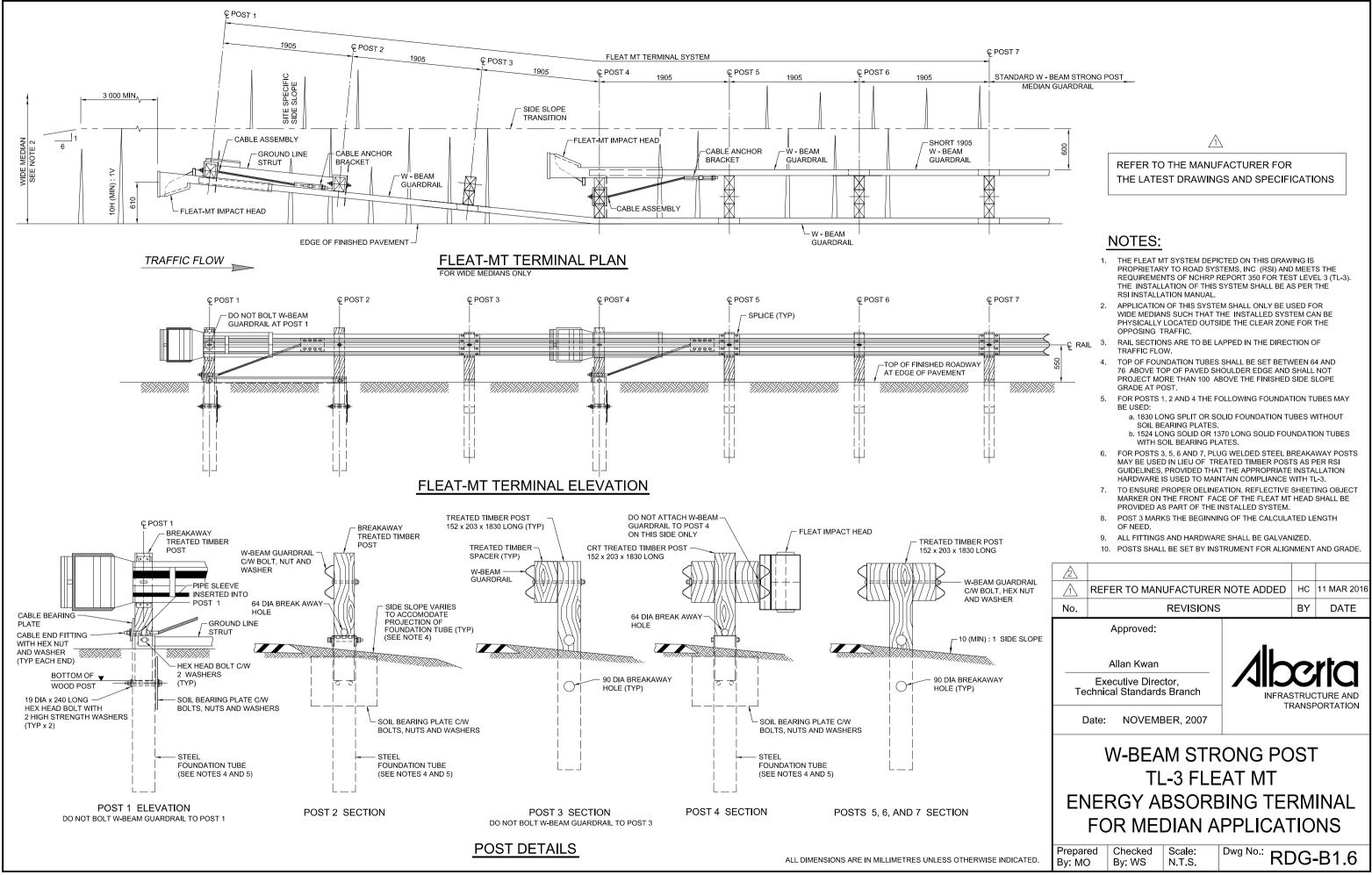


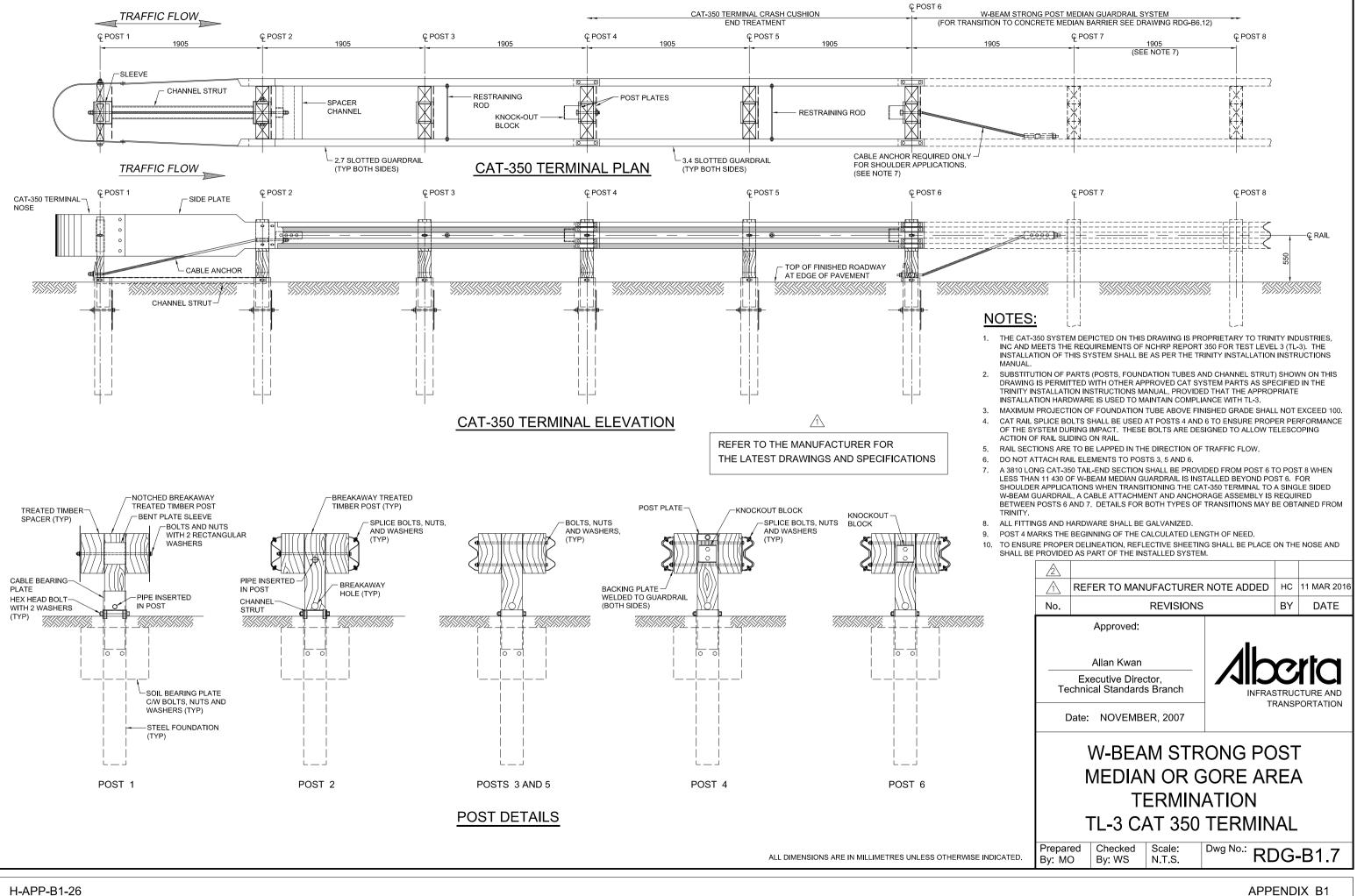
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE INDICATED.

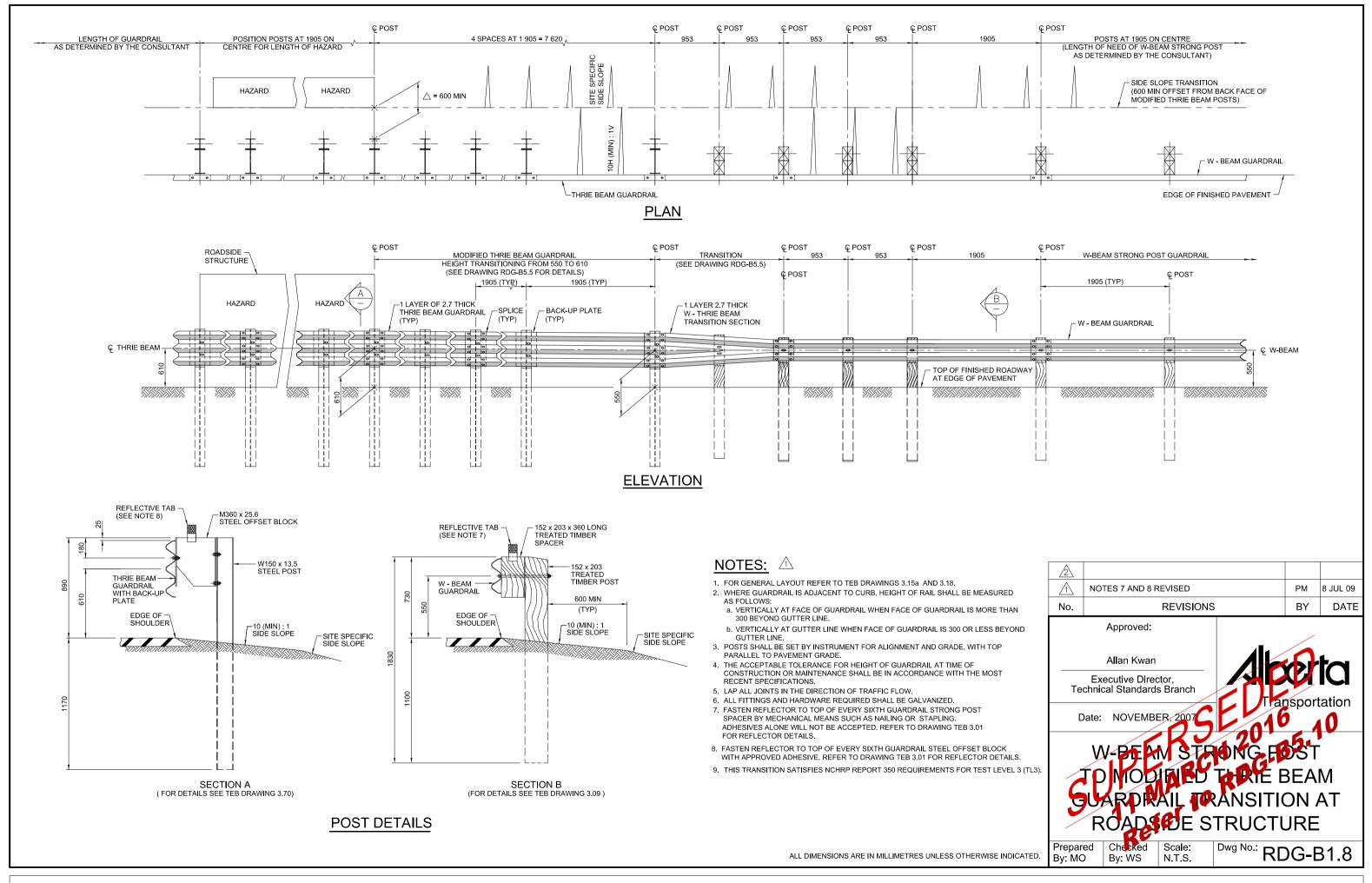
Ç POST 1

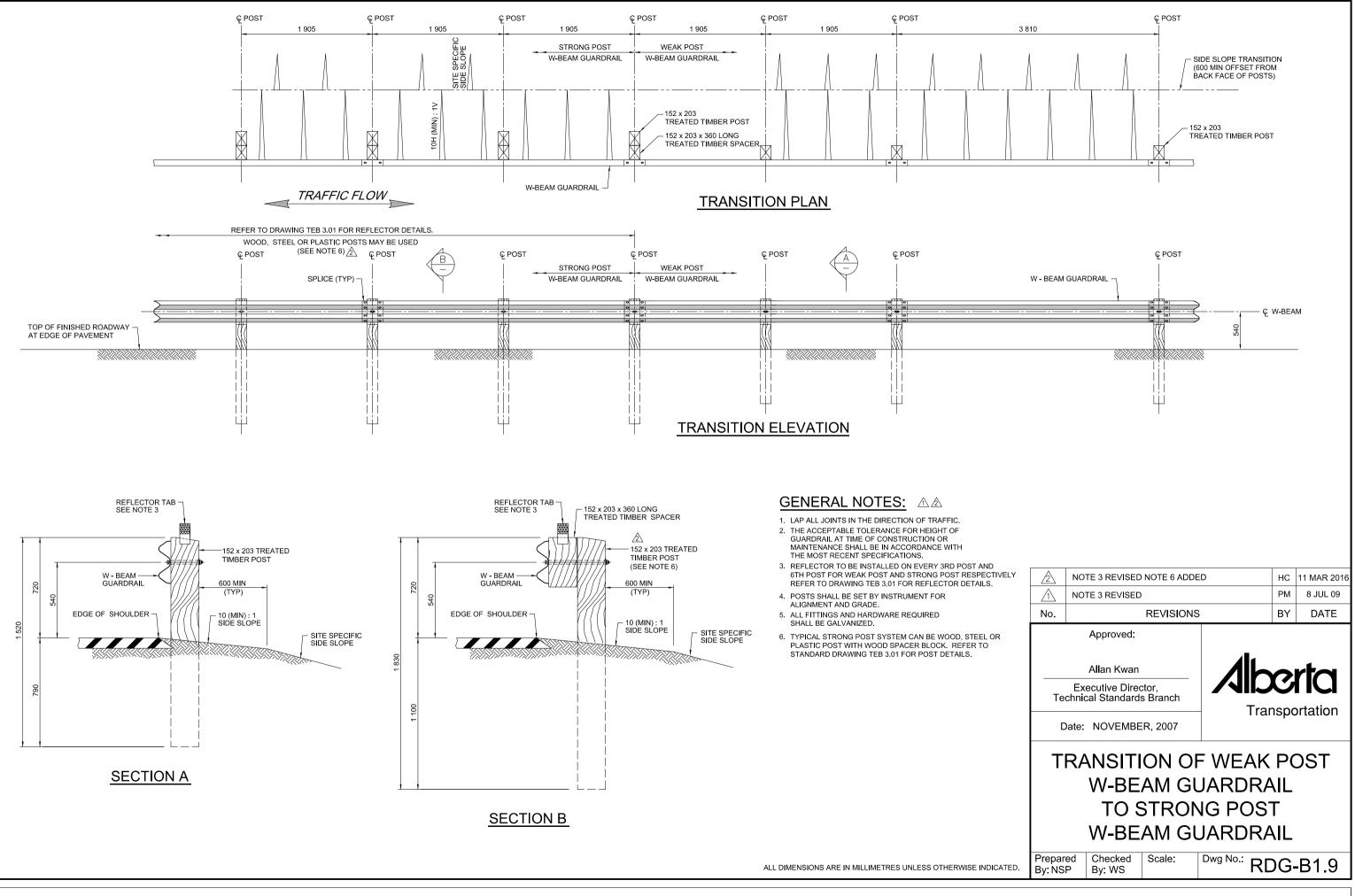


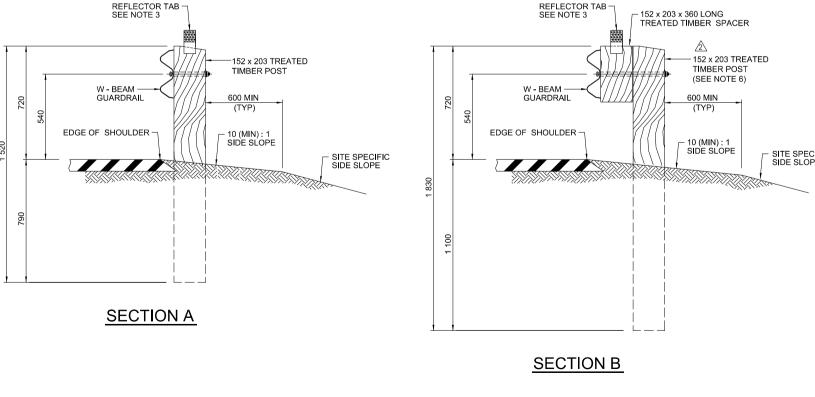


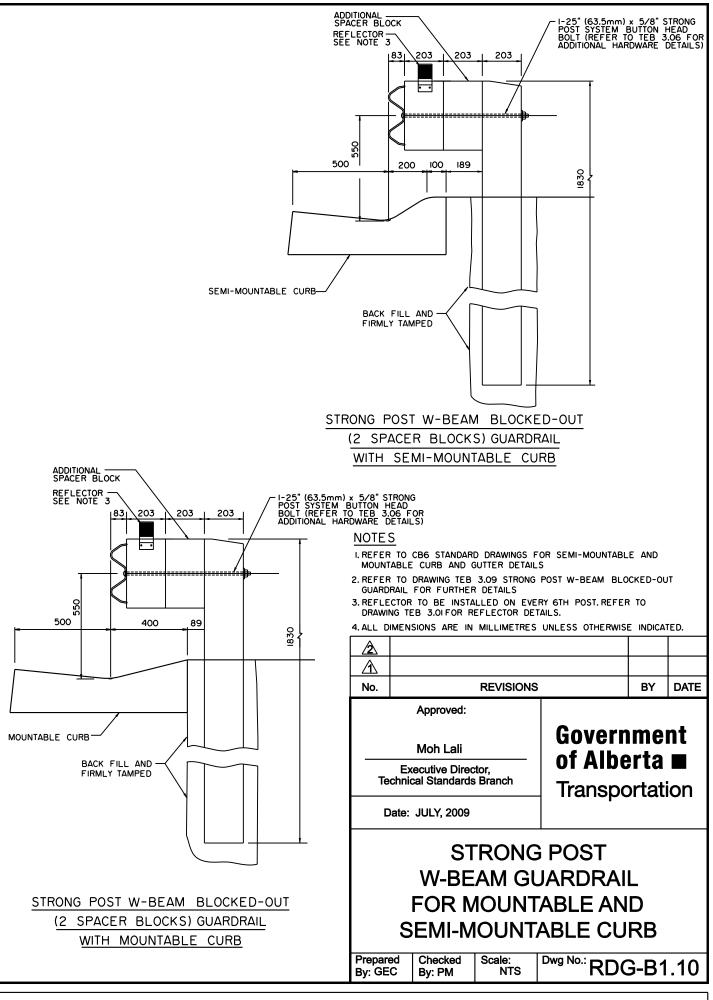




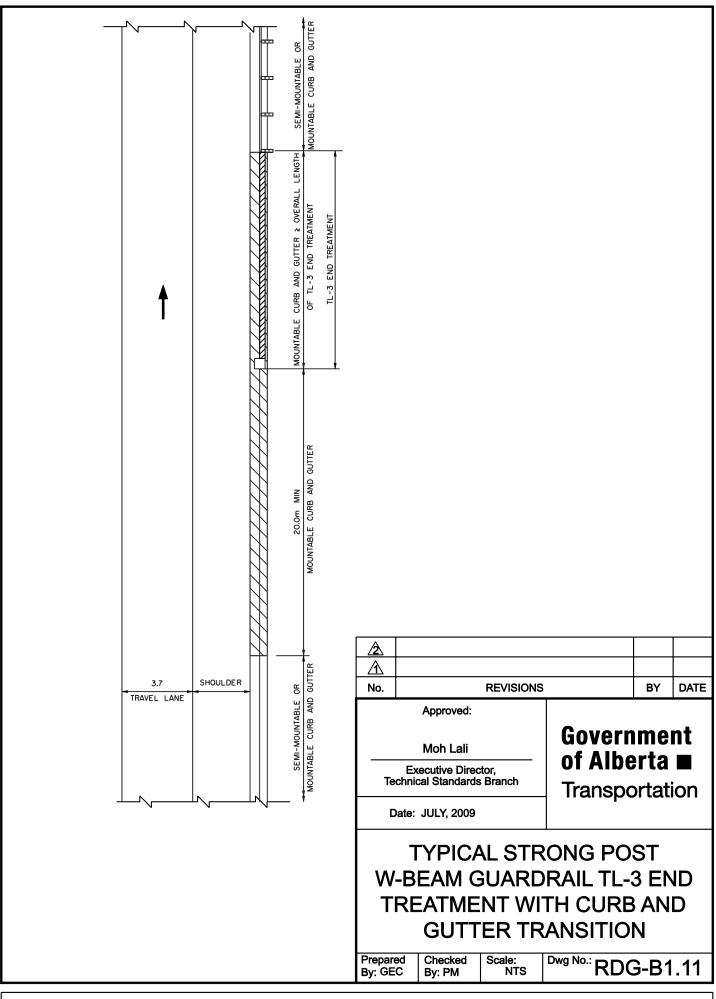


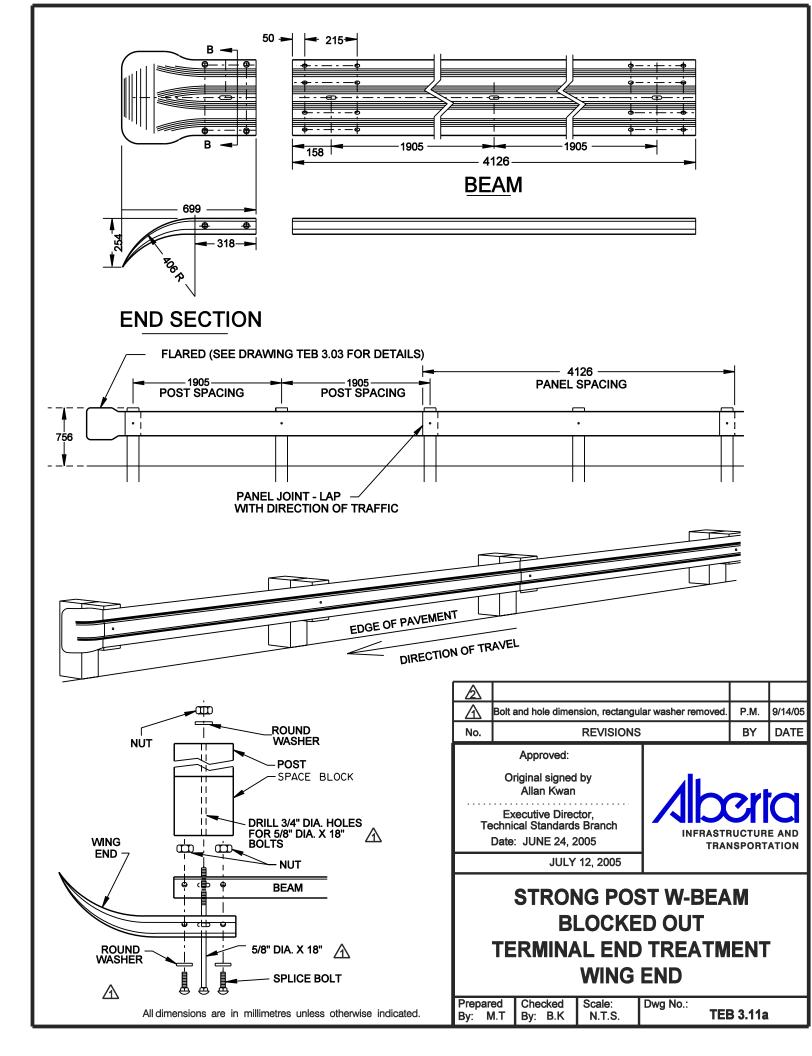


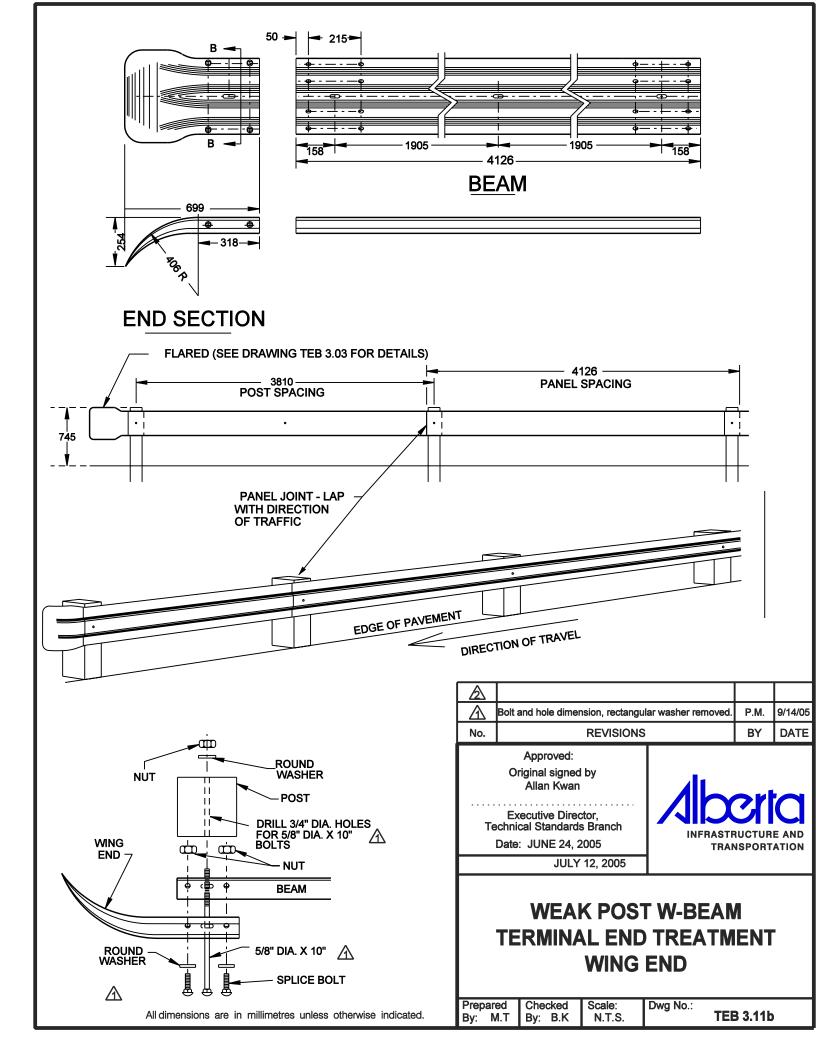


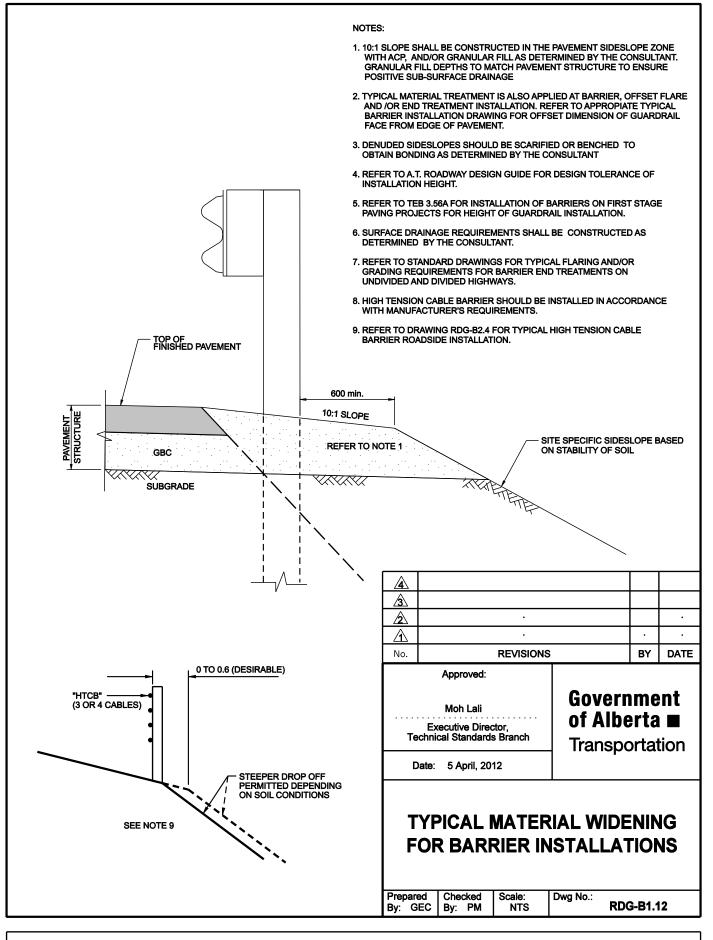


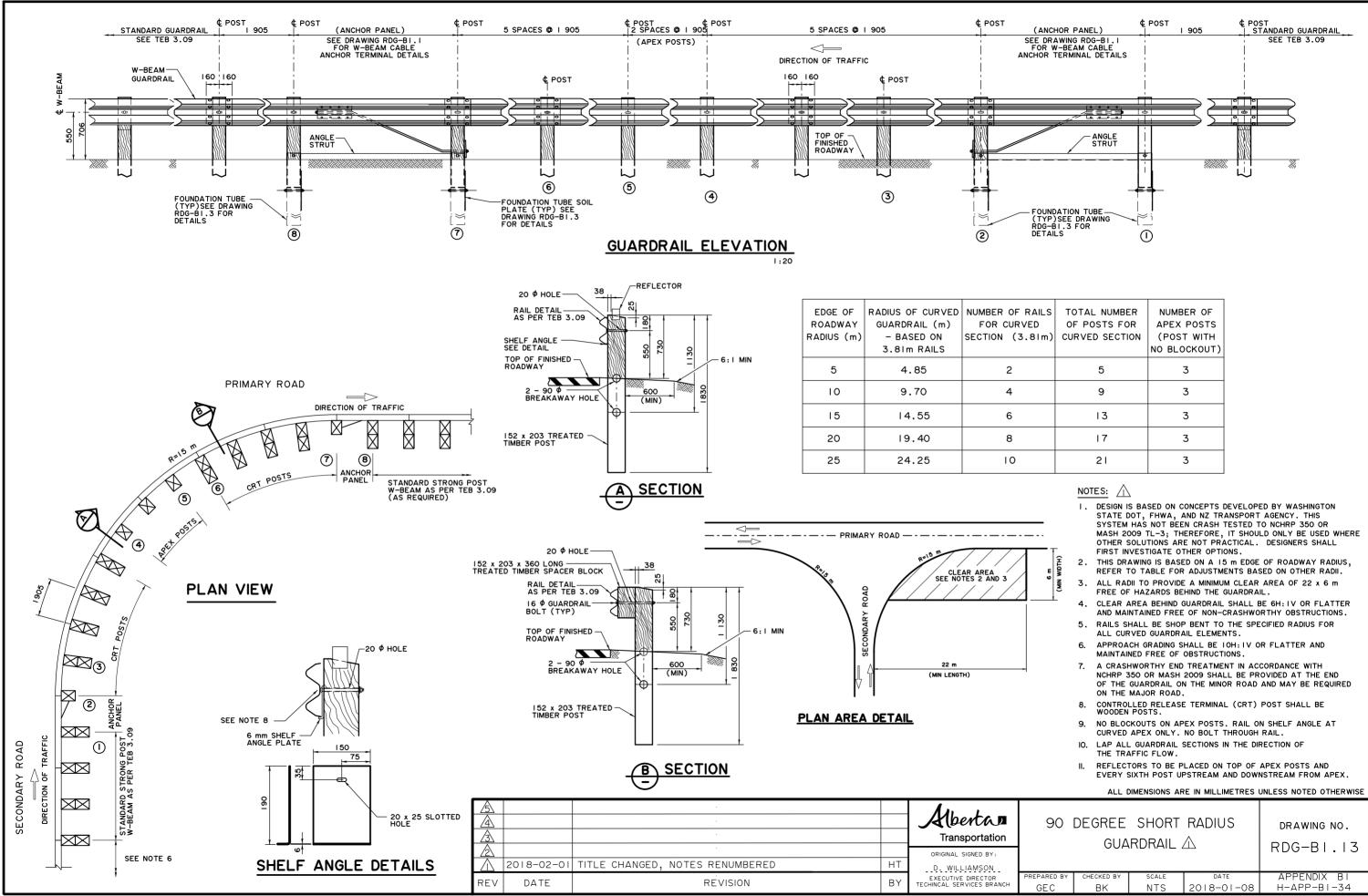
H-APP-B1-29





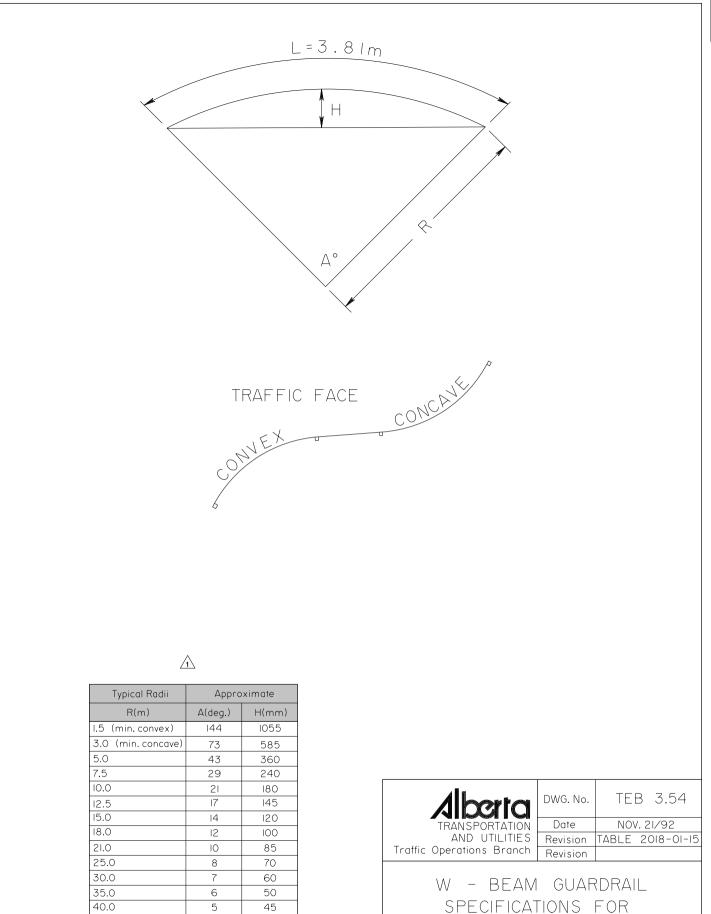






OF RAILS URVED (3.81m)	TOTAL NUMBER OF POSTS FOR CURVED SECTION	NUMBER OF APEX POSTS (POST WITH NO BLOCKOUT)
2	5	3
4	9	3
6	13	3
8	7	3
0	21	3

PREPARED BY	CHECKED BY	SCALE	DATE	APPENDIX BI
GEC	BK	NTS	2018-01-08	H-APP-BI-34



CURVED RAILS

A.D. Cherwenuk, Director

45.0

4.5

40

HIGH TENSION CABLE SYSTEM

Appendix B2 High Tension Cable Barrier

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Dwg. No.	Drawing Title	Page Number

RDG-B2.1	Typical HTCB Median Installation, Slopes 6H:1V or Flatter	Н-АРР-В2-3
RDG-B2.2	Typical HTCB Median Installation, $6H:1V > Slopes \ge 4H:1V$	H-APP-B2-4
RDG-B2.3	Typical HTCB Median Installation, Slopes Steeper Than 4H:1V	H-APP-B2-5
RDG-B2.4	Typical HTCB Roadside Installation	H-APP-B2-6

High Tension Cable Barrier System

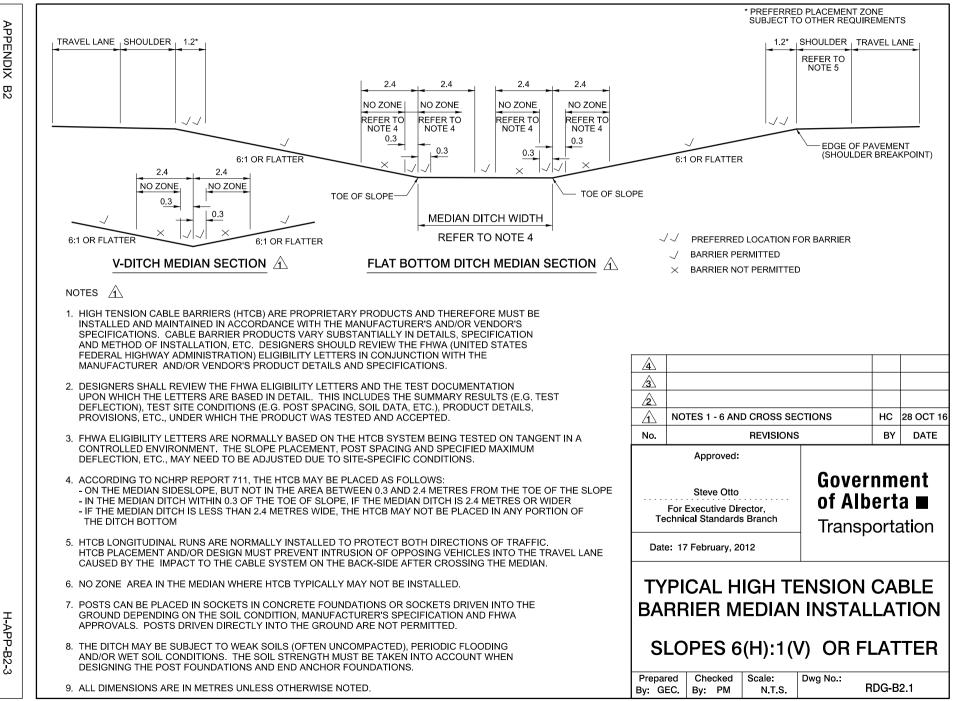
Product Development and Acceptance Testing

The Alberta Transportation Products List of proven, trial and potential products for HTCB (including vendor information) is available at the link indicated below.

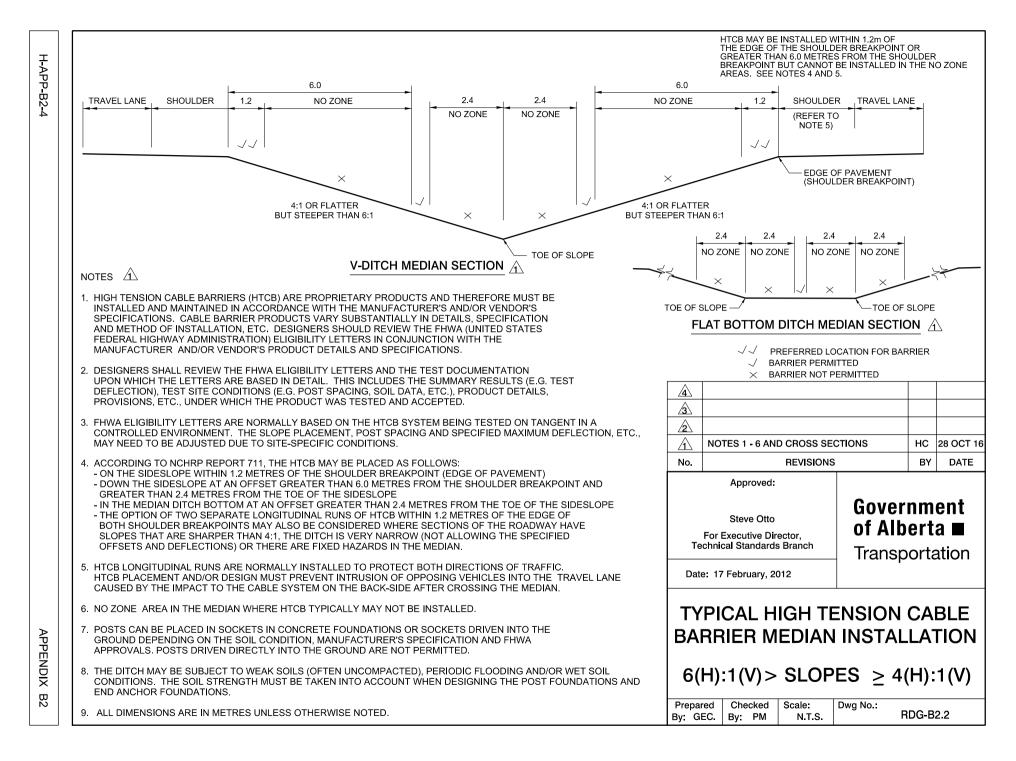
http://www.transportation.alberta.ca/689.htm

As of September 8, 2016 there are five proprietary HTCB systems in North America that have passed NCHRP 350 and/or MASH 2009. They are listed below in alphabetical order. Not all of the tested systems are on the Alberta Transportation Products List.

- Brifen Canada
 15521 Marine Drive
 White Rock, BC V4B 1C9, Canada
 http://www.brifen.ca
- Gibraltar
 4303 Innovation Loop
 Marble Falls, TX 78654, USA
 http://gibraltarus.com
- Nucor Steel Marion, Inc.
 912 Cheney Avenue Marion, Ohio 43302, USA <u>http://www.nucorhighway.com</u> (Note: Not on Alberta Transportation's Products List dated September 8, 2016)
- SAFENCE (Gregory Industries, Inc.) 4100 13th Street, SW Canton, Ohio 44710, USA http://www.gregorycorp.com
- Trinity Highway Safety Products
 2525 North Stemmons Freeway
 Dallas, Texas 75207, USA
 http://www.highwayguardrail.com



APPENDIX



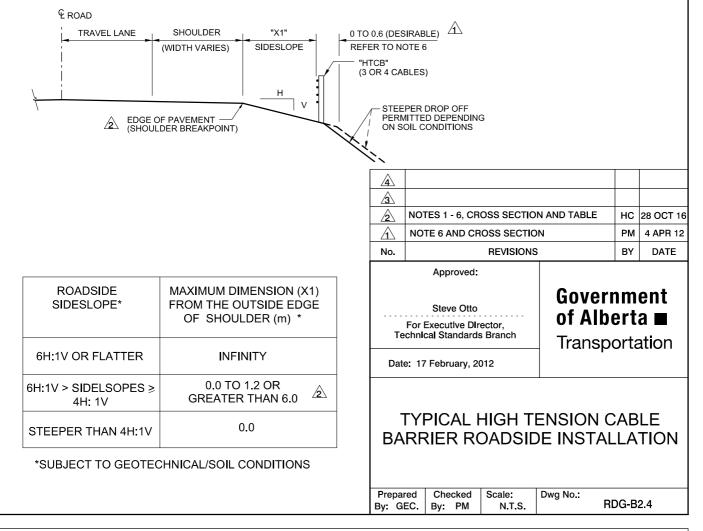
H-APP-B2-5

TRAVEL LANE SHOULDER NO ZONE	SHOULDER TRAVEL I	LANE
"HTCB" (3 OR 4 CABLES)		
STEEPER THAN 4:1 TOE OF SLOPE	EDGE OF PAVEMENT (SHOULDER BREAKPOINT)	
 NOTES A 1. HIGH TENSION CABLE BARRIERS (HTCB) ARE PROPRIETARY PRODUCTS AND THEREFORE MUST BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURER'S AND/OR VENDOR'S SPECIFICATIONS. CABLE BARRIER PRODUCTS VARY SUBSTANTIALLY IN DETAILS, SPECIFICATION AND METHOD OF INSTALLATION, ETC. DESIGNERS SHOULD REVIEW THE FHWA (UNITED STATES FEDERAL HIGHWAY ADMINISTRATION) ELIGIBILITY LETTERS IN CONJUNCTION WITH THE MANUFACTURER AND/OR VENDOR'S PRODUCT DETAILS AND SPECIFICATIONS. 2. DESIGNERS SHALL REVIEW THE FHWA ELIGIBILITY LETTERS AND THE TEST DOCUMENTATION UPON WHICH THE LETTERS ARE BASED IN DETAIL. THIS INCLUDES THE SUMMARY RESULTS (E.G. TEST DOCUMENTATION) 	▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ ▲ CABLES ON MEDIAN SIDE RE ▲ NOTES 1 - 5 AND CROSS SEC No. REVISIONS	
 DEFLECTION), TEST SITE CONDITIONS (E.G. POST SPACING, SOIL DATA, ETC.), PRODUCT DETAILS, PROVISIONS, ETC., UNDER WHICH THE PRODUCT WAS TESTED AND ACCEPTED. 3. FHWA ELIGIBILITY LETTERS ARE NORMALLY BASED ON THE HTCB SYSTEM BEING TESTED ON TANGENT IN A CONTROLLED ENVIRONMENT. THE SLOPE PLACEMENT, POST SPACING AND SPECIFIED MAXIMUM DEFLECTION, ETC., MAY NEED TO BE ADJUSTED DUE TO SITE-SPECIFIC CONDITIONS. 4. HTCB TYPICALLY SHOULD BE PLACED UNDER THE FOLLOWING CONDITIONS: AT THE SHOULDER BREAKPOINT (0.0 m LATERAL OFFSET FROM THE EDGE OF PAVEMENT). THE OPTION OF TWO SEPARATE LONGITUONIAL RUNS OF HTCB SHOULD ALSO BE CONSIDERED WHERE THE MEDIAN WIDTH IS NARROW, DESIRABLE DEFLECTION SPACE IS GREATER THAN THE SHOULDER WIDTH, AND/OR GENERAL RE-GRADING IS NOT AN OPTION, ETC. 	Approved: Steve Otto For Executive Director, Technical Standards Branch Date: 17 February, 2012	Government of Alberta ■ Transportation
 NO ZONE AREA IN THE MEDIAN WHERE HTCB TYPICALLY MAY NOT BE INSTALLED. POSTS CAN BE PLACED IN SOCKETS IN CONCRETE FOUNDATIONS OR SOCKETS DRIVEN INTO THE GROUND DEPENDING ON THE SOIL CONDITION, MANUFACTURER'S SPECIFICATION AND FHWA APPROVALS. POSTS DRIVEN DIRECTLY INTO THE GROUND ARE NOT PERMITTED. 	TYPICAL HIGH TE BARRIER MEDIAN	
7. THE DITCH MAY BE SUBJECT TO WEAK SOILS (OFTEN UNCOMPACTED), PERIODIC FLOODING AND/OR WET SOIL CONDITIONS. THE SOIL STRENGTH MUST BE TAKEN INTO ACCOUNT WHEN DESIGNING THE POST FOUNDATIONS AND	SLOPES STEEPER	ΤΗΔΝ 4(Η)-1 (

APPENDIX B2

NOTES A

- 1. HIGH TENSION CABLE BARRIERS (HTCB) ARE PROPRIETARY PRODUCTS AND THEREFORE MUST BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH THE MANUFACTURER'S AND/OR VENDOR'S SPECIFICATIONS. CABLE BARRIER PRODUCTS VARY SUBSTANTIALLY IN DETAILS, SPECIFICATION AND METHOD OF INSTALLATION, ETC. DESIGNERS SHOULD REVIEW THE FHWA (UNITED STATES FEDERAL HIGHWAY ADMINISTRATION) ELIGIBILITY LETTERS IN CONJUNCTION WITH THE MANUFACTURER AND/OR VENDOR'S PRODUCT DETAILS AND SPECIFICATIONS.
- 2. DESIGNERS SHALL REVIEW THE FHWA ELIGIBILITY LETTERS AND THE TEST DOCUMENTATION UPON WHICH THE LETTERS ARE BASED IN DETAIL. THIS INCLUDES THE SUMMARY RESULTS (E.G. TEST DEFLECTION), TEST SITE CONDITIONS (E.G. POST SPACING, SOIL DATA, ETC.), PRODUCT DETAILS PROVISIONS, ETC., UNDER WHICH THE PRODUCT WAS TESTED AND ACCEPTED.
- 3. FHWA ELIGIBILITY LETTERS ARE NORMALLY BASED ON THE HTCB SYSTEM BEING TESTED ON TANGENT IN A CONTROLLED ENVIRONMENT. THE SLOPE PLACEMENT, POST SPACING AND SPECIFIED MAXIMUM DEFLECTION, ETC., MAY NEED TO BE ADJUSTED DUE TO SITE-SPECIFIC CONDITIONS.
- 4. HTCB SYSTEMS CAN TYPICALLY BE PLACED DOWN THE SIDESLOPES IF THE SLOPES ARE 4H:1V OR FLATTER. THIS SLOPE REFERS TO THE SLOPE ON THE ROADSIDE BETWEEN THE SHOULDER BREAK POINT AND THE BARRIER SYSTEM (DIMENSION "X1"). THE AREA IMMEDIATELY BEHIND THE BARRIER SYSTEM MAY BE CONSTRUCTED AT STEEPER SLOPES DEPENDING ON THE STABILITY OF THE SOIL.
- 5. HTCB SYSTEMS MAY NOT BE PLACED DOWN THE SLOPE ON SIDESLOPES STEEPER THAN 4H:1V UNLESS THE SYSTEM HAS BEEN SUCCESSFULLY CRASH TESTED UNDER THESE CONDITIONS (WITH AN FHWA ELIGIBILITY LETTER). HTCB MAY BE PLACED AT THE EDGE OF PAVEMENT (EDGE OF SHOULDER ON UNPAVED ROADS) AS SHOWN IN THE TABLE.
- 6. A CONTINUATION OF THE FLATTER SLOPE BEHIND THE BARRIER, FOR A DISTANCE OF 0.0 METRES TO 0.6 METRES (DESIRABLE SHOULD BE CONSIDERED. THE DISTANCE BEHIND THE BARRIER SYSTEM MAY VARY DEPENDING ON THE EMBANKMENT SLOPE, POST FOUNDATION/DEPTH, SOIL/GEOTECHNICAL CONDITION, POST SPACING, EXPECTED IMPACT CONDITION, ETC.
- 7. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE NOTED.



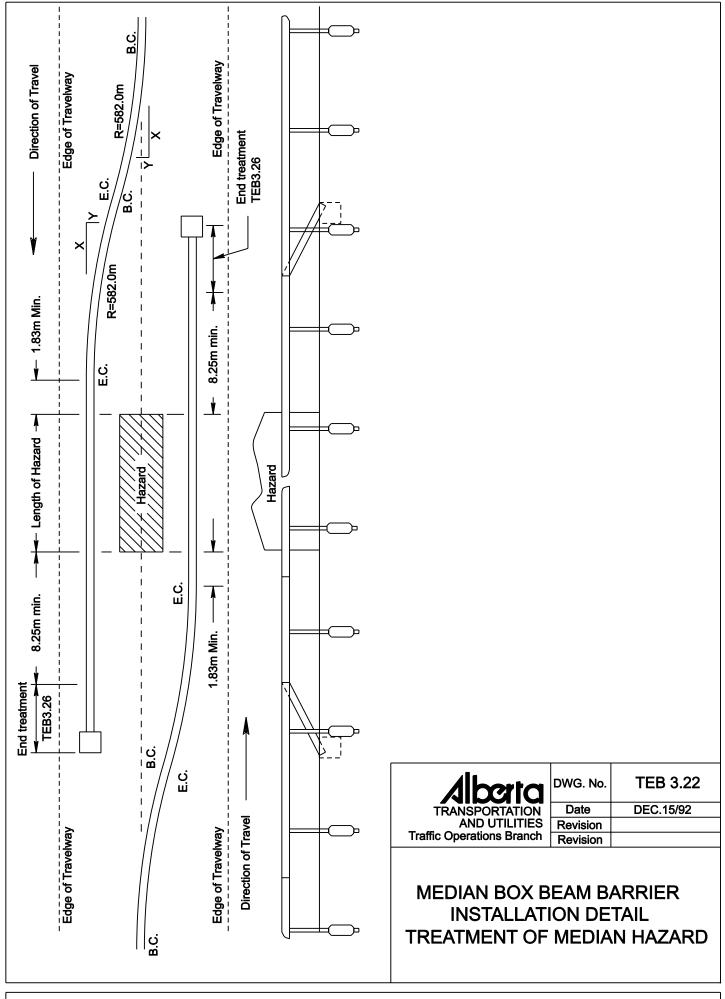
WEAK POST BOX BEAM GUARDRAIL

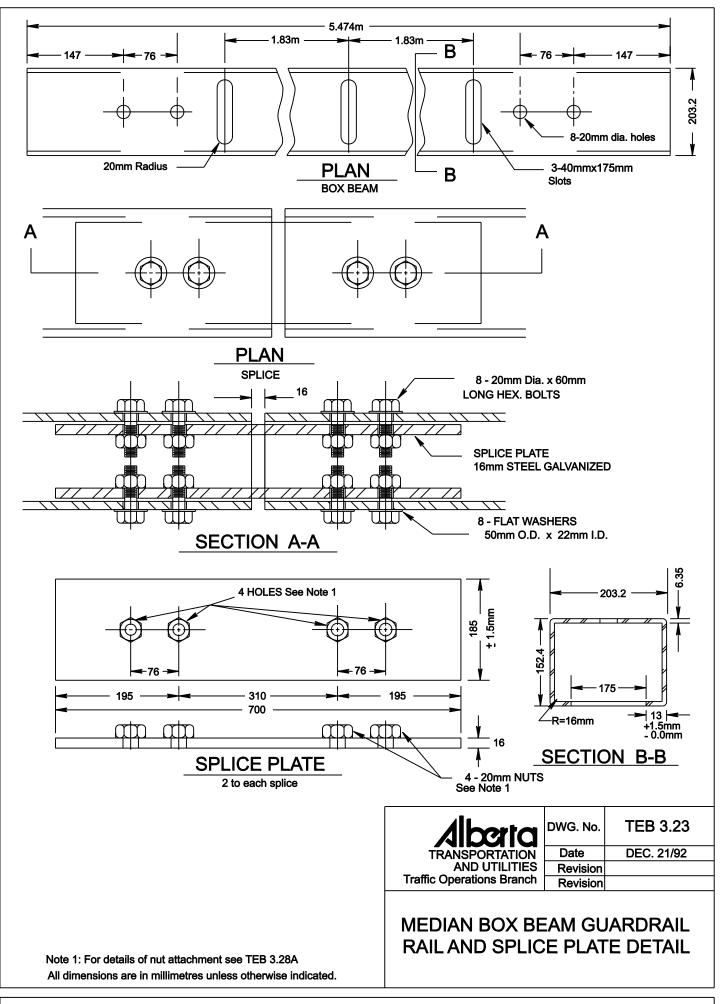
Appendix B3 Weak Post Beam Guardrail

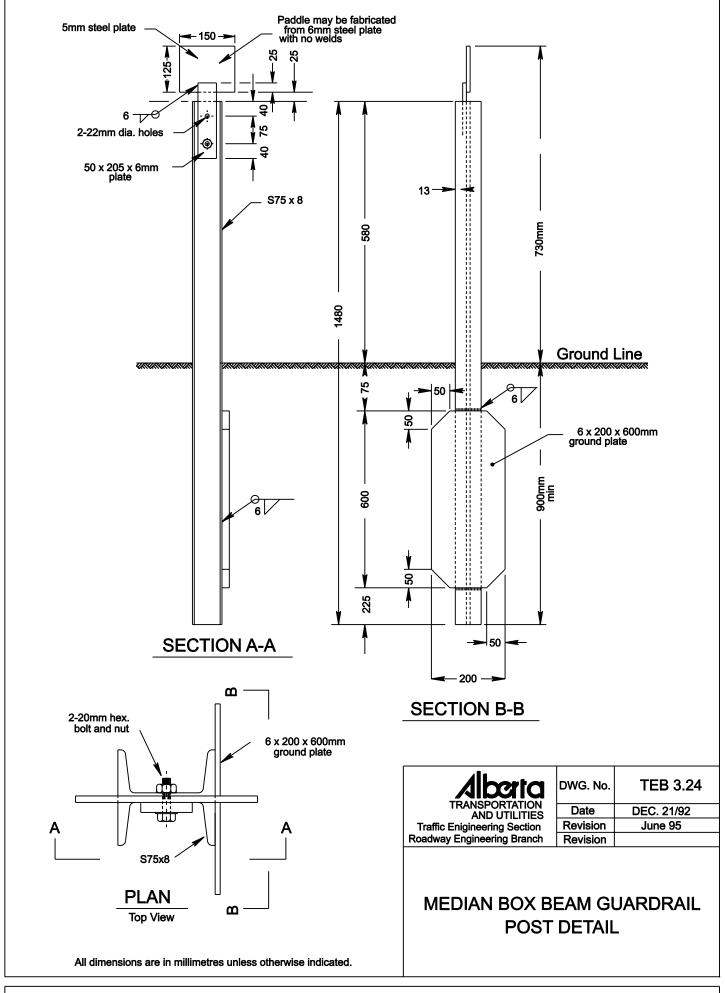
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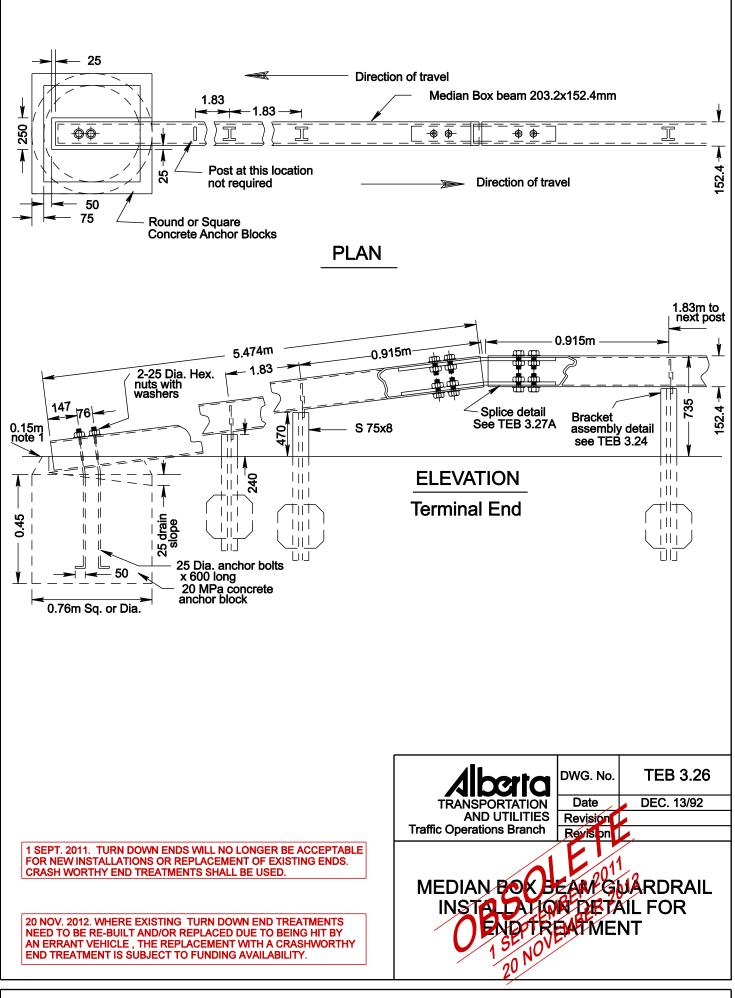
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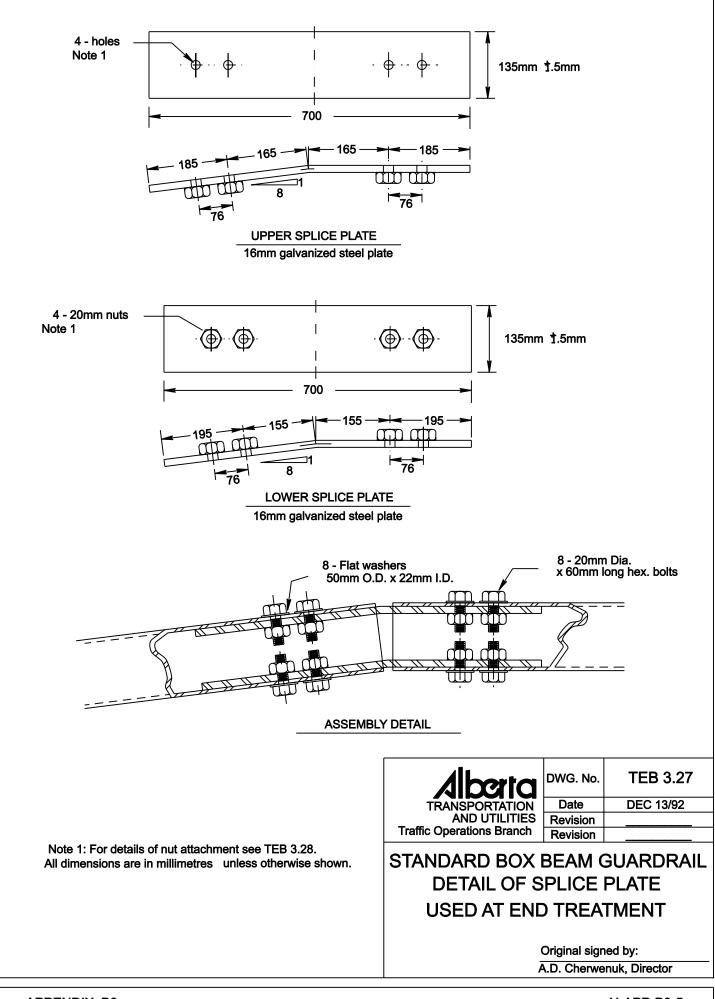
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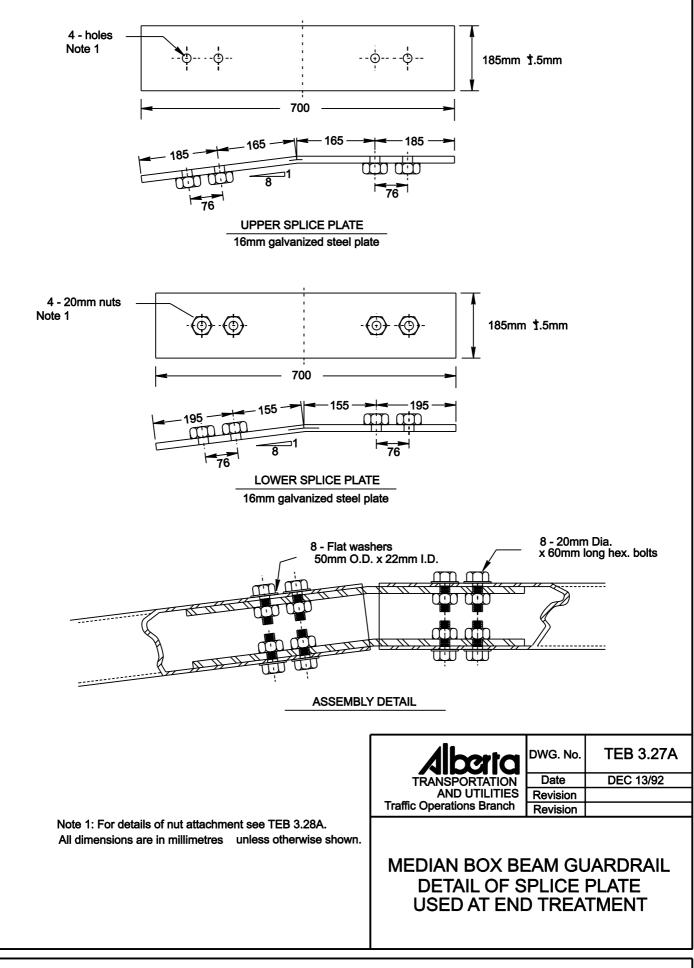


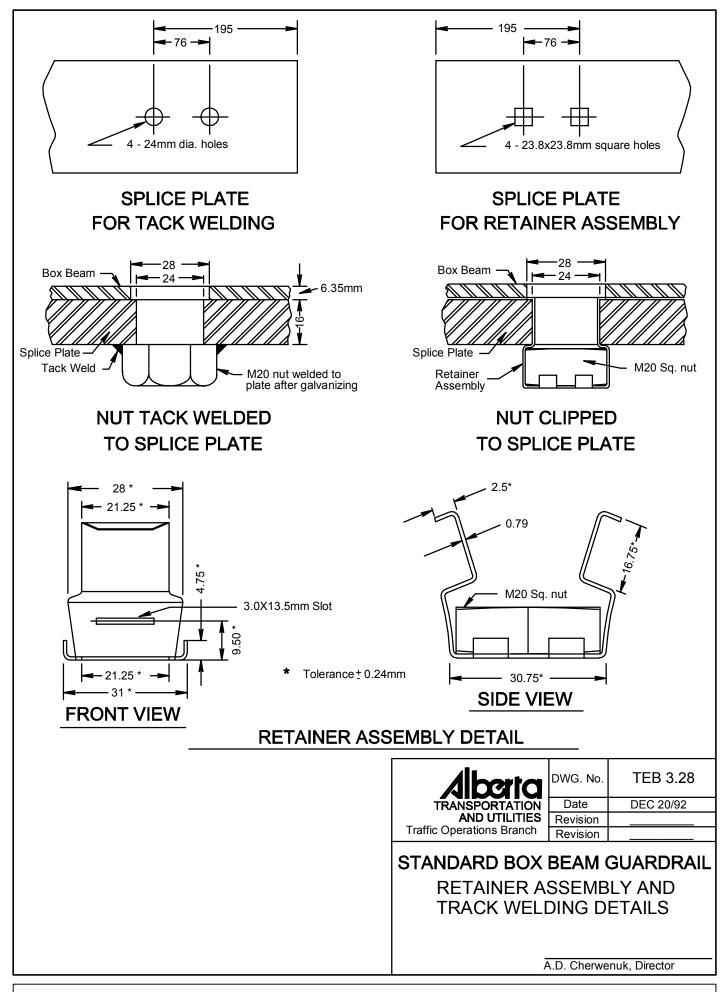


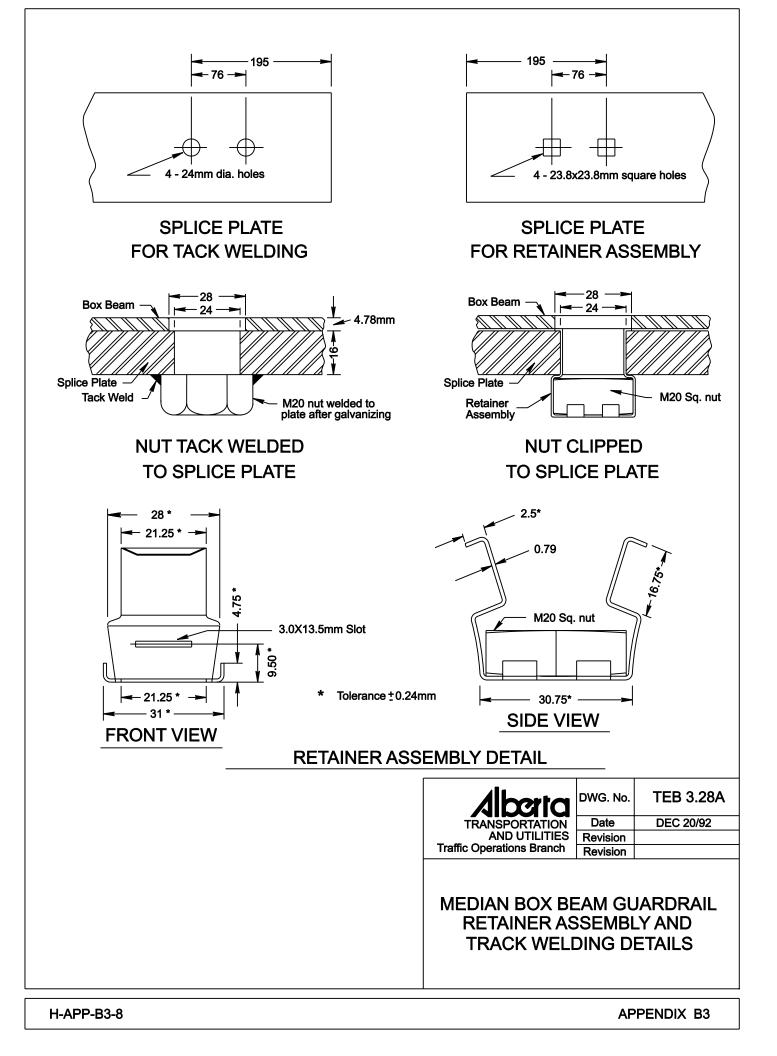


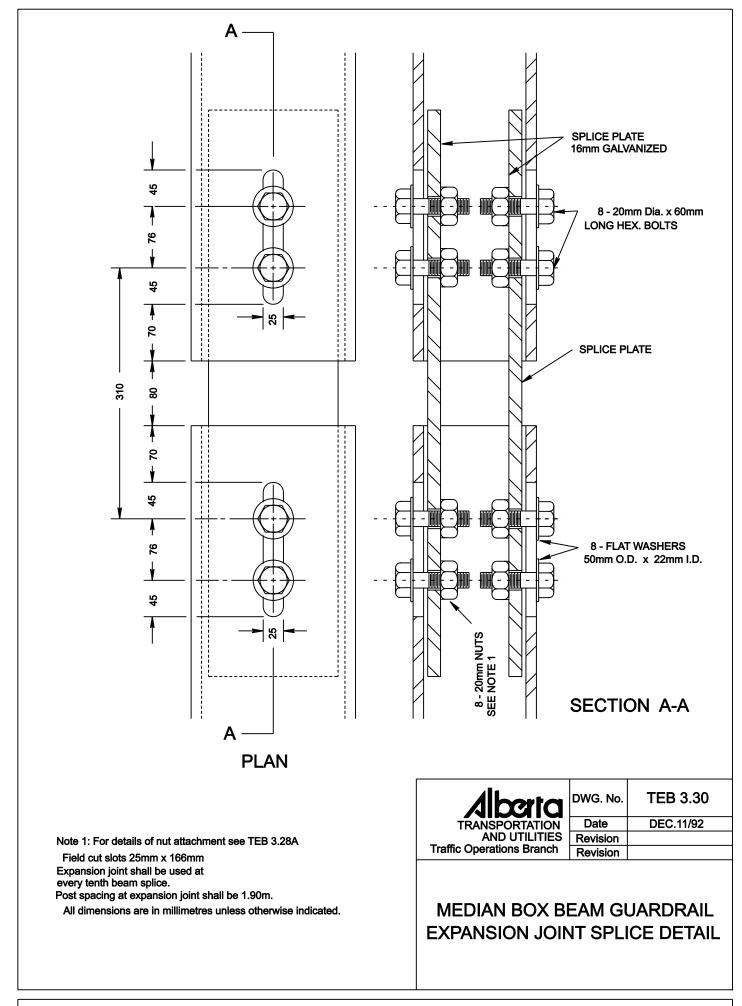


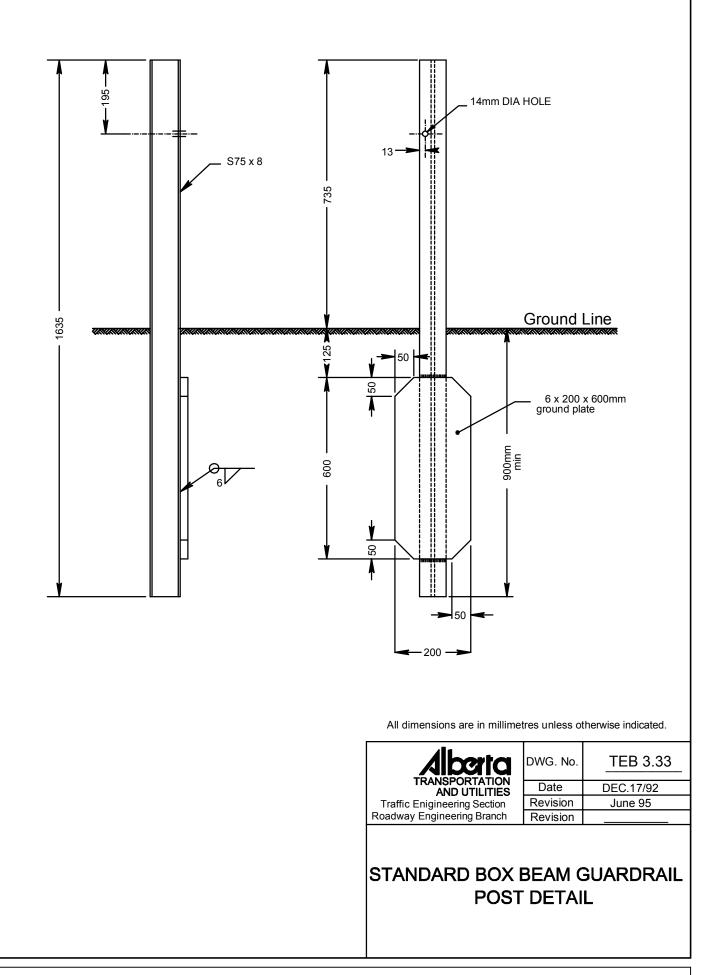


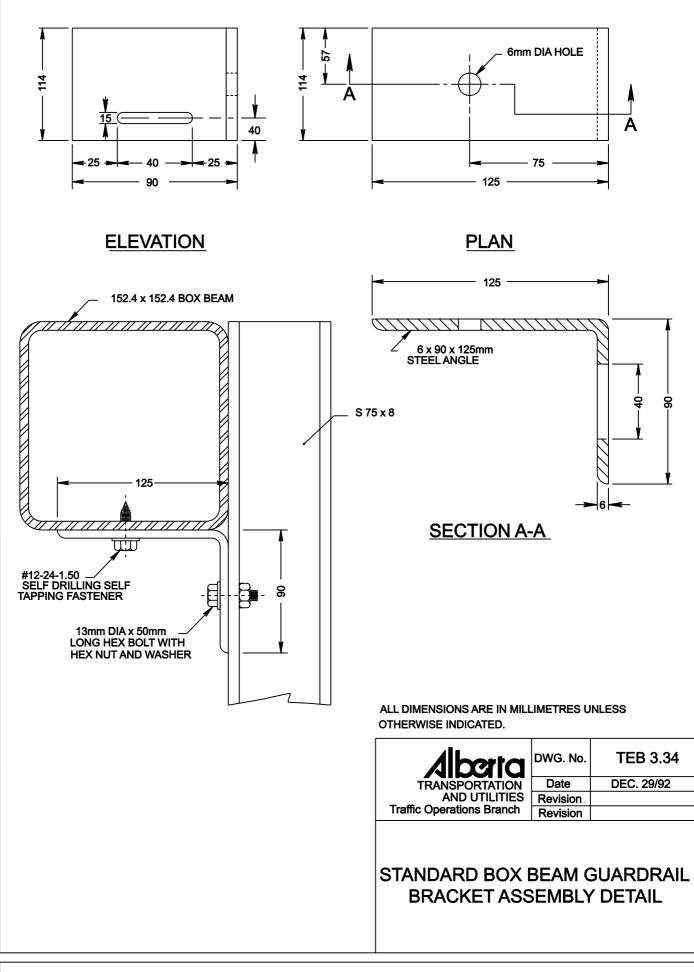


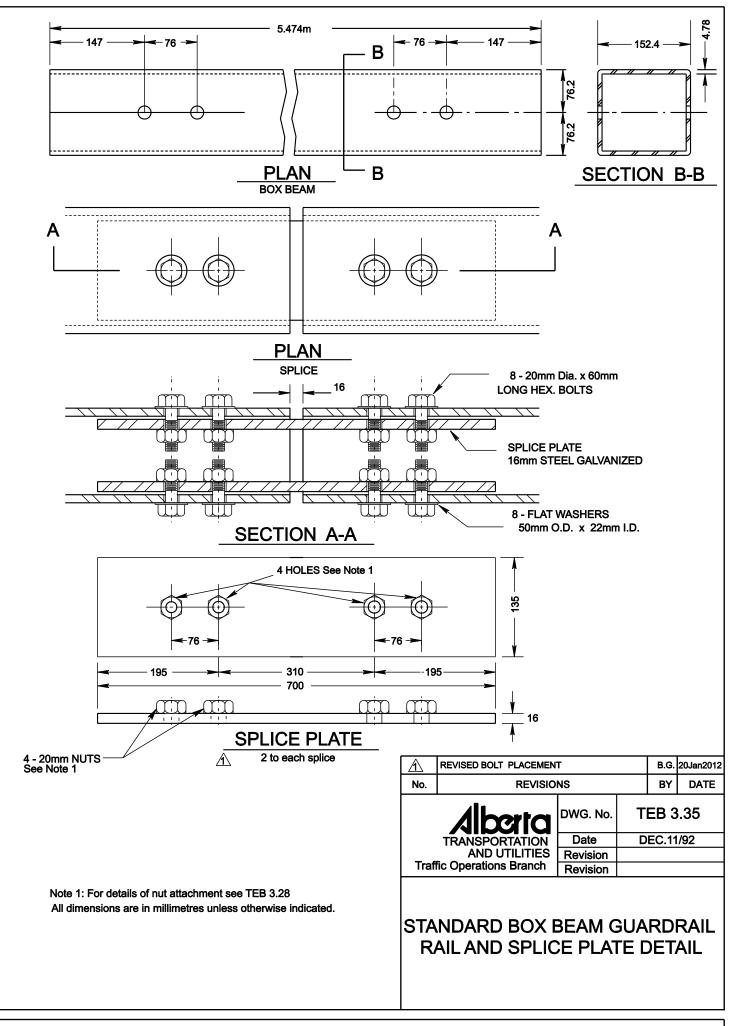






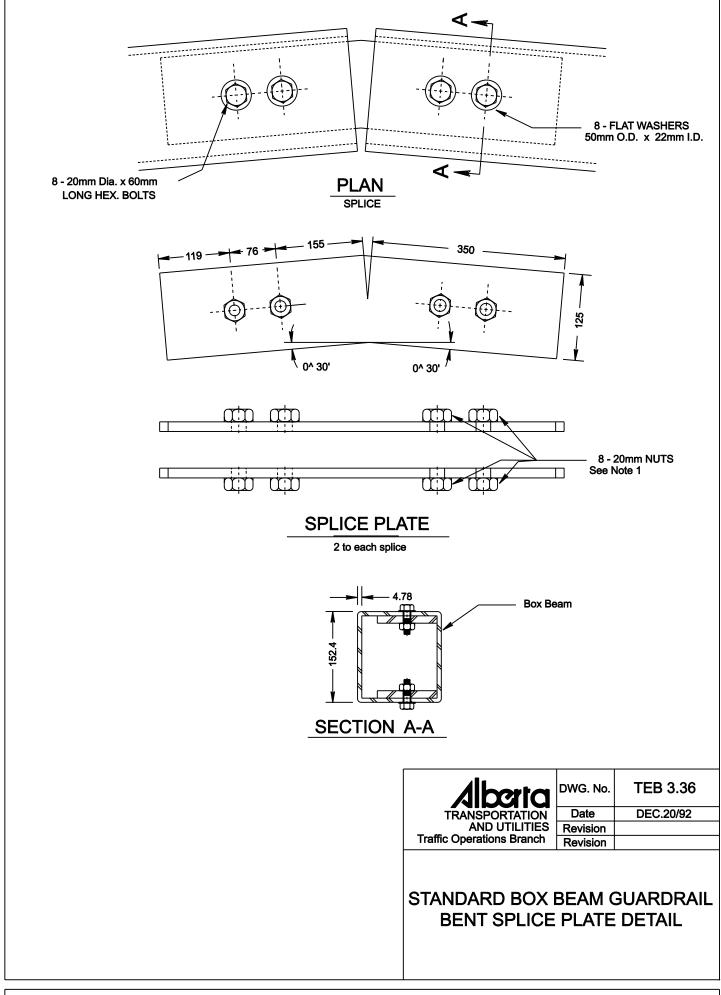


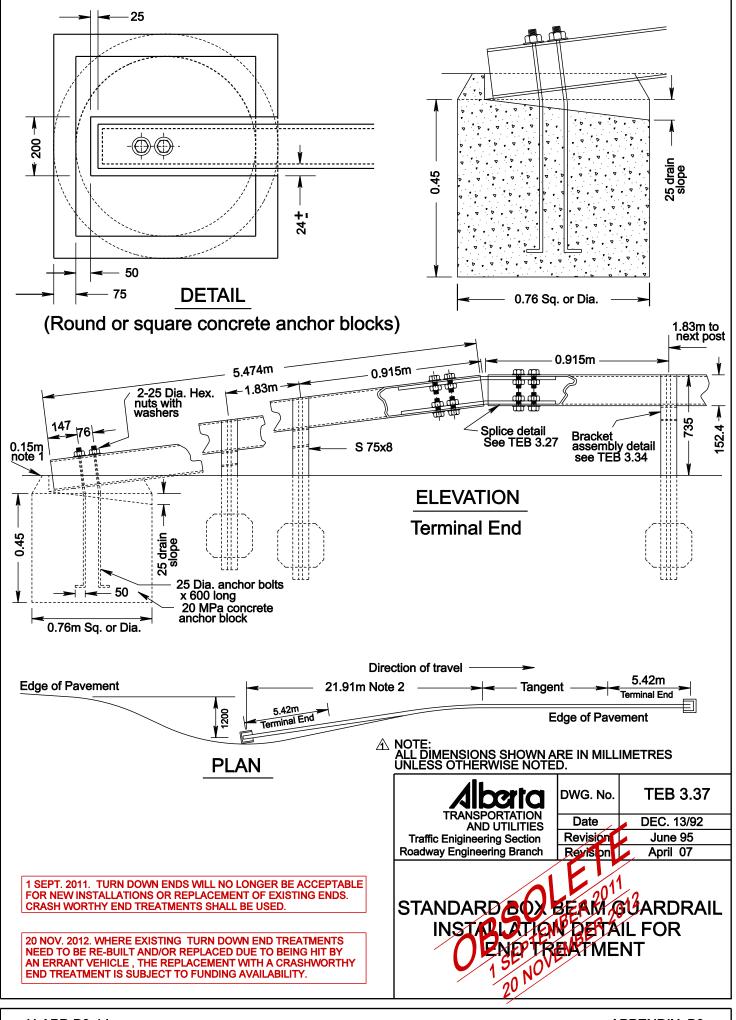


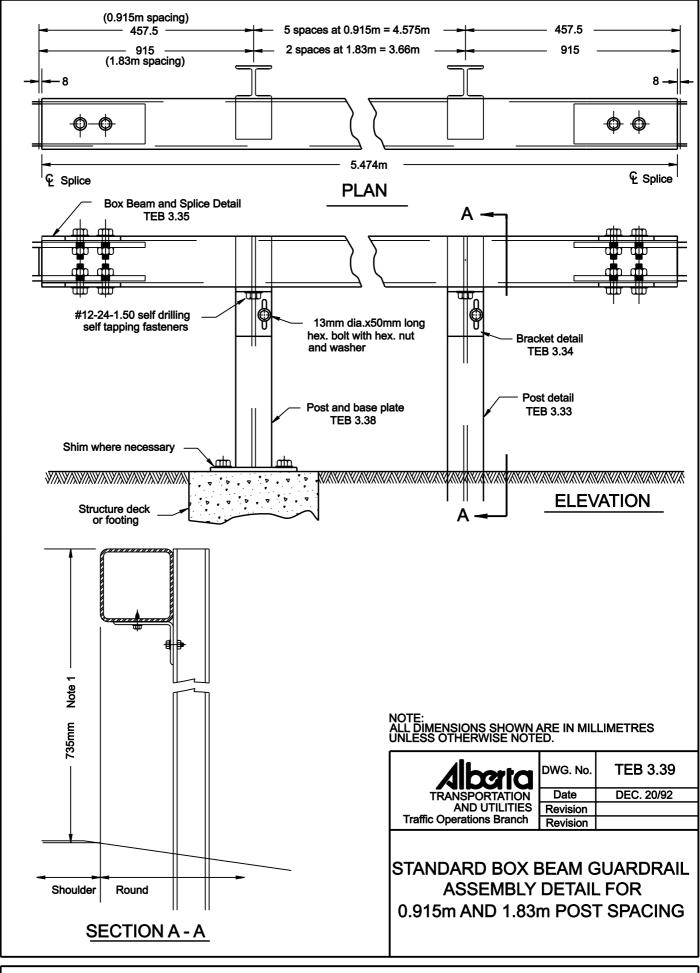


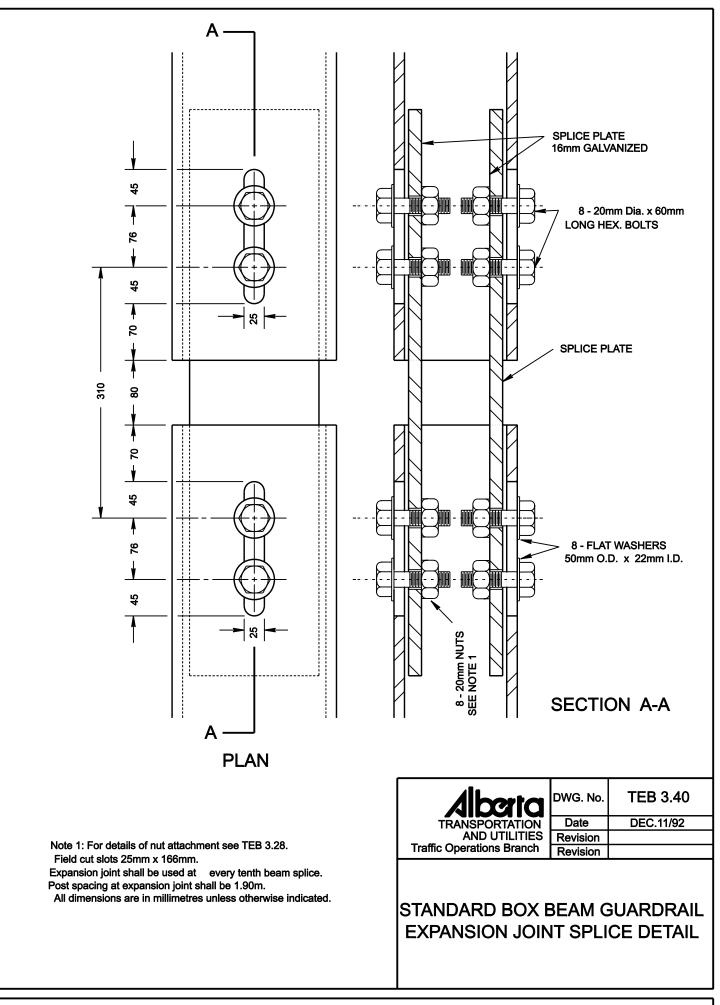
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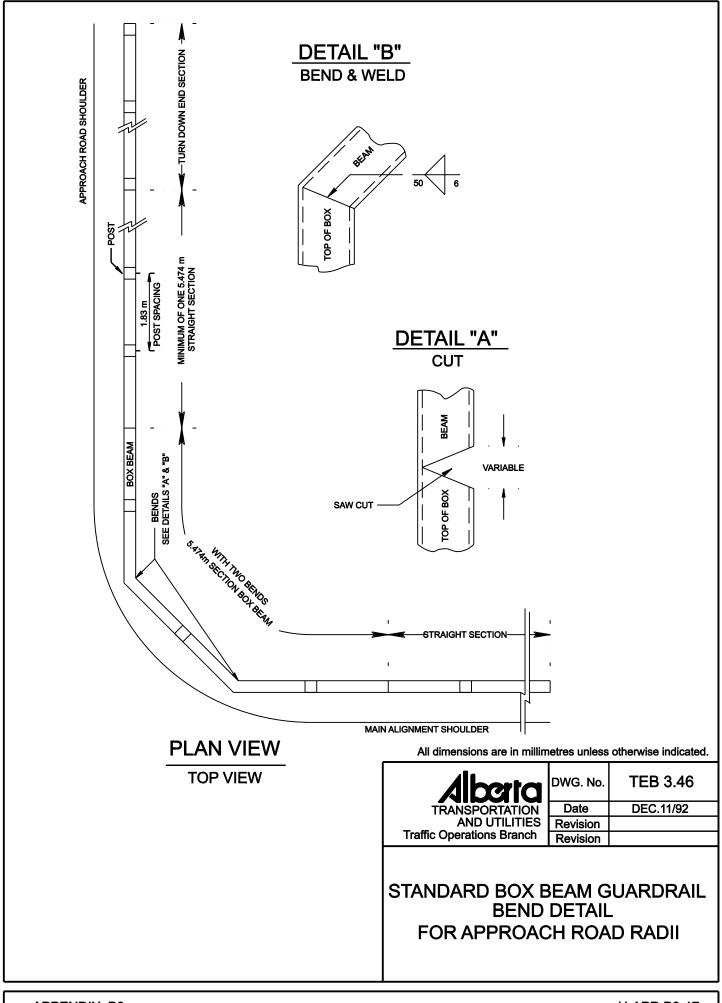
APPENDIX B3











H-APP-B3-17

PRECAST F-SHAPE AND "NJ" SHAPE CONCRETE BARRIERS

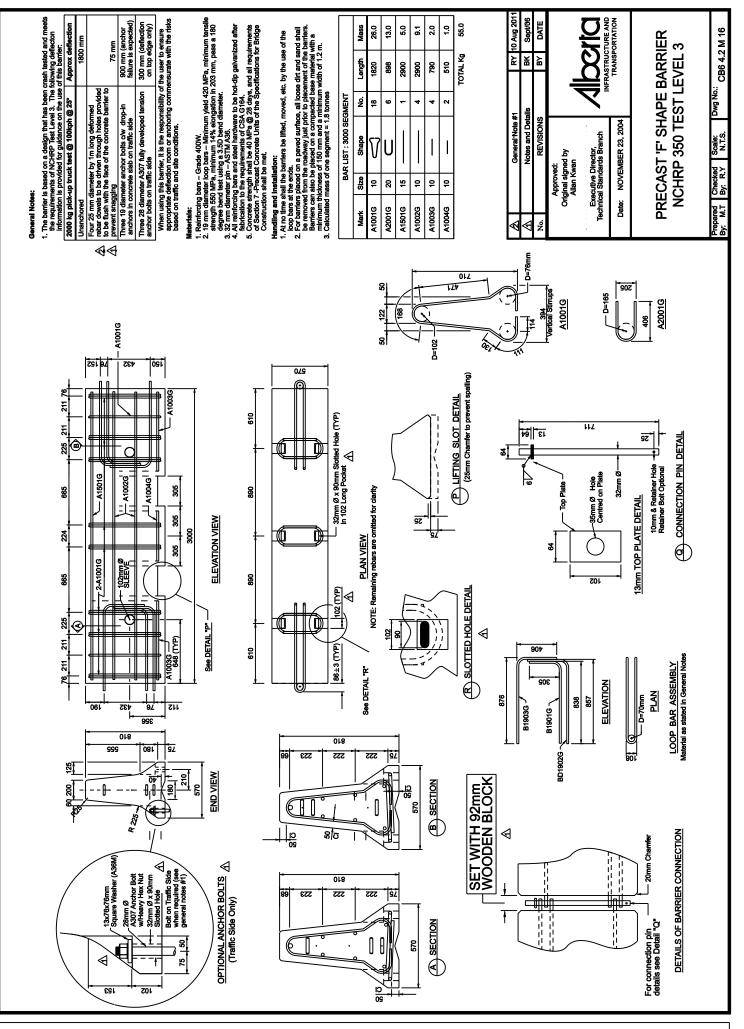
Appendix B4

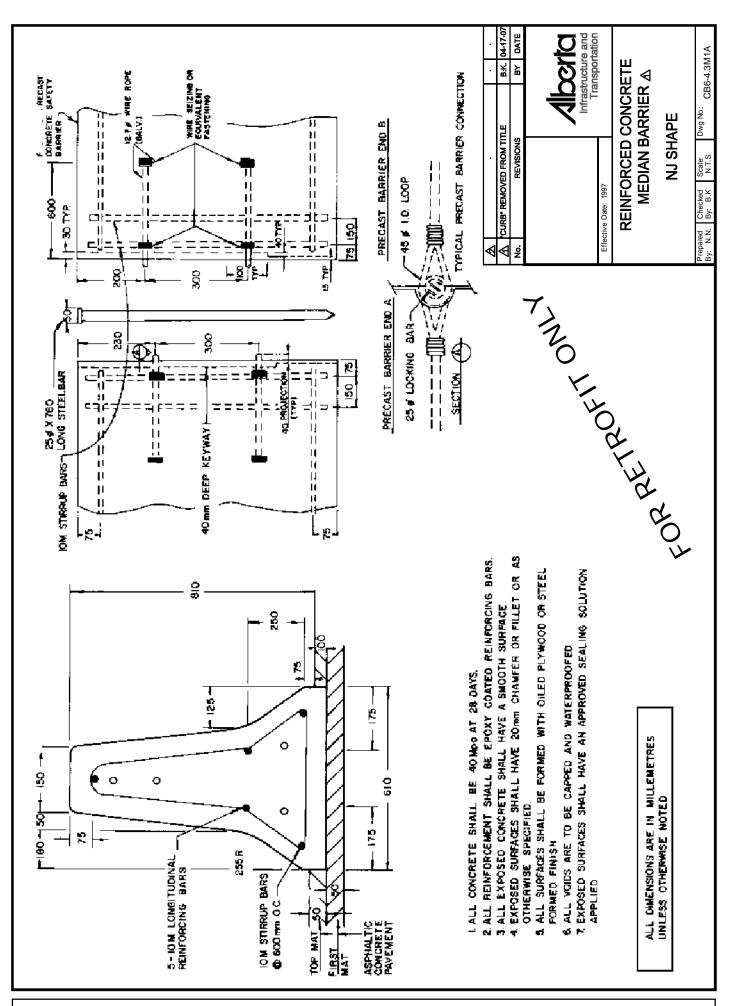
Precast F-Shape and "NJ" Shape Concrete Barriers

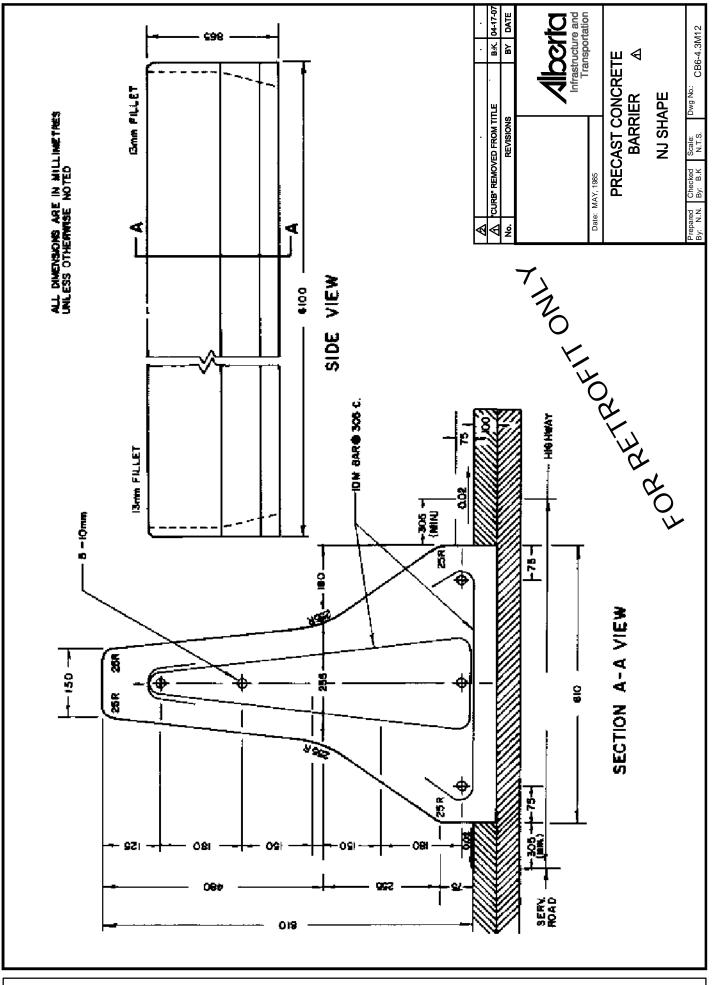
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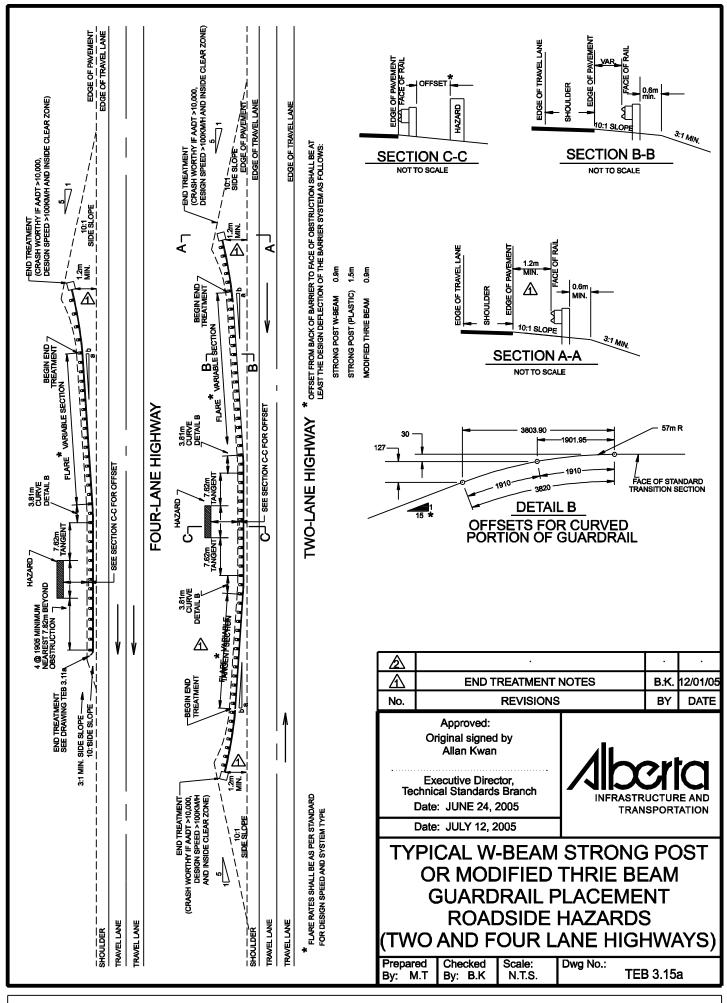


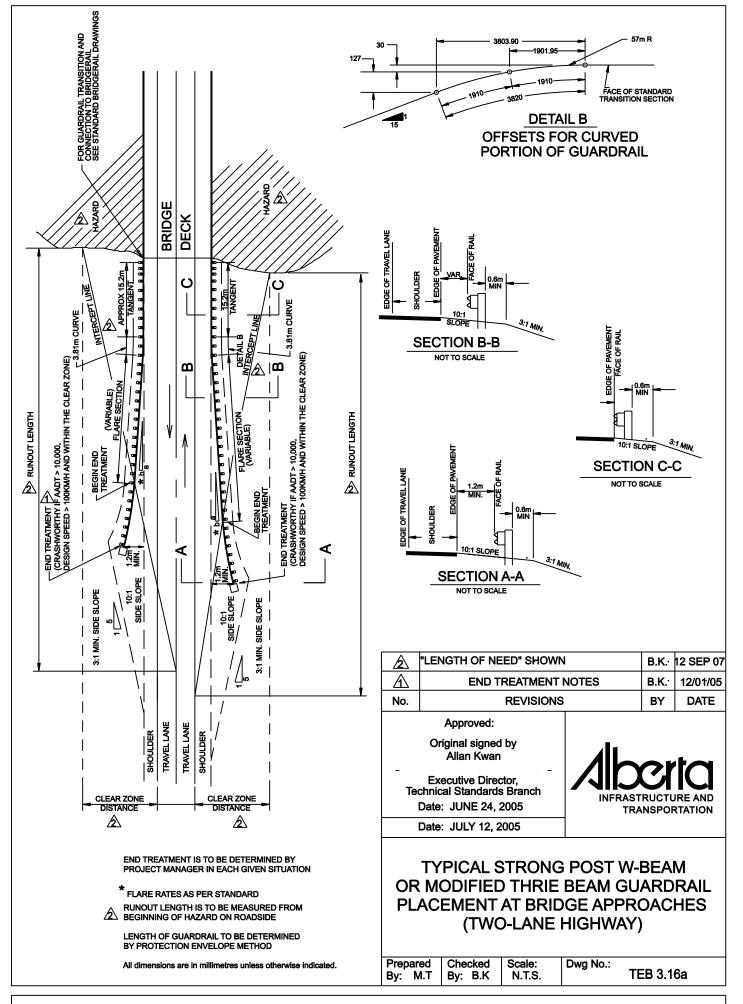
THRIE BEAM GUARDRAIL

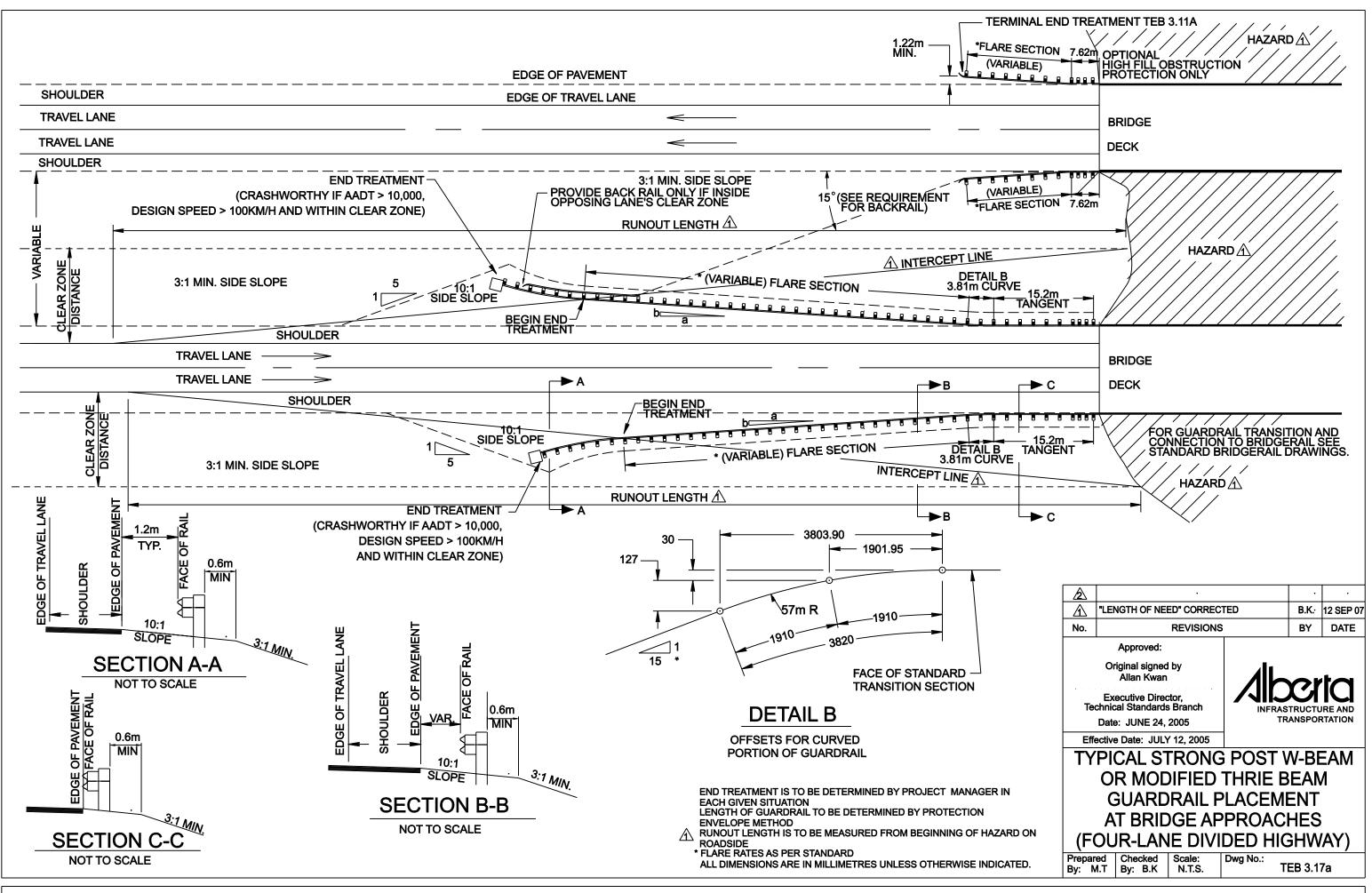
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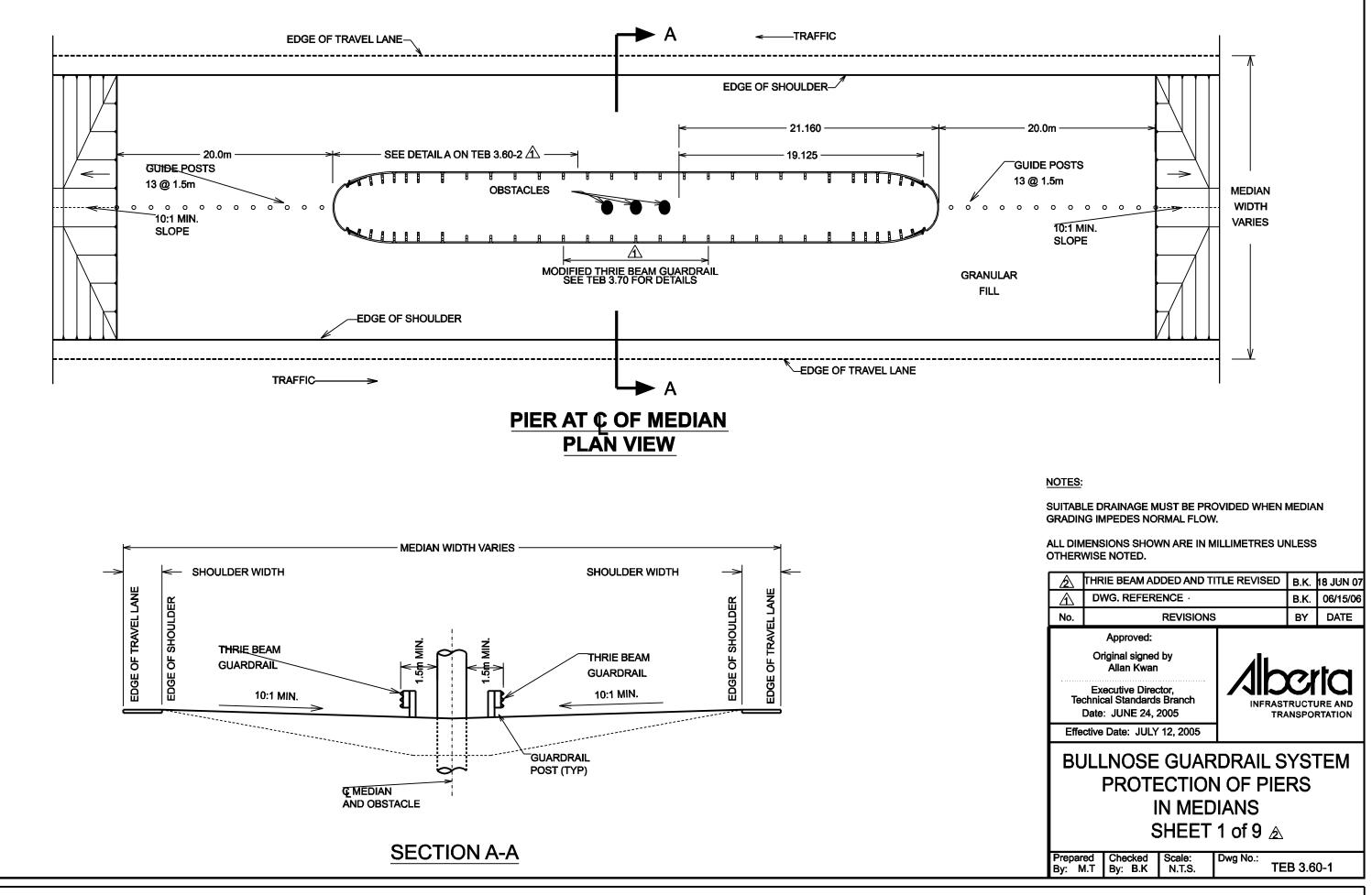


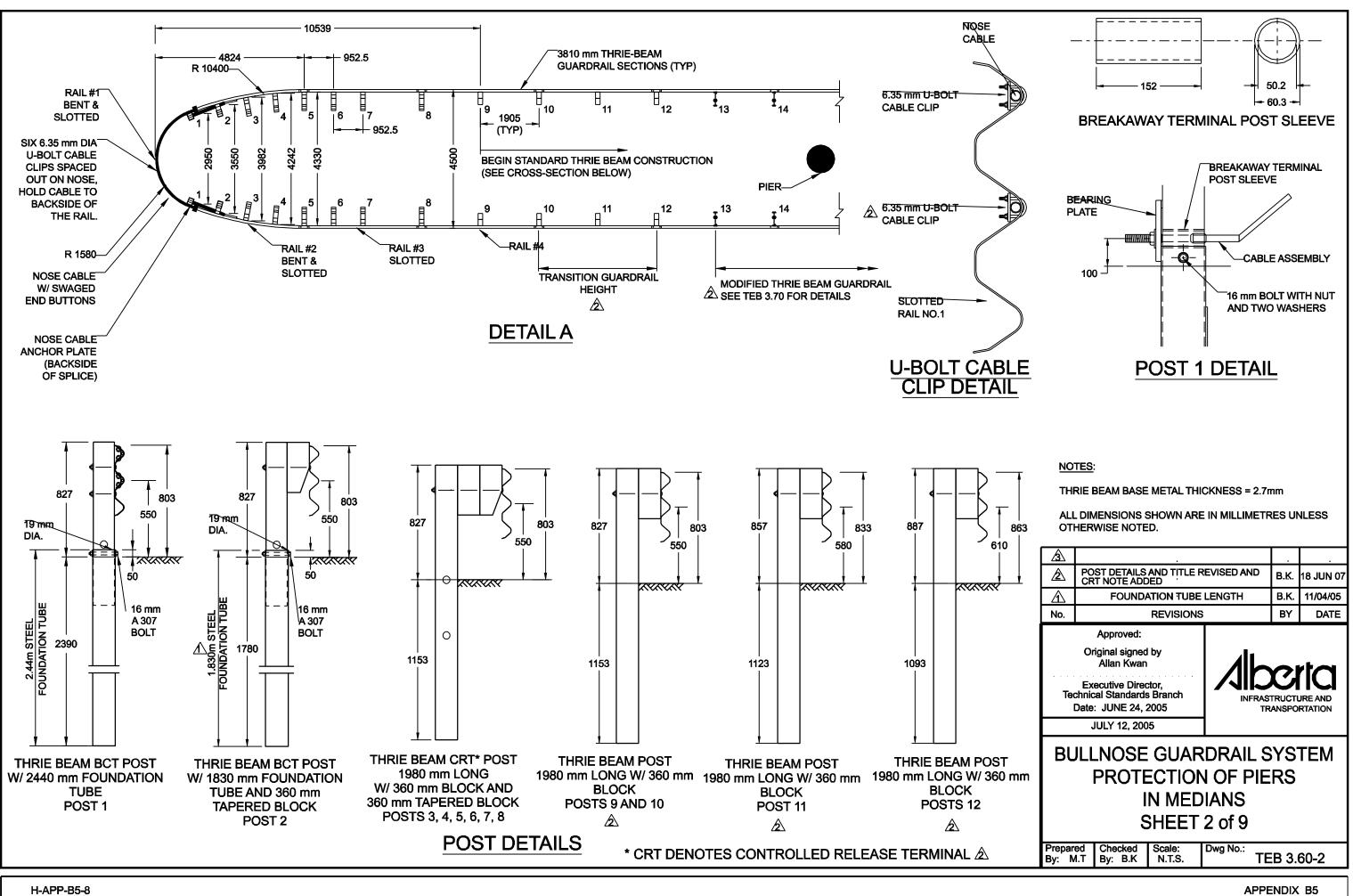


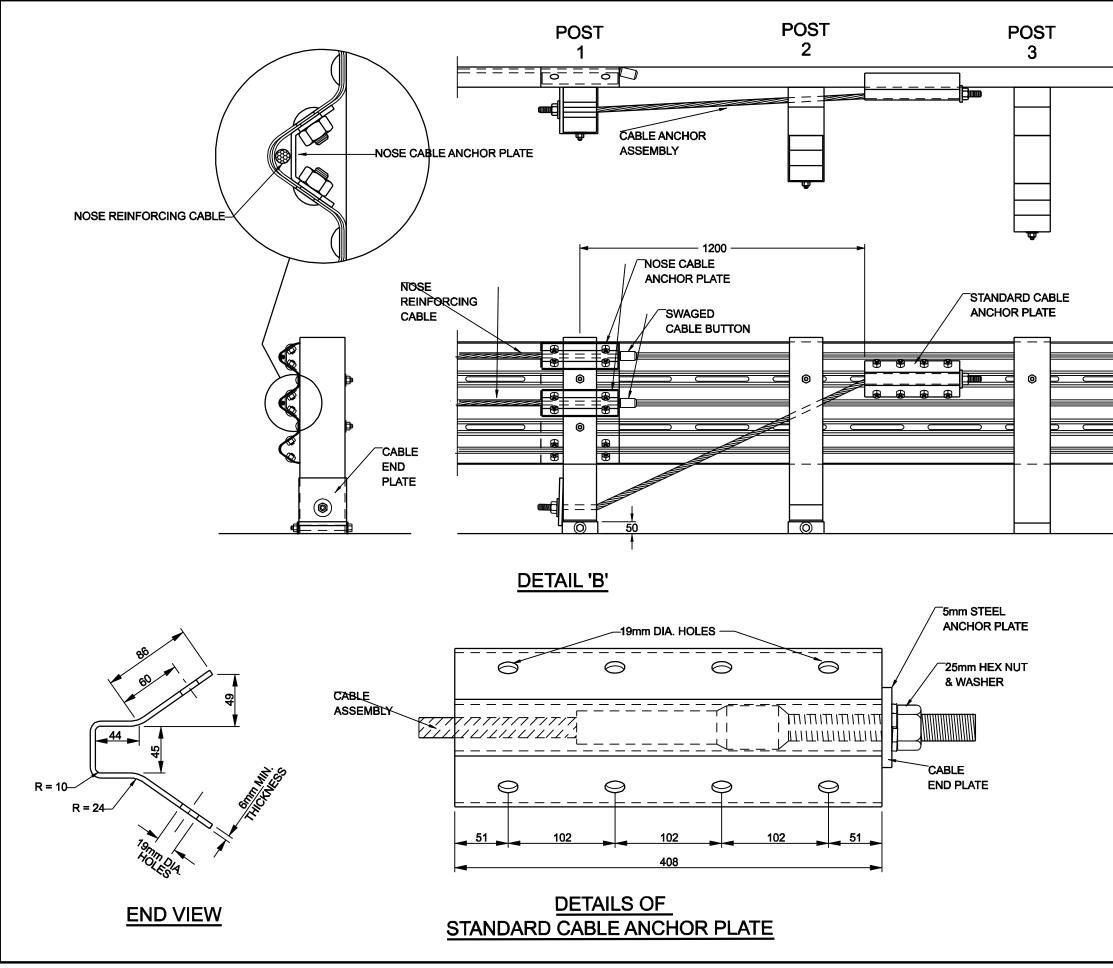


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	IEDIAN BARRIER		
SHOULDER TRAVEL LANE SHOULDER MEDIAN SHOULDER TRAVEL LANE SHOULDER 15° WEDIAN SHOULDER TRAVEL LANE TRAVEL LANE SHOULDER DESIGN SPEED > 1000KM/H AND INSIDE CLEAR ZONE)			
CONTINUOUS M	EDIAN BARRIER		
	a *	∔ - MEDIA ↓ -	- - - - - - - -
	·	•	•
			·
 * FLARE RATE AS PER STANDARD FOR DESIGN SPEED. CONSIDER ONLY WHERE MEDIAN WIDTH SUFFICIENT TO PROVIDE 8m MINIMUM FROM OPPOSING TRAVEL LANE TO BACK SIDE OF TERMINAL. FOR NARROW MEDIAN, IMPACT SYSTEMS ARE REQUIRED. THE LENGTH OF NEED SHALL BE BASED ON THE PROTECTION ENVELOPE. CLEARANCE BETWEEN GUARDRAIL AND OBSTRUCTION: STRONG POST (WOOD AND STEEL POSTS) 0.9m THRIE BEAM 0.9m STRONG POST (PLASTIC POSTS) 1.5m DATA FOR 15m CURVE D=3830' R=225.0m SR=7.508m CL=15.0m END TREATMENT IS TO BE DETERMINED BY PROJECT MANAGER TO EACH GIVEN SITUATION. 	No. REVISIONS Approved: Original signed by Allan Kwan Executive Director, Technical Standards Branch Date: JUNE 24, 2005 JUNE 74, 2005 Effective Date: JULY 12, 2005 INFRASTRU TRANS TYPICAL STRONG POS GUARDRAIL PLACEMEN FOR MEDIAN HAZARD	SPORTA ST E BE NT	TION
All dimensions are in millimetres unless otherwise indicated.	Prepared Checked Scale: Dwg No.: By: M.T By: B.K N.T.S. TEB 3	3.18a	





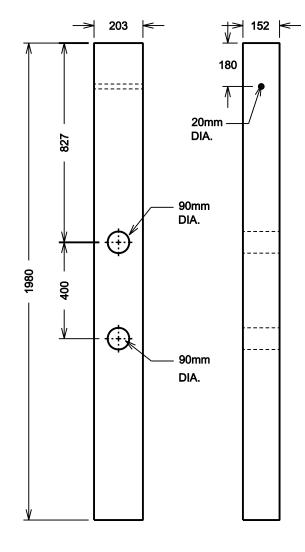


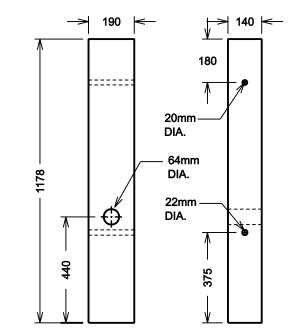


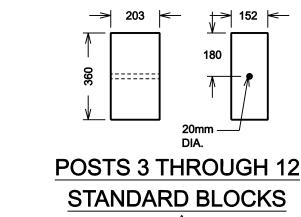
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CABLE ANCHOR SHEET 3 of 9 A						
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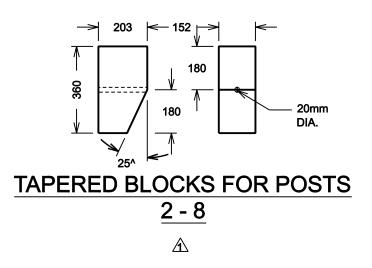




THRIE BEAM ANCHOR POSTS

THRIE BEAM CRT* POSTS

* CRT DENOTES CONTROLLED RELEASE TERMINAL \triangle



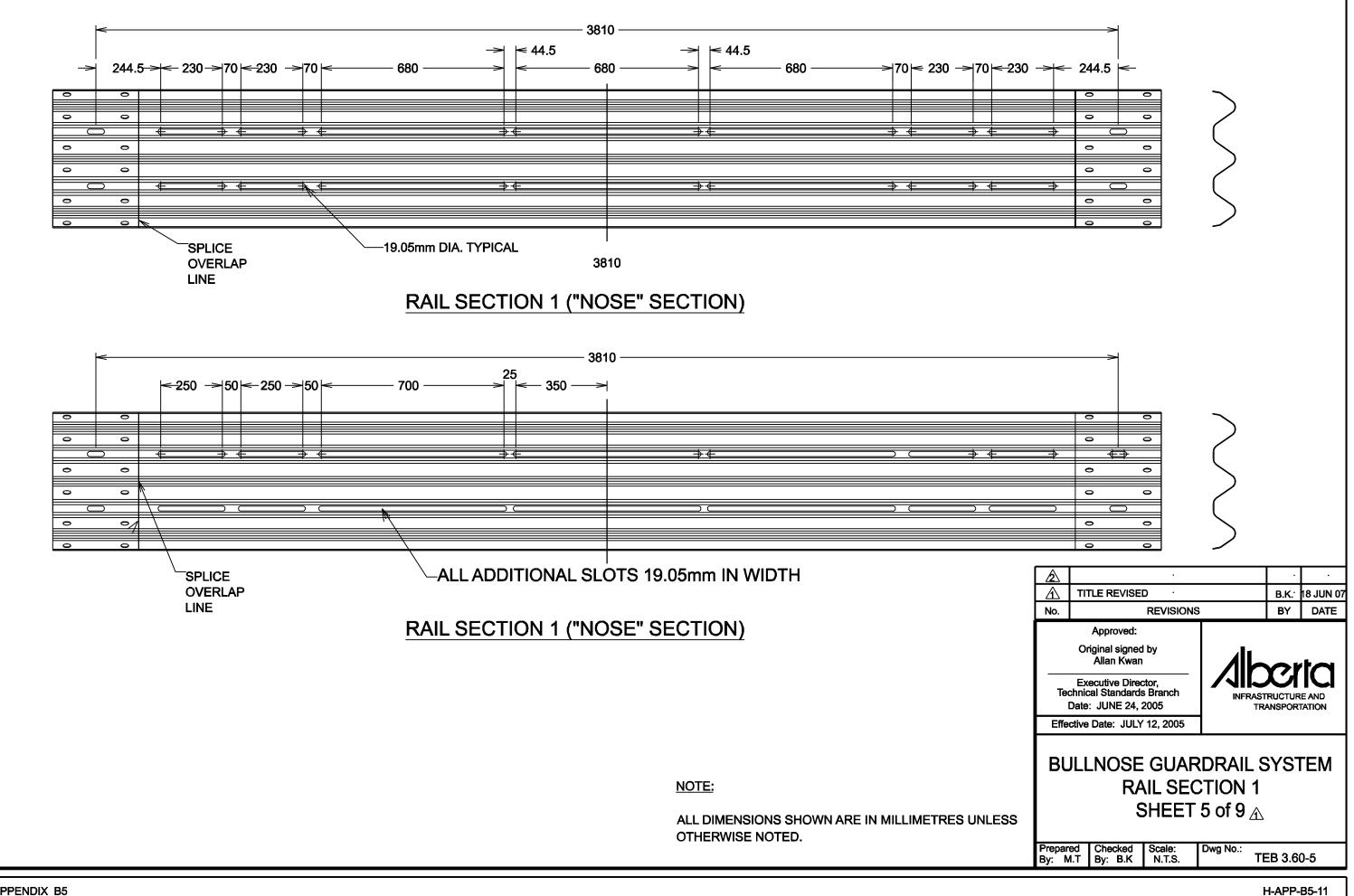
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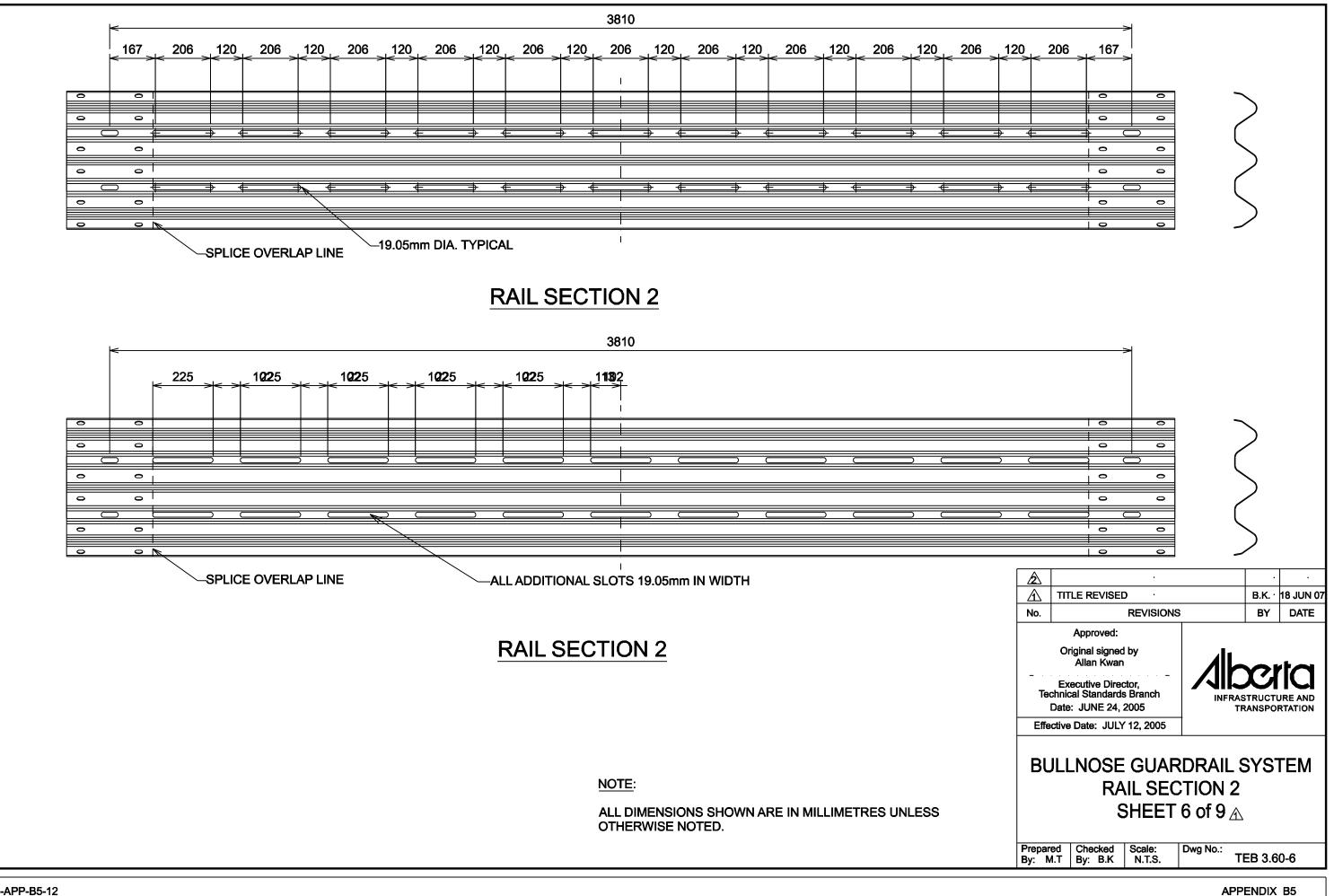
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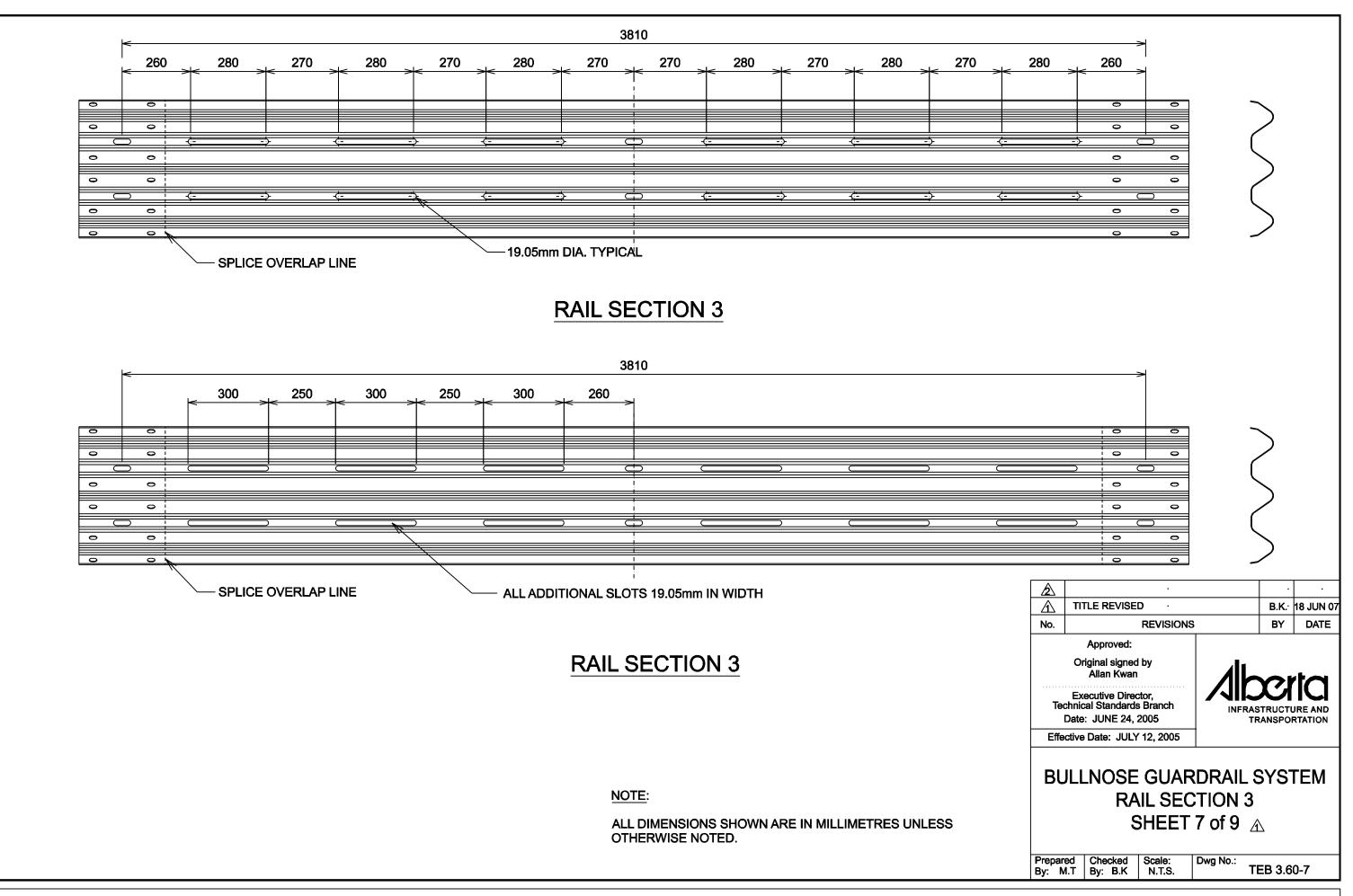
CRT, ANCHOR AND BLOCK POSTS ARE WOOD.

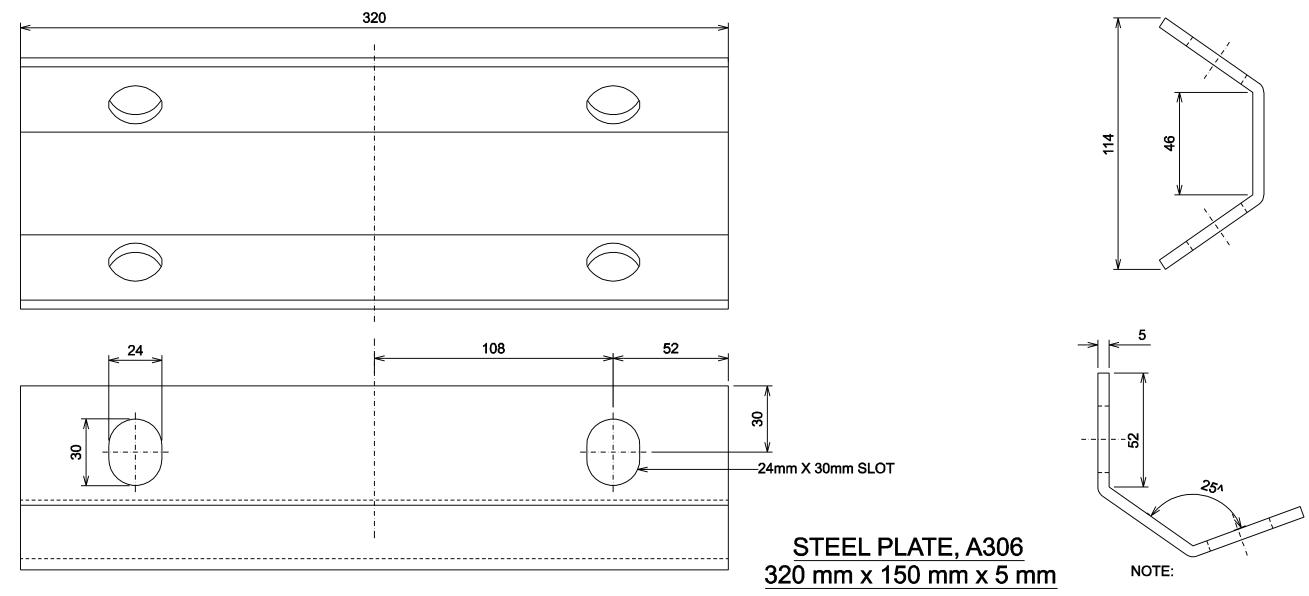
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BULLNOSE GUARDRAIL SYSTEM POSTS & BLOCKS SHEET 4 of 9 A						
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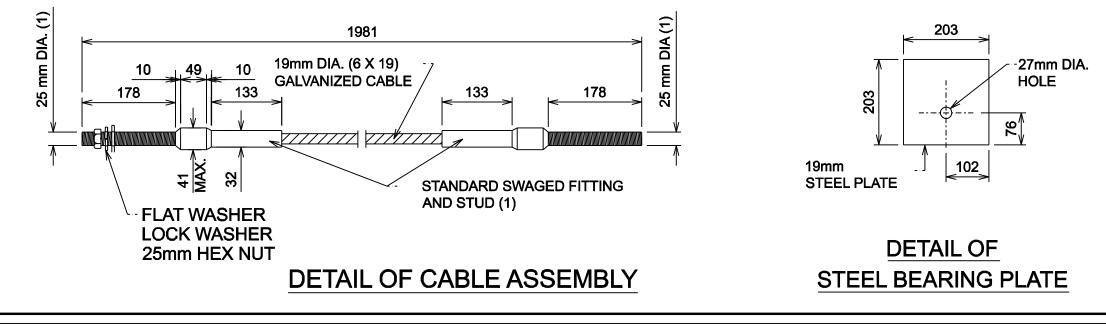






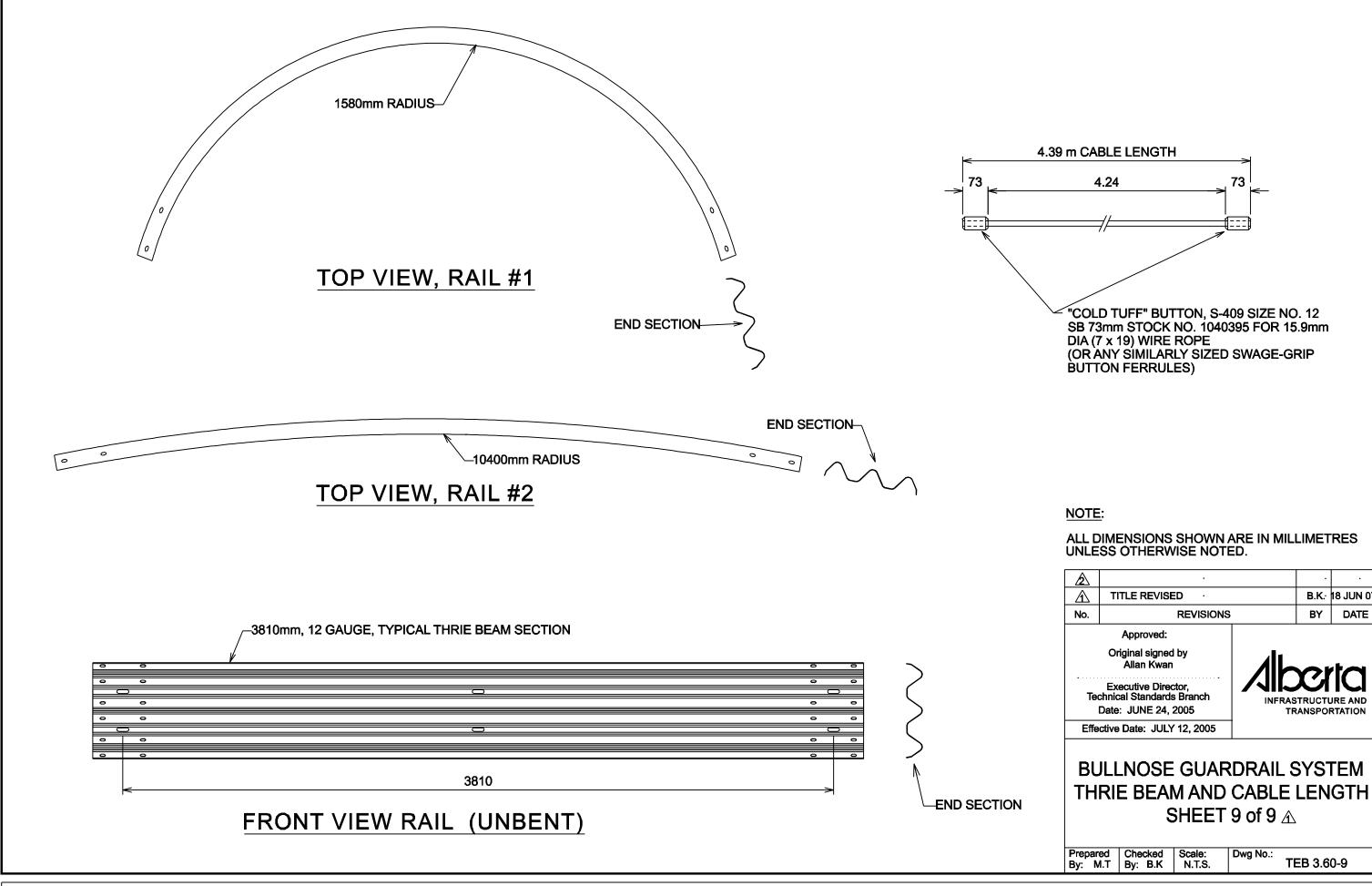


⁽¹⁾ STUD, THREADED ENTIRE LENGTH.



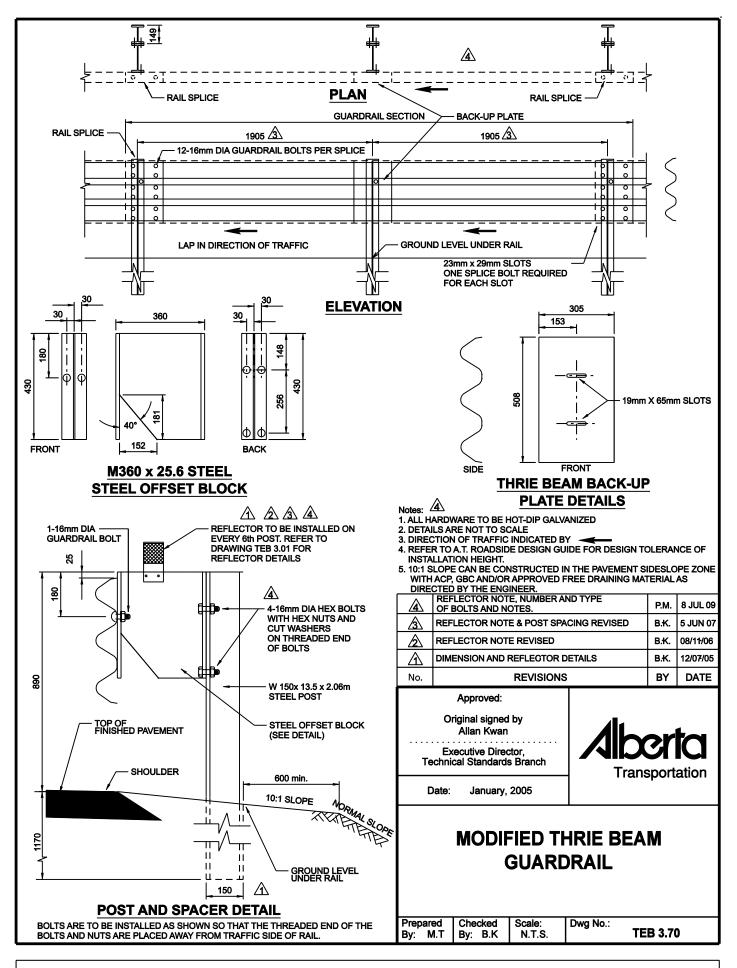
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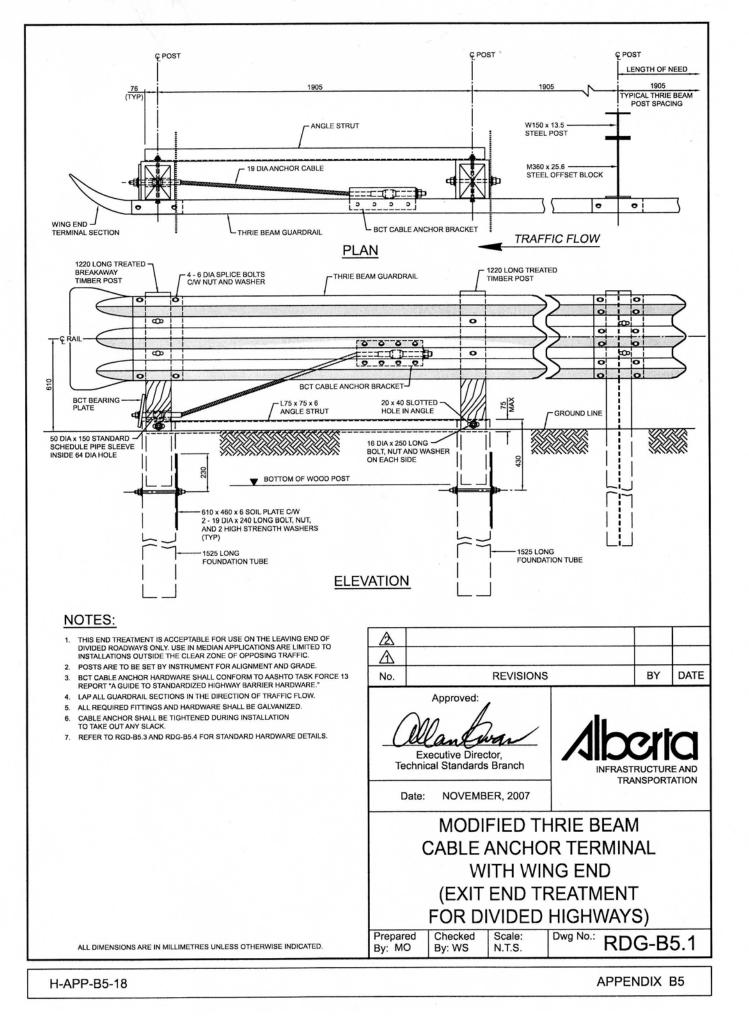
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BULLNOSE GUARDRAIL SYSTEM PLATES AND CABLE ASSEMBLY SHEET 8 of 9 A						
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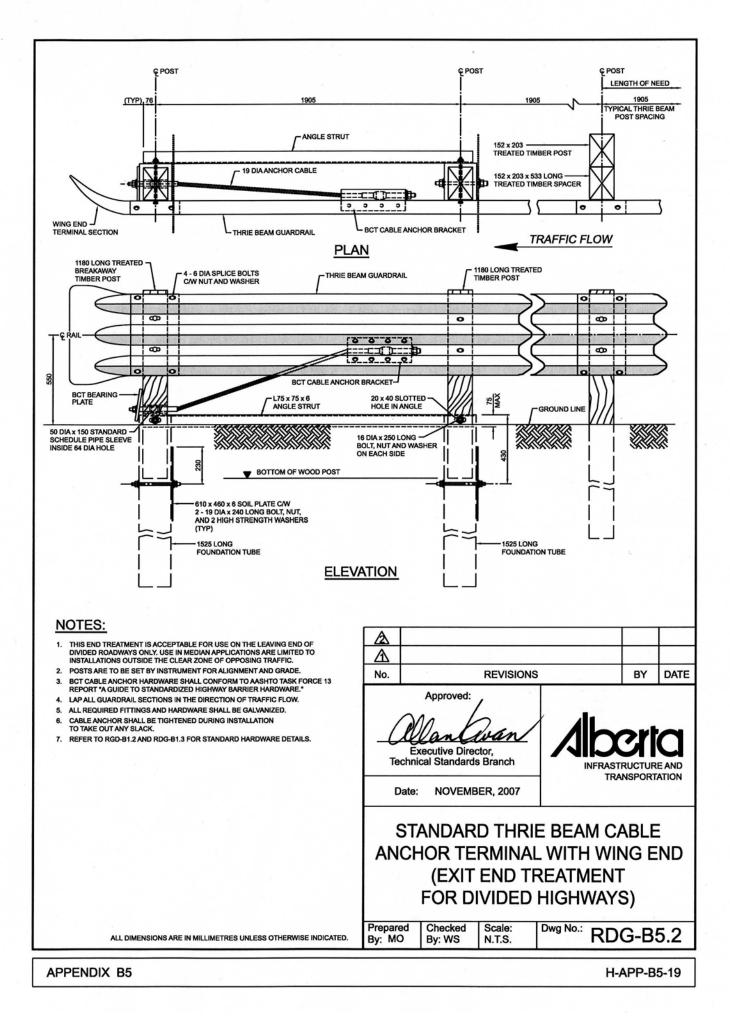


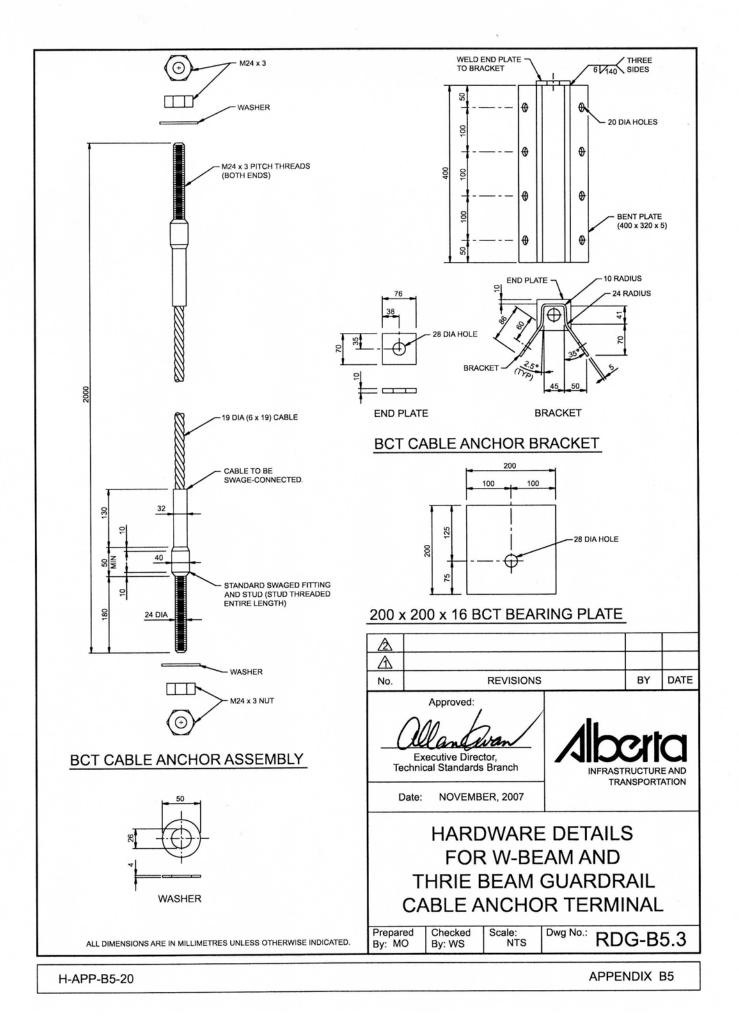
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BULLNOSE GUARDRAIL SYSTEM THRIE BEAM AND CABLE LENGTH SHEET 9 of 9 A						
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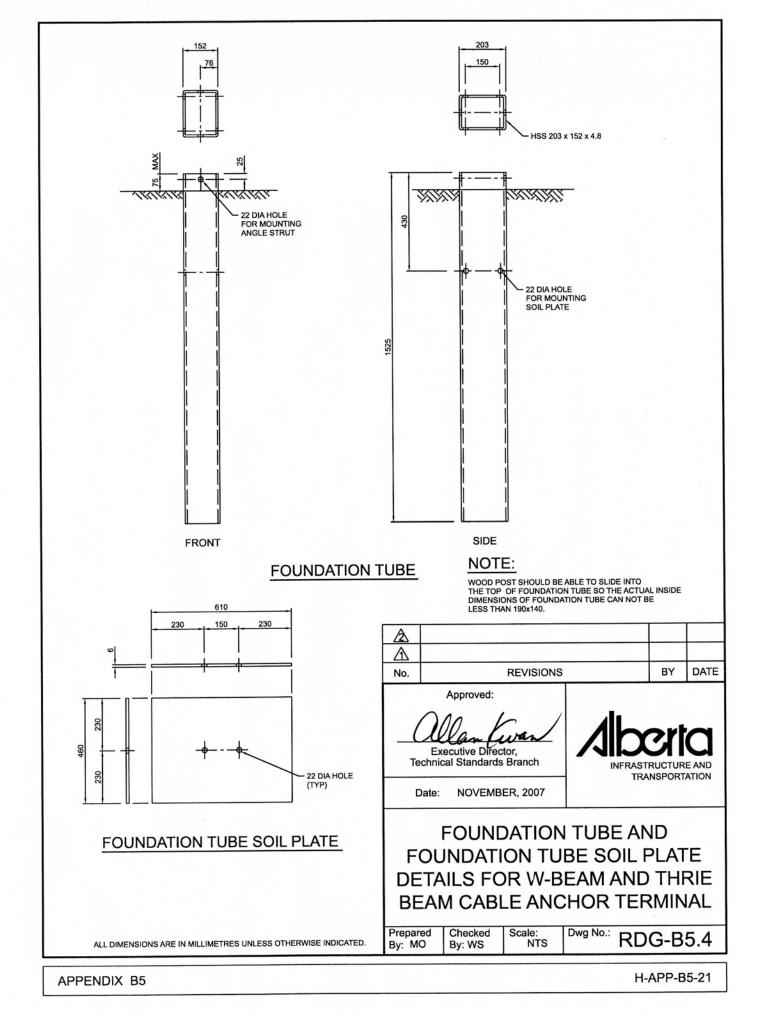
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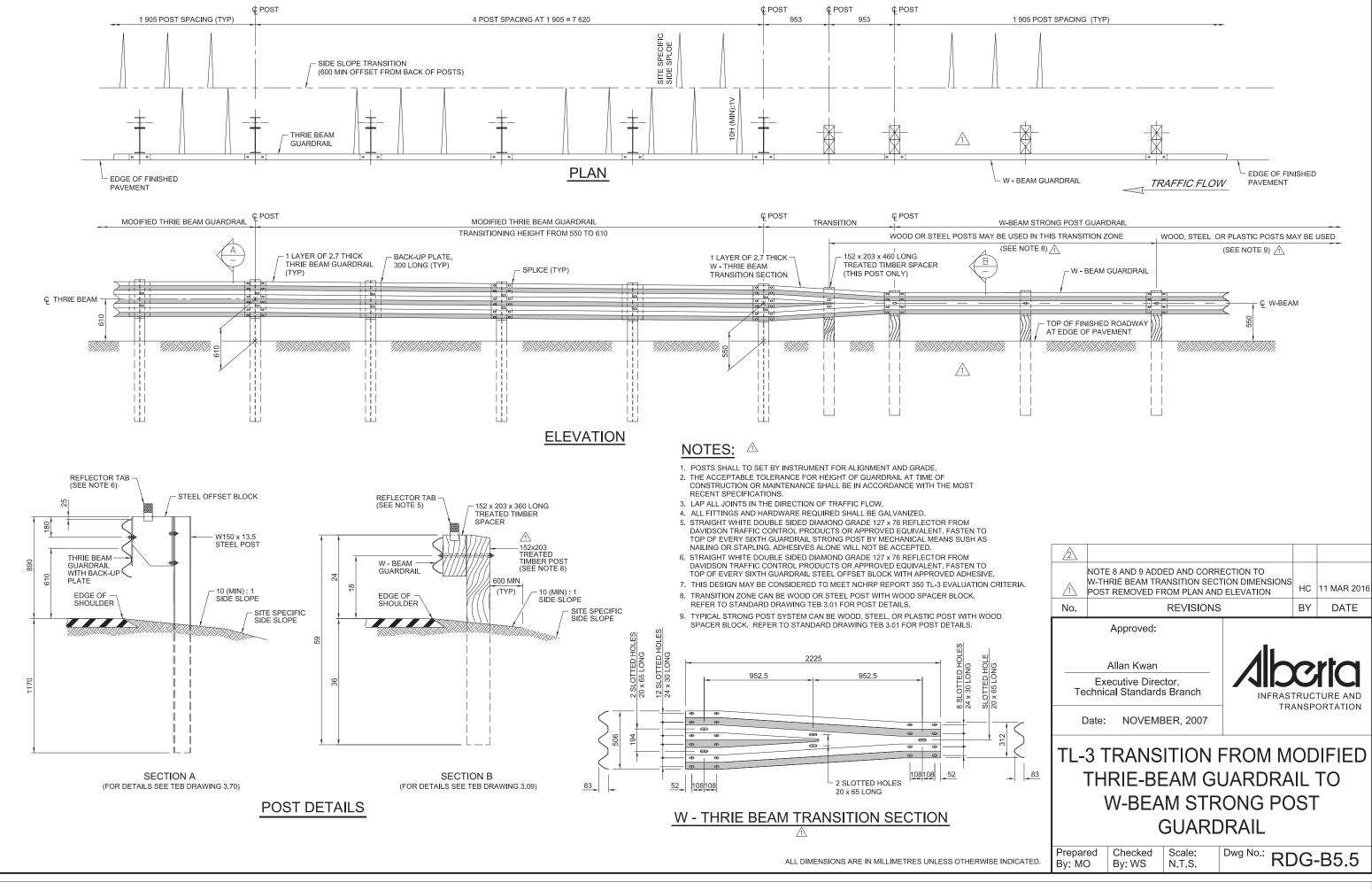




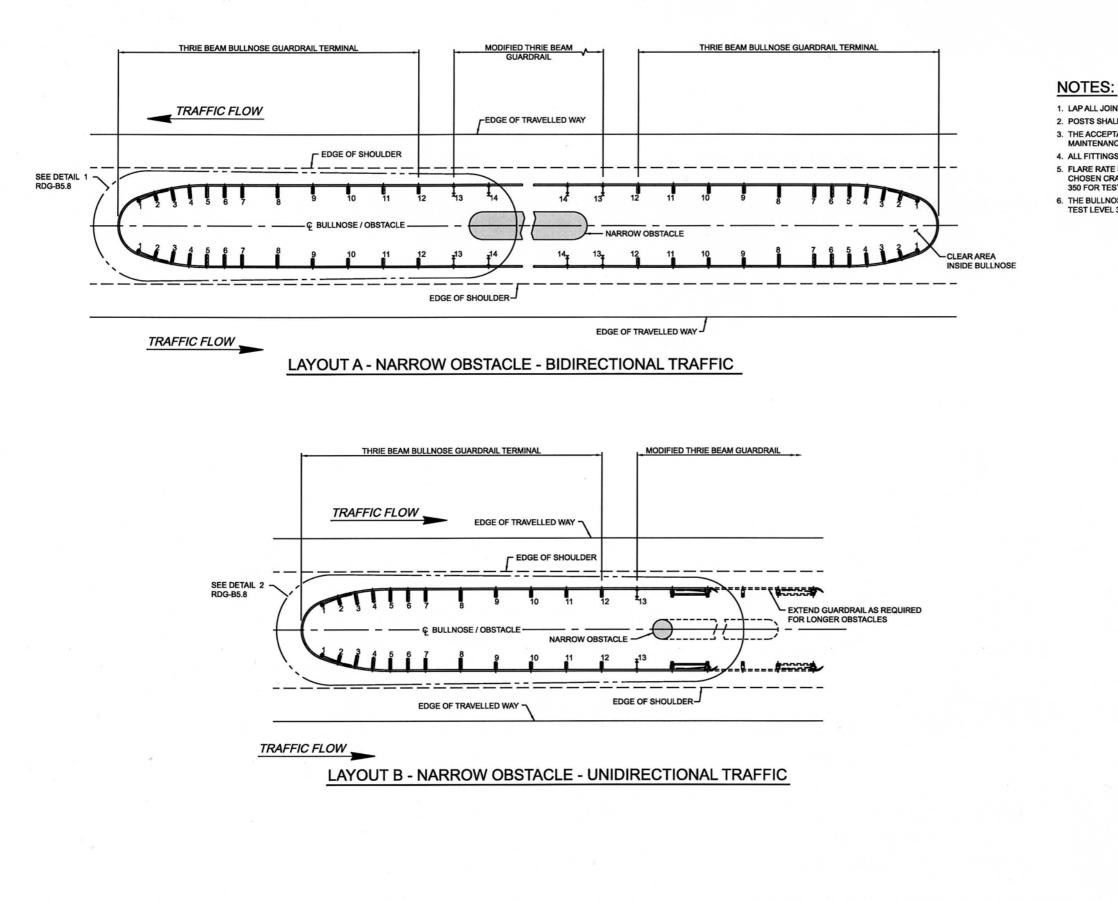




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H-APP-B5-23



H-APP-B5-24

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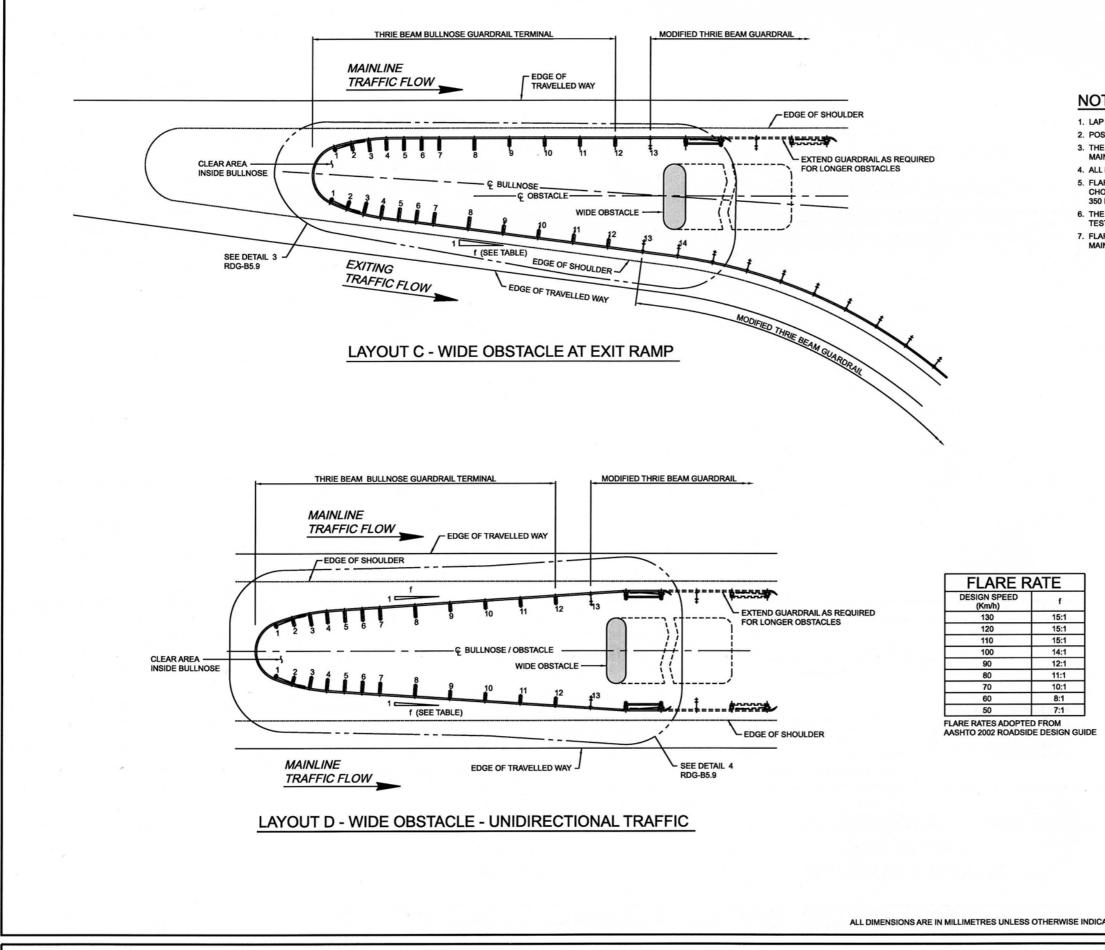
2. POSTS SHALL BE SET BY INSTRUMENT FOR ALIGNMENT AND GRADE.

3. THE ACCEPTABLE TOLERANCE FOR HEIGHT OF GUARDRAIL AT TIME OF CONSTRUCTION OR MAINTENANCE SHALL BE IN ACCORDANCE WITH THE MOST RECENT SPECIFICATIONS. 4. ALL FITTINGS AND HARDWARE SHALL BE GALVANIZED.

5. FLARE RATE SHALL BE SPECIFIED WITHIN THE LIMITS SET BY THE MANUFACTURER OF THE CHOSEN CRASH WORTHY END TERMINAL TO MEET THE REQUIREMENTS OF NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3).

6. THE BULLNOSE GUARDRAIL SYSTEM SATISFIES THE REQUIREMENTS OF NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3) AND IS THEREFORE APPLICABLE FOR USE ON HIGH SPEED ROAD FACILITIES.

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THRIE BEAM BULLNOSE GUARDRAIL GENERAL LAYOUTS											
Prepare By: MC	G-B	5.6									
APPENDIX B5											



NOTES:

1. LAP ALL JOINTS IN THE DIRECTION OF TRAFFIC FLOW.

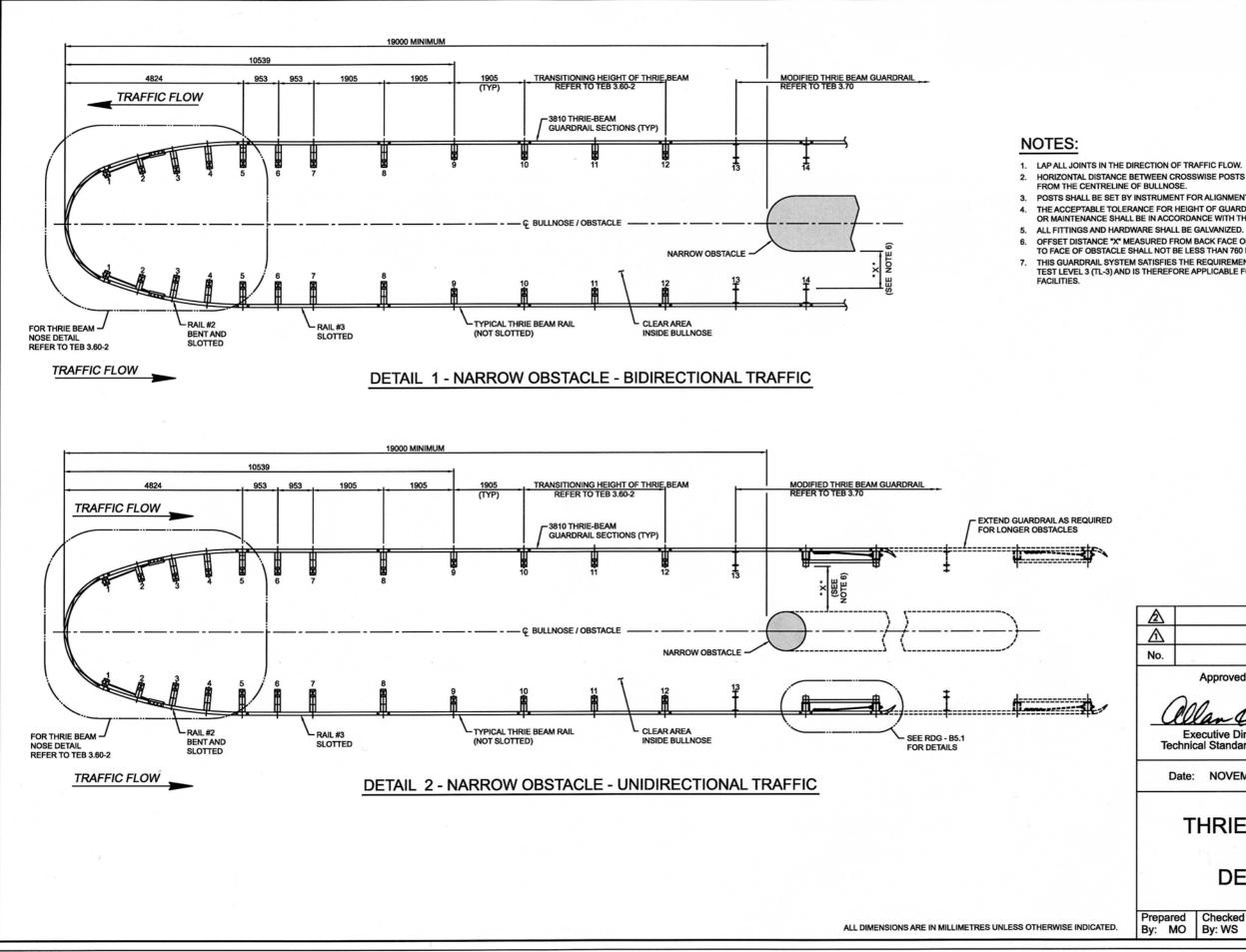
2. POSTS SHALL BE SET BY INSTRUMENT FOR ALIGNMENT AND GRADE.

 THE ACCEPTABLE TOLERANCE FOR HEIGHT OF GUARDRAIL AT TIME OF CONSTRUCTION OR MAINTENANCE SHALL BE IN ACCORDANCE WITH THE MOST RECENT SPECIFICATIONS.
 ALL FITTINGS AND HARDWARE SHALL BE GALVANIZED.

5. FLARE RATE SHALL BE SPECIFIED WITHIN THE LIMITS SET BY THE MANUFACTURER OF THE CHOSEN CRASH WORTHY END TERMINAL TO MEET THE REQUIREMENTS OF NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3).

 THE BULLNOSE GUARDRAIL SYSTEM SATISFIES THE REQUIREMENTS OF NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3) AND IS THEREFORE APPLICABLE FOR USE ON HIGH SPEED ROAD FACILITIES.
 FLARE RATES SHOWN ARE RELATIVE TO OBSTACLE CENTRELINE WHICH IS ALIGNED PARALLEL TO MAINLINE TRAFFIC FLOW. FLARING OF GUARDRAIL BEGINS AT POST 5.

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H-APP-B5-26

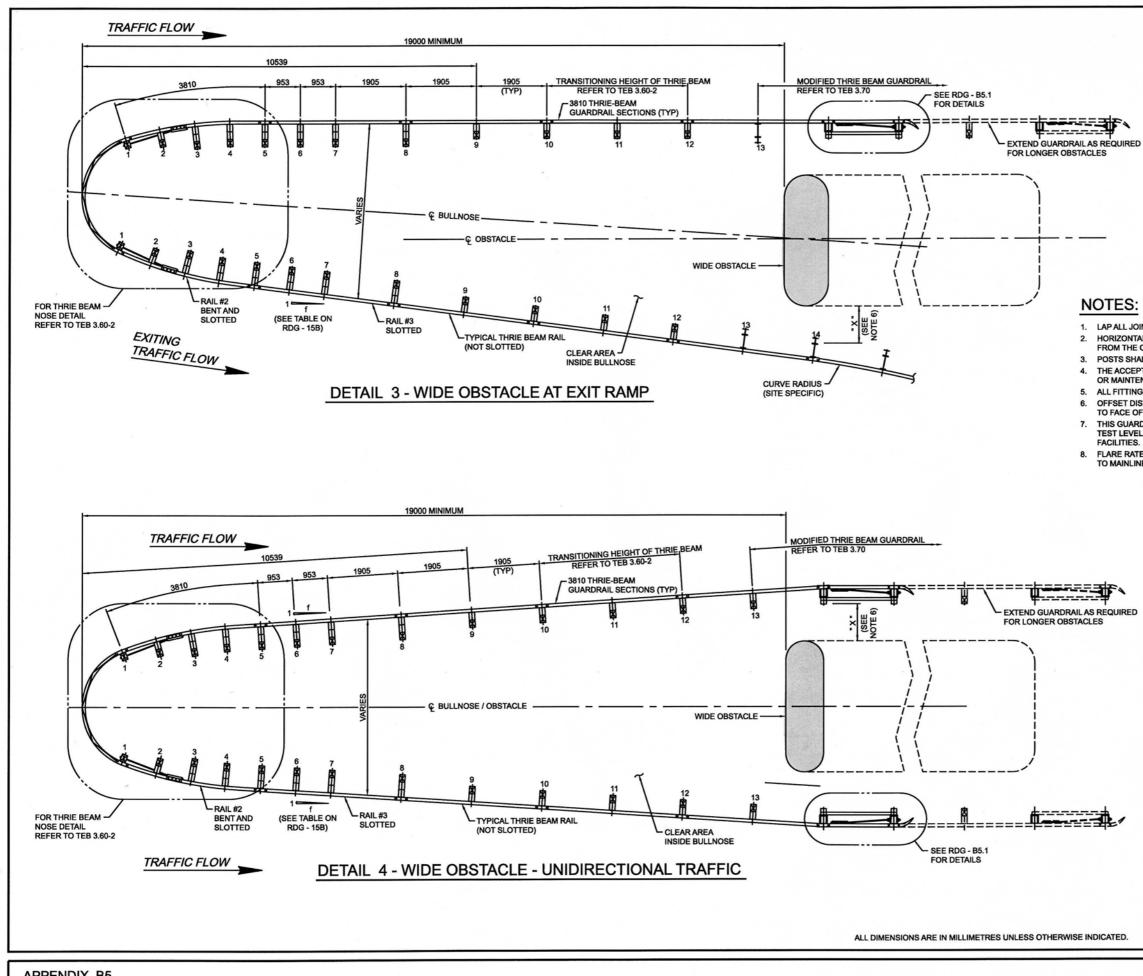
2. HORIZONTAL DISTANCE BETWEEN CROSSWISE POSTS ARE MEASURED PERPENDICULAR

3. POSTS SHALL BE SET BY INSTRUMENT FOR ALIGNMENT AND GRADE.

4. THE ACCEPTABLE TOLERANCE FOR HEIGHT OF GUARDRAIL AT TIME OF CONSTRUCTION OR MAINTENANCE SHALL BE IN ACCORDANCE WITH THE MOST RECENT SPECIFICATIONS.

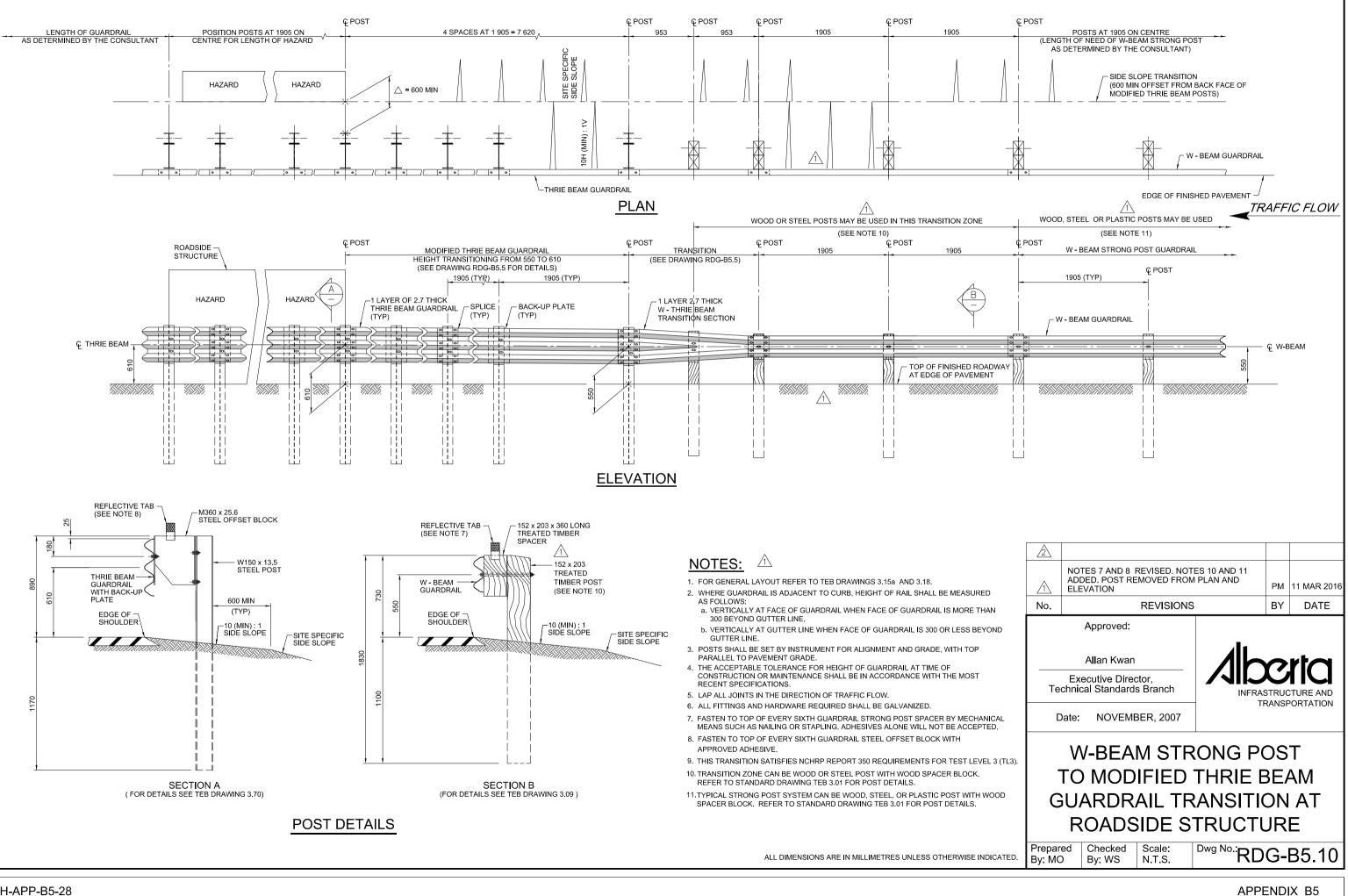
OFFSET DISTANCE "X" MEASURED FROM BACK FACE OF THRIE BEAM GUARDRAIL POST TO FACE OF OBSTACLE SHALL NOT BE LESS THAN 760 HOWEVER 1000 IS PREFERRED. 7. THIS GUARDRAIL SYSTEM SATISFIES THE REQUIREMENTS OF NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3) AND IS THEREFORE APPLICABLE FOR USE ON HIGH SPEED ROAD

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	THRIE BEAM BULLNOSE GUARDRAIL DETAILED PLANS										
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APPENDIX B5											



- LAP ALL JOINTS IN THE DIRECTION OF TRAFFIC FLOW.
- HORIZONTAL DISTANCE BETWEEN CROSSWISE POSTS ARE MEASURED PERPENDICULAR FROM THE CENTRELINE OF BULLNOSE.
- POSTS SHALL BE SET BY INSTRUMENT FOR ALIGNMENT AND GRADE.
- THE ACCEPTABLE TOLERANCE FOR HEIGHT OF GUARDRAIL AT TIME OF CONSTRUCTION OR MAINTENANCE SHALL BE IN ACCORDANCE WITH THE MOST RECENT SPECIFICATIONS. 5. ALL FITTINGS AND HARDWARE SHALL BE GALVANIZED.
- 6. OFFSET DISTANCE "X" MEASURED FROM BACK FACE OF THRIE BEAM GUARDRAIL POST TO FACE OF OBSTACLE SHALL NOT BE LESS THAN 760 HOWEVER 1000 IS PREFERRED. THIS GUARDRAIL SYSTEM SATISFIES THE REQUIREMENTS OF NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3) AND IS THEREFORE APPLICABLE FOR USE ON HIGH SPEED ROAD
- 8. FLARE RATES SHOWN ARE RELATIVE TO OBSTACLE CENTRELINE WHICH IS ALIGNED PARALLEL TO MAINLINE TRAFFIC FLOW. FLARING OF GUARDRAIL BEGINS AT POST 5.

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NDICATED.	Prepare By: M	ed 1O	Checked By: WS	Scale: N.T.S.	Dwg No.: RD	G-B	5.9
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CAST-IN-PLACE OR EXTRUDED F-SHAPE CONCRETE BARRIER AND SINGLE SLOPE CONCRETE BARRIER

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Appendix B6

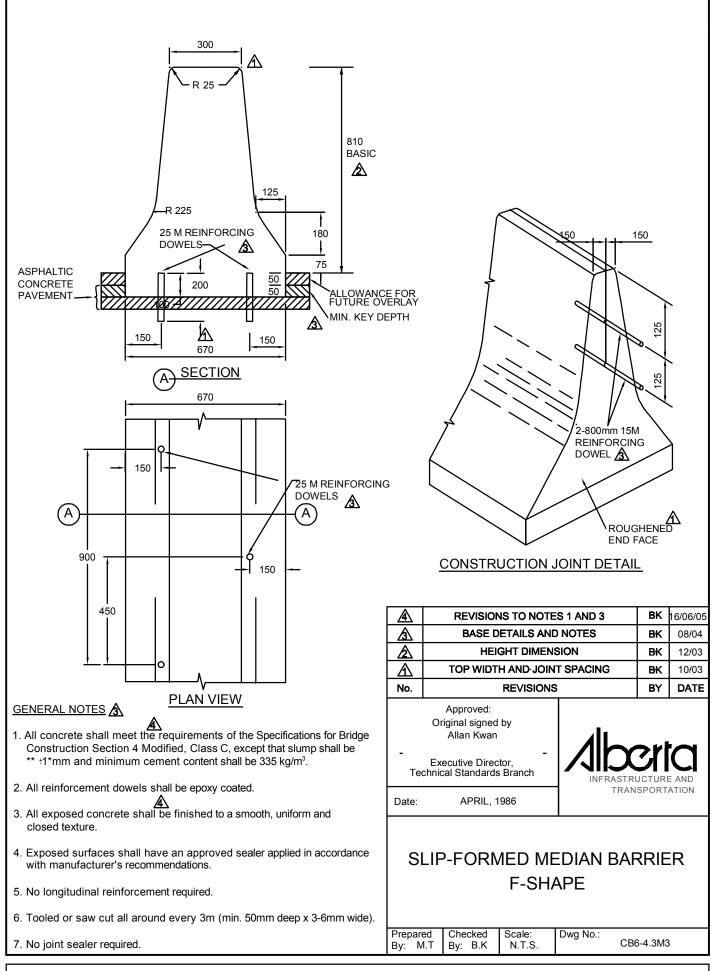
Cast-In Place Or Extruded F-Shape Concrete Barrier and Single Slope Concrete Barrier

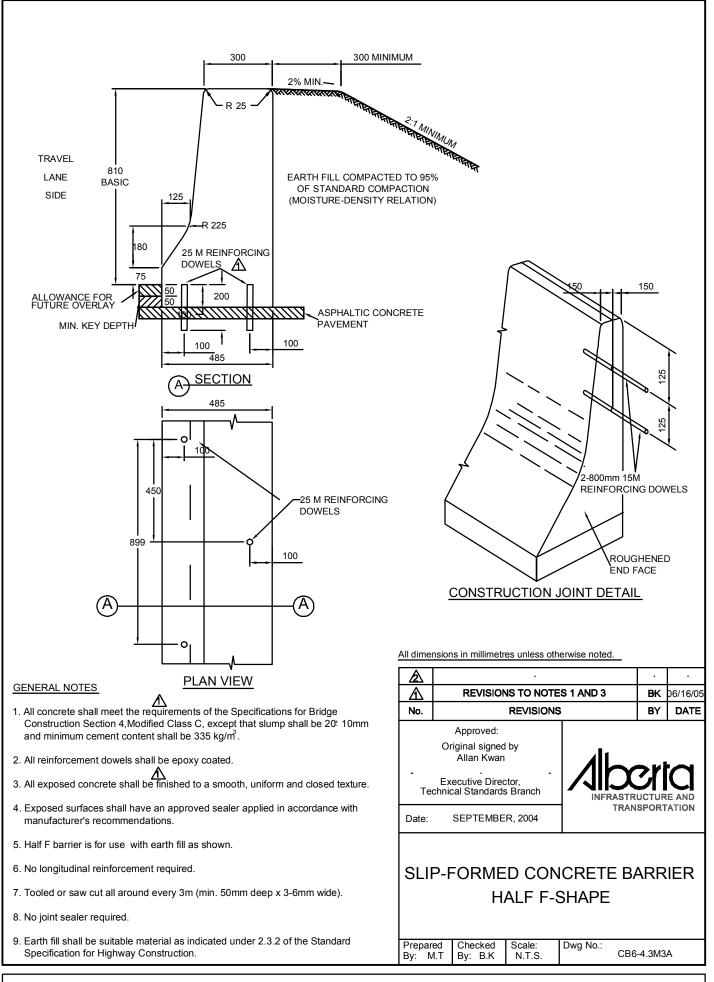
TABLE OF CONTENTS

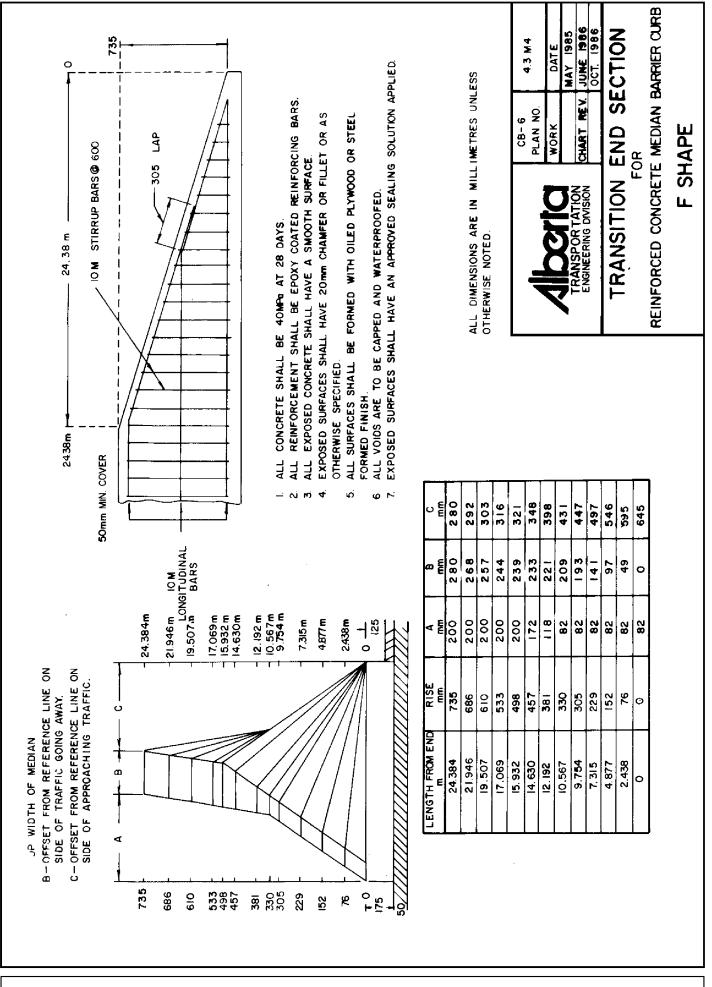
Drw. No.	Drawing Title	Page Number
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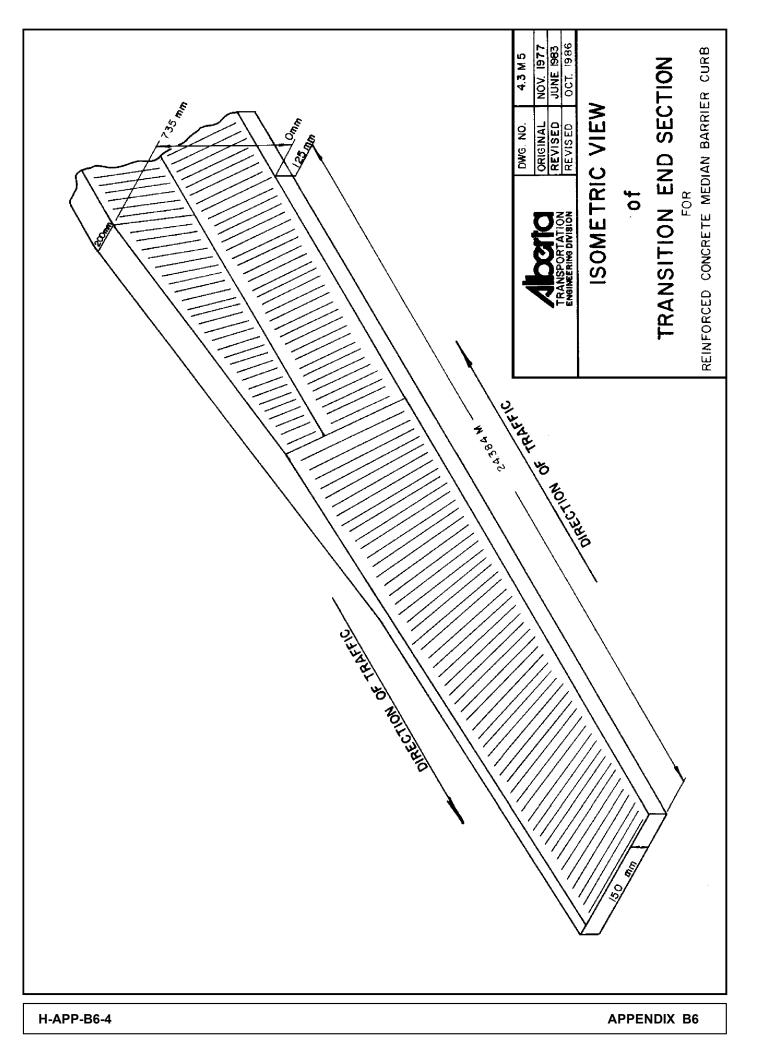
CB6-4.3M3	Slip-Formed Median Barrier F-shape	H-APP-B6-1
CB6-4.3M3A	Slip-formed Concrete Barrier Half F-Shape	H-APP-B6-2
4.3 M4	Transition End Section for Reinforced Concrete Median Barrier Curb F- Shape	H-APP-B6-3
4.3 M5	Isometric View of Transition End Section for Reinforced Concrete Median Barrier Curb	H-APP-B6-4
RDG-B6.1	TL-4 Standard Single Slope Concrete Barrier Details	H-APP-B6-5
RDG-B6.3	TL-4 Single Slope Concrete Median Barrier Transition Around Existing/New Bridge Pier – Sheet 1 of 2	H-APP-B6-7
RDG-B6.4	TL-4 Single Slope Concrete Median Barrier Transition Around Existing Bridge Pier – Sheet 2 of 2	H-APP-B6-8
RDG-B6.5	TL-4 Single Slope Concrete Median Barrier Transition Around New Bridge Pier – Sheet 1 of 3	H-APP-B6-9
RDG-B6.6	TL-4 Single Slope Concrete Median Barrier Transition Around New Bridge Pier – Sheet 2 of 3	H-APP-B6-10
RDG-B6.7	TL-4 Single Slope Concrete Median Barrier Transition Around Existing/New Bridge Pier – Sheet 3 of 3	H-APP-B6-11
RDG-B6.8	TL-2 and TL-3 Concrete Median Barrier Termination TRACC Crash Cushion System (Bidirectional)	Н-АРР-В6-12
RDG-B6.9	TL-2 and TL-3 Concrete Median Barrier Termination Quadguard Crash Cushion System (Bidirectional)	Н-АРР-В6-13
RDG-B6.10	TL-2 and TL-3 Unidirectional Quadguard Crash Cushion System for Wide Median Hazards	H-APP-B6-14
RDG-B6.11	Quadguard and TRACC Crash Cushion Systems Concrete Pad Foundation	H-APP-B6-15
RDG-B6.12	Transition of TL-4 Single Slope Concrete Barrier to W-Beam Median Guardrail – Sheet 1 of 2	H-APP-B6-16
RDG-B6.13	Transition of TL-4 Single Slope Concrete Barrier to W-Beam Median Guardrail – Sheet 2 of 2	H-APP-B6-17
RDG-B6.14	TL-4 Single Slope Concrete Barrier Transition to PL-2 Standard Bridge Concrete Barrier	H-APP-B6-18
RDG-B6.15	Transition of W-Beam Guardrail to TL-4 Single Slope Concrete Roadside Barrier	H-APP-B6-19

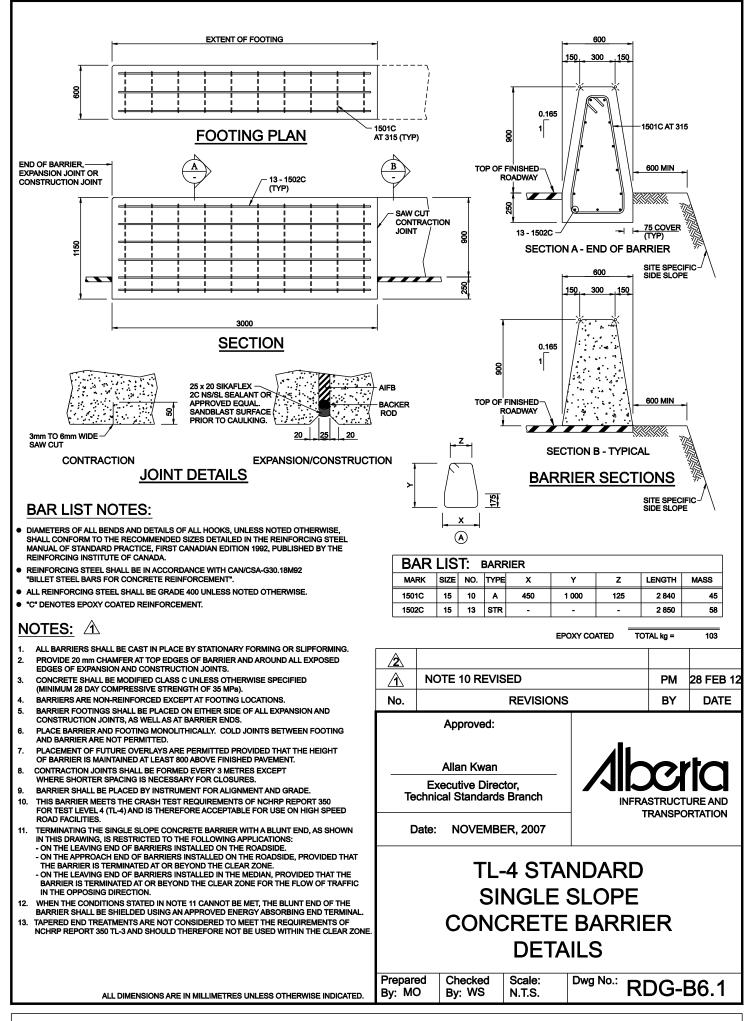
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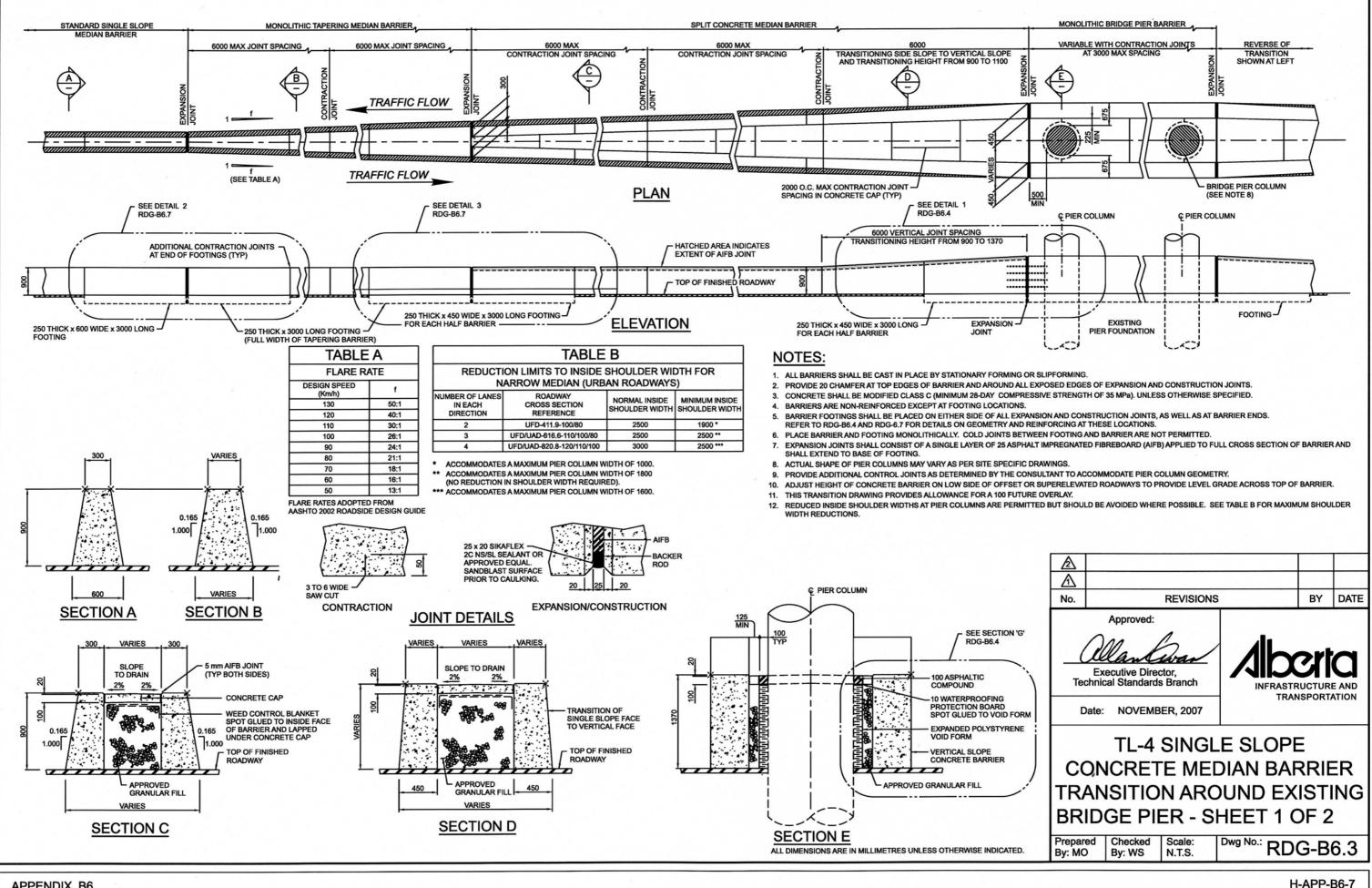


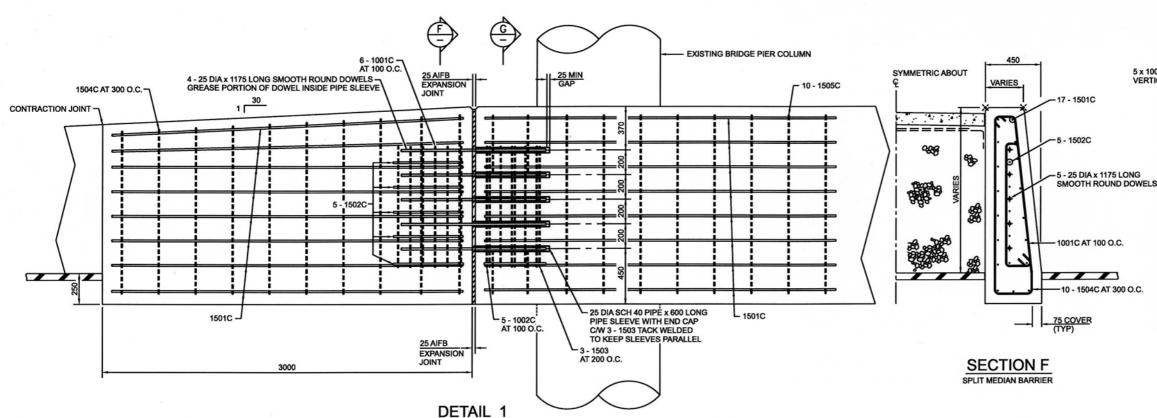






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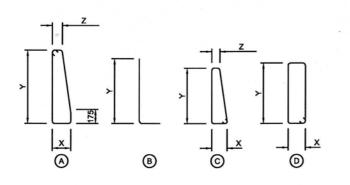


BAR LIST: BARRIER											
MARK	SIZE	NO.	TYPE	x	Y	z	LENGTH	MASS			
1001C	10	6	С	220	750	95	2 025	10			
1002C	10	5	D	220	750		2 140	8			
1501C	15	34	STR				2 850	152			
1501C	15	6	-				550	152			
		-	STR					-			
1503	15	3	В		750		930	4			
1504C	15	10	A	IN INCREMEN Z = VARIES F				49			
1505C	15	10	D	300	1 200		2 780	44			

TOTAL kg = PLAIN EPOXY COATED

TOTAL kg =

268



BAR LIST NOTES:

- DIAMETERS OF ALL BENDS AND DETAILS OF ALL HOOKS, UNLESS NOTED OTHERWISE, SHALL CONFORM TO THE RECOMMENDED SIZES DETAILED IN THE REINFORCING STEEL MANUAL OF STANDARD PRACTICE, FIRST CANADIAN EDITION 1992, PUBLISHED BY THE REINFORCING INSTITUTE OF CANADA.
- REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CAN/CSA-G30.18M92 "BILLET STEEL BARS FOR CONCRETE REINFORCEMENT"
- ALL REINFORCING STEEL SHALL BE GRADE 400 UNLESS NOTED OTHERWISE.
- C DENOTES EPOXY COATED REINFORCEMENT.
- SMOOTH ROUND DOWELS SHALL BE ASTM GRADE A36, OR APPROVED EQUAL, WITH A MINIMUM YIELD STRENGTH OF 250 MPa.
- ALL CONCRETE SHALL BE MODIFIED CLASS C (MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 35 MPa) UNLESS OTHERWISE SPECIFIED.
- ALL CONCRETE CORNERS SHALL HAVE A 20 CHAMFER OR FILLET UNLESS NOTED OTHERWISE
- ALL REINFORCING STEEL SHALL HAVE 75 CLEAR COVER UNLESS NOTED OTHERWISE.

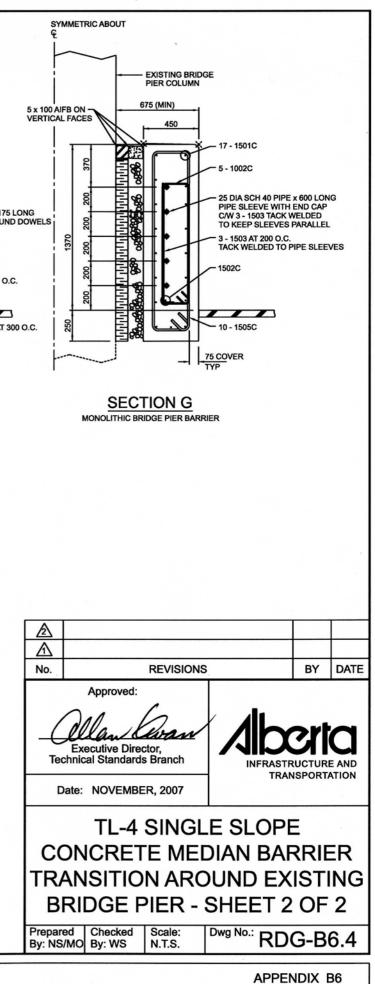
CONSTRUCTION NOTES:

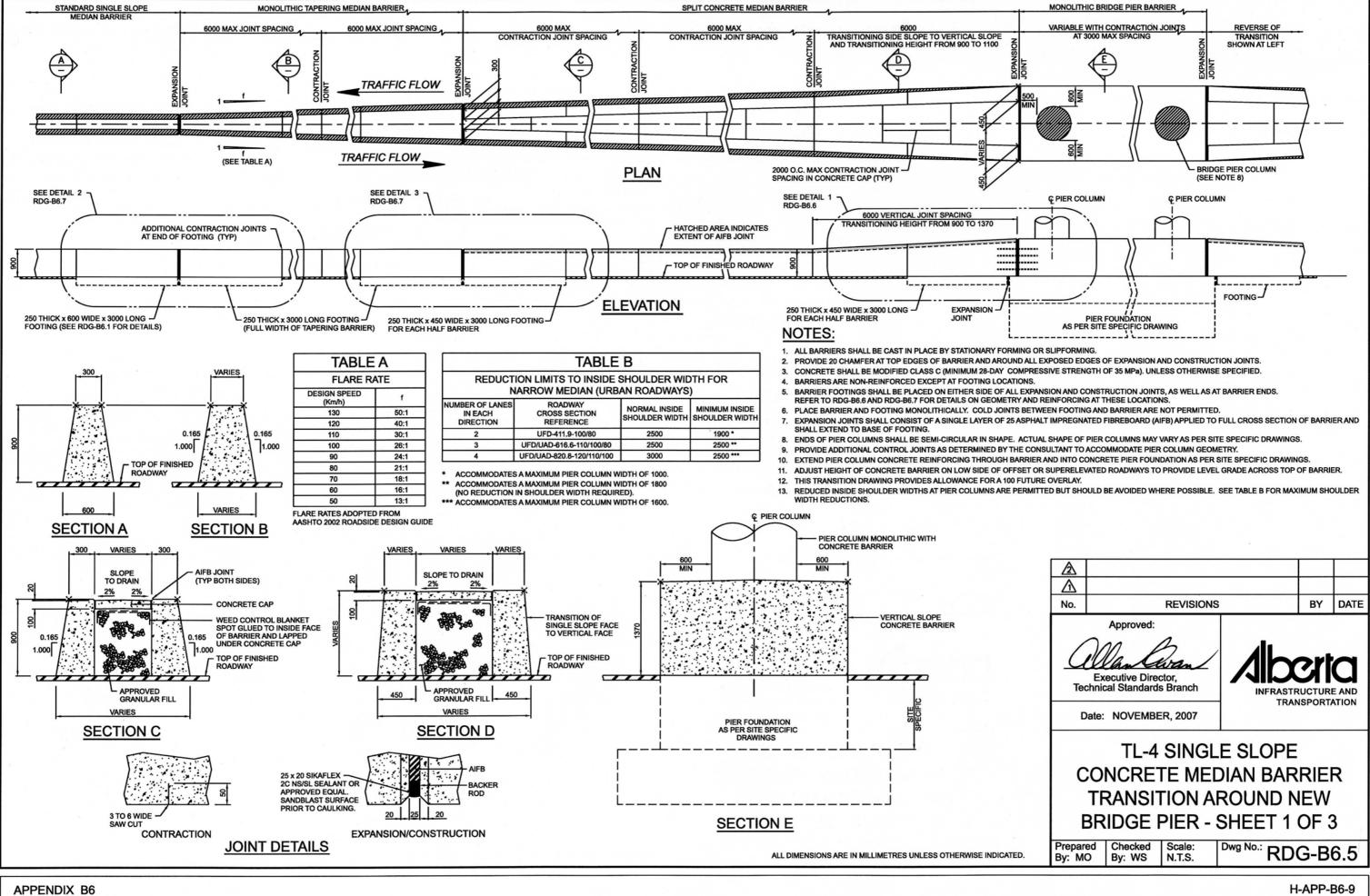
- 1. THE 3 1503 L-SHAPED DOWELS SHALL BE SHOP WELDED TO THE 25 DIA X 40 SCHEDULE PIPE SLEEVES, COMPLETE WITH END CAP, TO ENSURE THE SLEEVES ARE MAINTAINED PARALLEL.
- A 25 DIA X 25 LONG COMPRESSIBLE PLUG, SUCH AS POLYSTYRENE, SHALL BE INSERTED INTO EACH PIPE SLEEVE PRIOR TO SLIDING THE 25 DIA SMOOTH ROUND DOWELS INSIDE THE SLEEVES.
- 3. DURING PLACEMENT OF THE BARRIER CONCRETE SHOWN IN SECTION G, THE 25 DIA SMOOTH ROUND DOWELS SHALL BE SET 575 FROM FACE OF THE 25 THICK AIFB EXPANSION JOINT. THE DOWELS SHALL BE TIED SECURELY TO THE 1001C STIRRUPS TO PREVENT THE DOWELS FROM SLIDING OUT DURING CONCRETE PLACING AND VIBRATING.

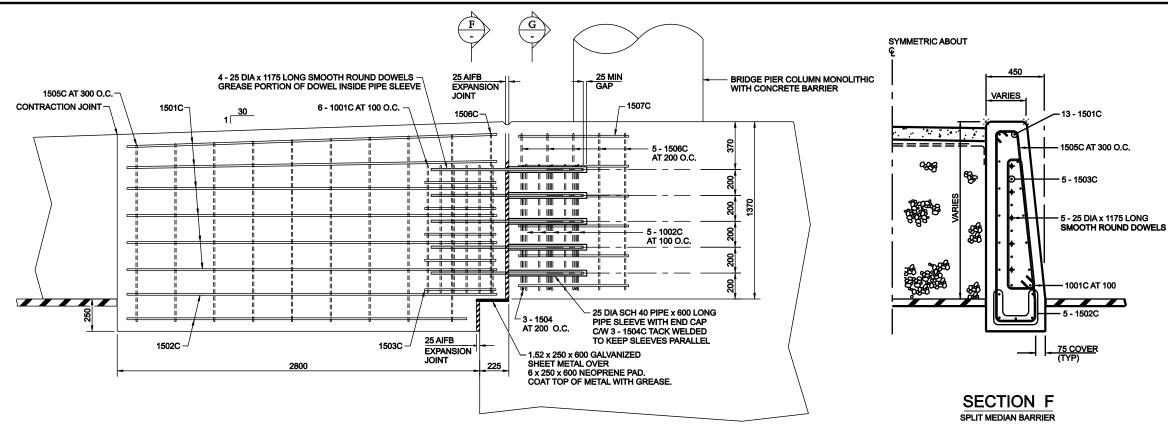
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE INDICATED.

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H-APP-B6-8







DETAIL 1

BAR LIST: BARRIER A											
MARK	SIZE	NO.	TYPE	Х	Y	Z	LENGTH	MASS			
1001C	10	6	С	220	950	95	2 425	12			
1002C	10	5	D	220	950		2 540	10			
1501C	15	13	STR				2 850	58			
1502C	15	5	STR				2 625	21			
1503C	15	5	STR				550	4			
1504	15	3	В		950		1130	5			
1505C	15	9	A	IN INCREMEN	ROM 175 TO 28	3105 (AVG)	44				
1506C	15	6	D	300	1220		3 320	31			
1507C	15	13	STR				850	17			

X ₿ \odot D

PLAIN

EPOXY COATED

TOTAL kg =

TOTAL kg =

- 5

197

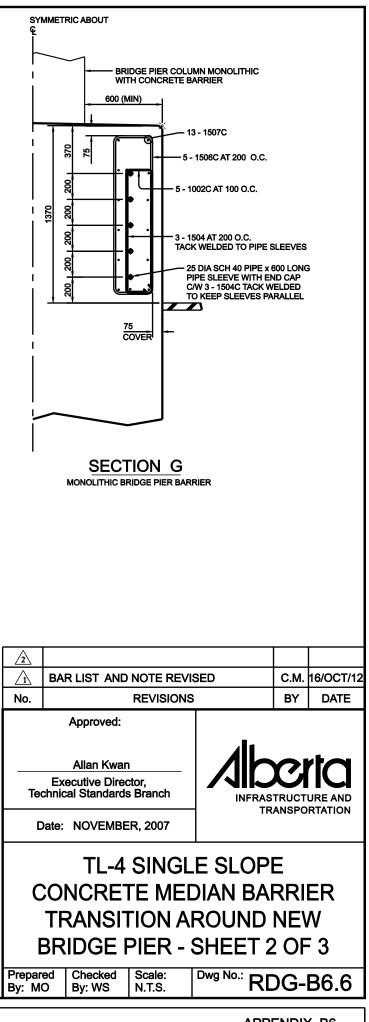
BAR LIST NOTES:

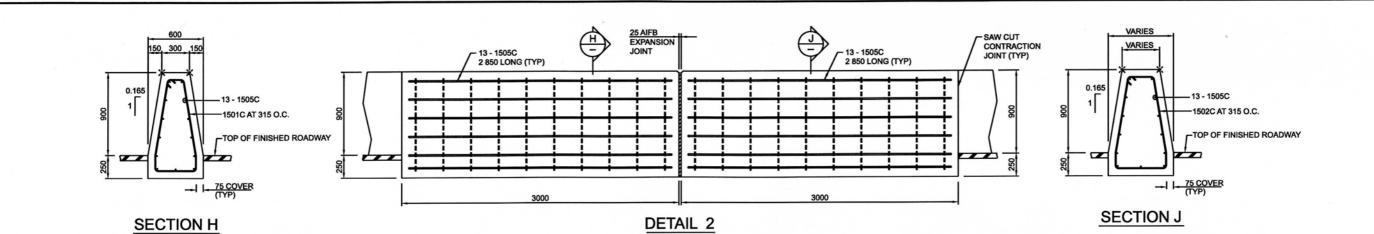
- DIAMETERS OF ALL BENDS AND DETAILS OF ALL HOOKS, UNLESS NOTED OTHERWISE, SHALL CONFORM TO THE RECOMMENDED SIZES DETAILED IN THE REINFORCING STEEL MANUAL OF STANDARD PRACTICE, FIRST CANADIAN EDITION 1992, PUBLISHED BY THE REINFORCING INSTITUTE OF CANADA
- REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CAN/CSA-G30.18M92 "BILLET STEEL BARS FOR CONCRETE REINFORCEMENT".
- ALL REINFORCING STEEL SHALL BE GRADE 400 UNLESS NOTED OTHERWISE.
- " C " DENOTES EPOXY COATED REINFORCEMENT.
- SMOOTH ROUND DOWELS SHALL BE ASTM GRADE A36, OR APPROVED EQUAL, WITH A MINIMUM YIELD STRENGTH OF 250 MPa.
- CONCRETE SHALL BE CLASS C (MINIMUM 28-DAY COMPRESSIVE STRENGTH $\underline{\Lambda}$ OF 35 MPa). UNLESS OTHERWISE SPECIFIED.
 - ALL CONCRETE CORNERS SHALL HAVE A 20 CHAMFER OR FILLET UNLESS NOTED OTHERWISE.
 - ALL REINFORCING STEEL SHALL HAVE 75 CLEAR COVER UNLESS NOTED OTHERWISE.

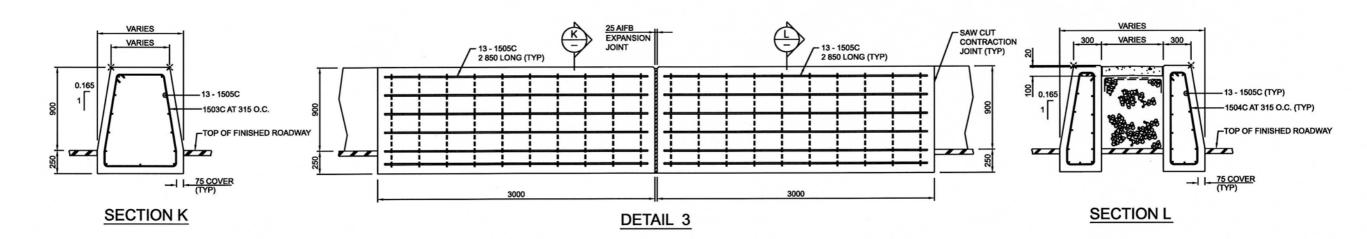
CONSTRUCTION NOTES:

- 1. THE 3 1504 L-SHAPED DOWELS SHALL BE SHOP WELDED TO THE 25 DIA X 40 SCHEDULE PIPE SLEEVES, COMPLETE WITH END CAP, TO ENSURE THE SLEEVES ARE MAINTAINED PARALLEL.
- 2. A 25 DIA X 25 LONG COMPRESSIBLE PLUG, SUCH AS POLYSTYRENE, SHALL BE INSERTED INTO EACH PIPE SLEEVE PRIOR TO SLIDING THE 25 DIA SMOOTH ROUND DOWELS INSIDE THE SLEEVES.
- 3. DURING PLACEMENT OF THE BARRIER CONCRETE SHOWN IN SECTION G, THE 25 DIA SMOOTH ROUND DOWELS SHALL BE SET 575 FROM FACE OF THE 25 THICK AIFB EXPANSION JOINT. THE DOWELS SHALL BE TIED SECURELY TO THE 1001C STIRRUPS TO PREVENT THE DOWELS FROM SLIDING OUT DURING CONCRETE PLACING AND VIBRATING.

ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE INDICATED.







MARK	SIZE	NO.	TYPE	х	Y	z	LENGTH	MASS
1501C	15	10	Α	450	1 000	178	2 930	46
1502C	15	10	A	IN INCREMENT Y = 1 000	ROM 178 TO (1	2 930 + 5 700/f	46.00 + 89.49/	
1503C	15	10	A	IN INCREMENT Y = 1 000	ROM (763 - 570	4 253 - 5 700/f	66.77 - 89.49/1	
1504C	15	20	в	IN INCREME Y = 1 000 Z = VARIES F	X = VARIES FROM 164 TO (164 + 2850/f) IN INCREMENTS OF 633/f			39.10 + 44.75/1
1505C	15	65	STR				2 850	291

EPOXY COATED TOTAL kg = 443 + 44.8/f

BAR LIST NOTES:

175

х

X

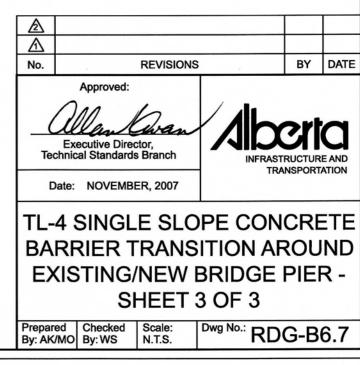
B

75

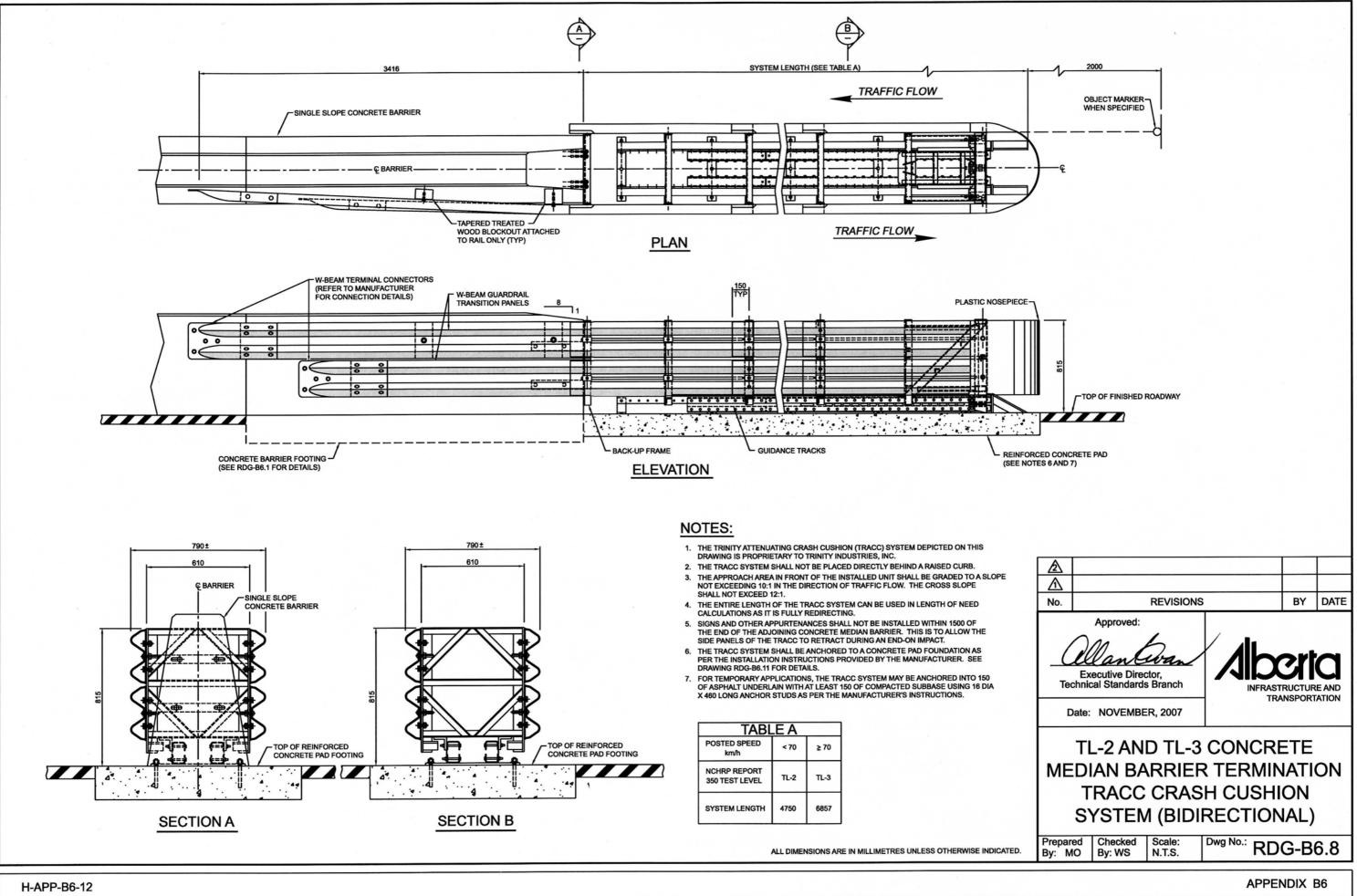
- DIAMETERS OF ALL BENDS AND DETAILS OF ALL HOOKS, UNLESS NOTED OTHERWISE, SHALL CONFORM TO THE RECOMMENDED SIZES DETAILED IN THE REINFORCING STEEL MANUAL OF STANDARD PRACTICE, FIRST CANADIAN EDITION 1992, PUBLISHED BY THE REINFORCING INSTITUTE OF CANADA.
- REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CAN/CSA-G30.18M92 "BILLET STEEL BARS FOR CONCRETE REINFORCEMENT".
- ALL REINFORCING STEEL SHALL BE GRADE 400 UNLESS NOTED OTHERWISE.
- C DENOTES EPOXY COATED REINFORCEMENT.
- THE FLARE RATE VALUE " f" IS DEFINED IN TABLE A ON RDG-B6.3 AND RDG-B6.5.

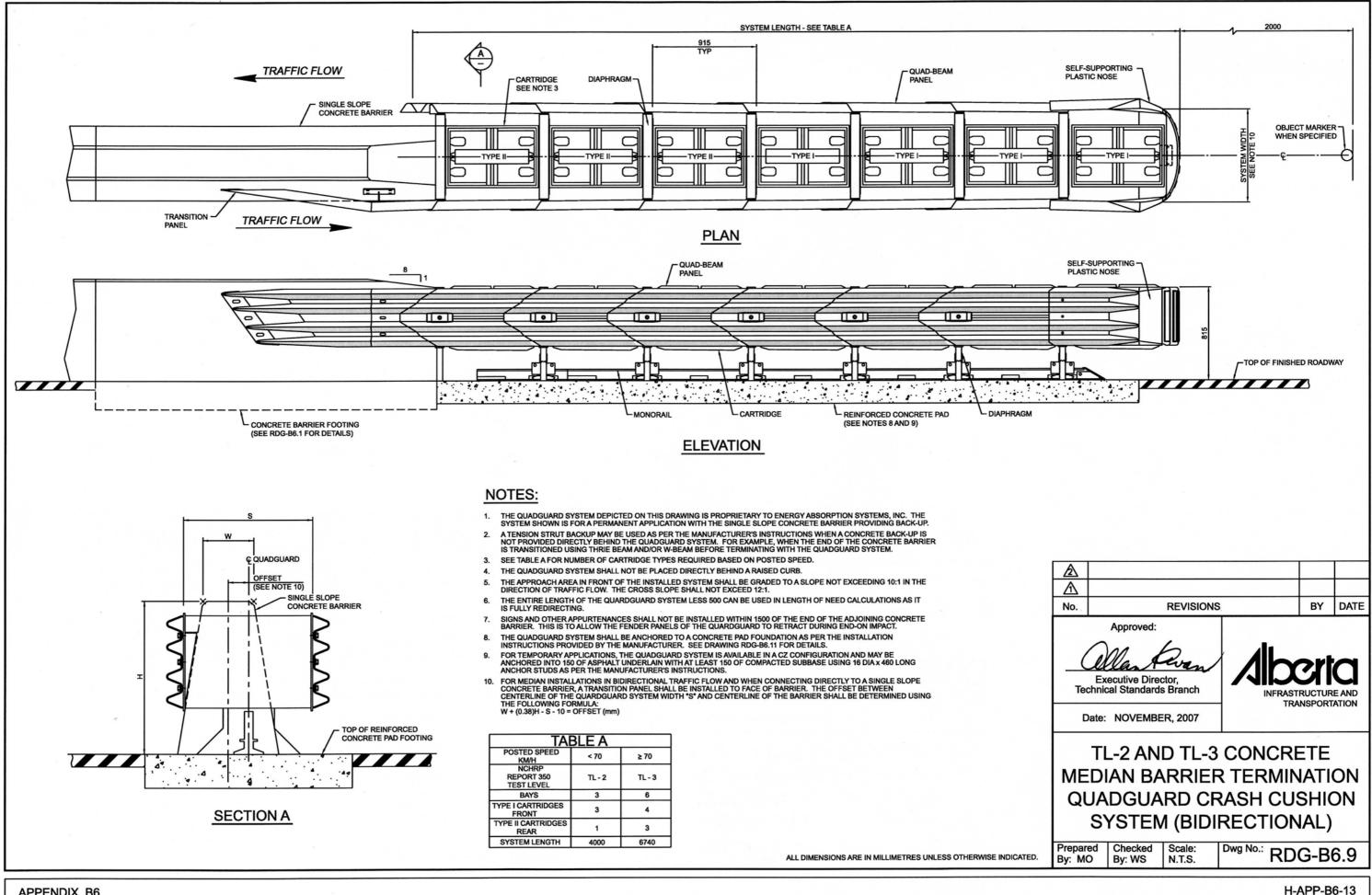
ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE INDICATED.

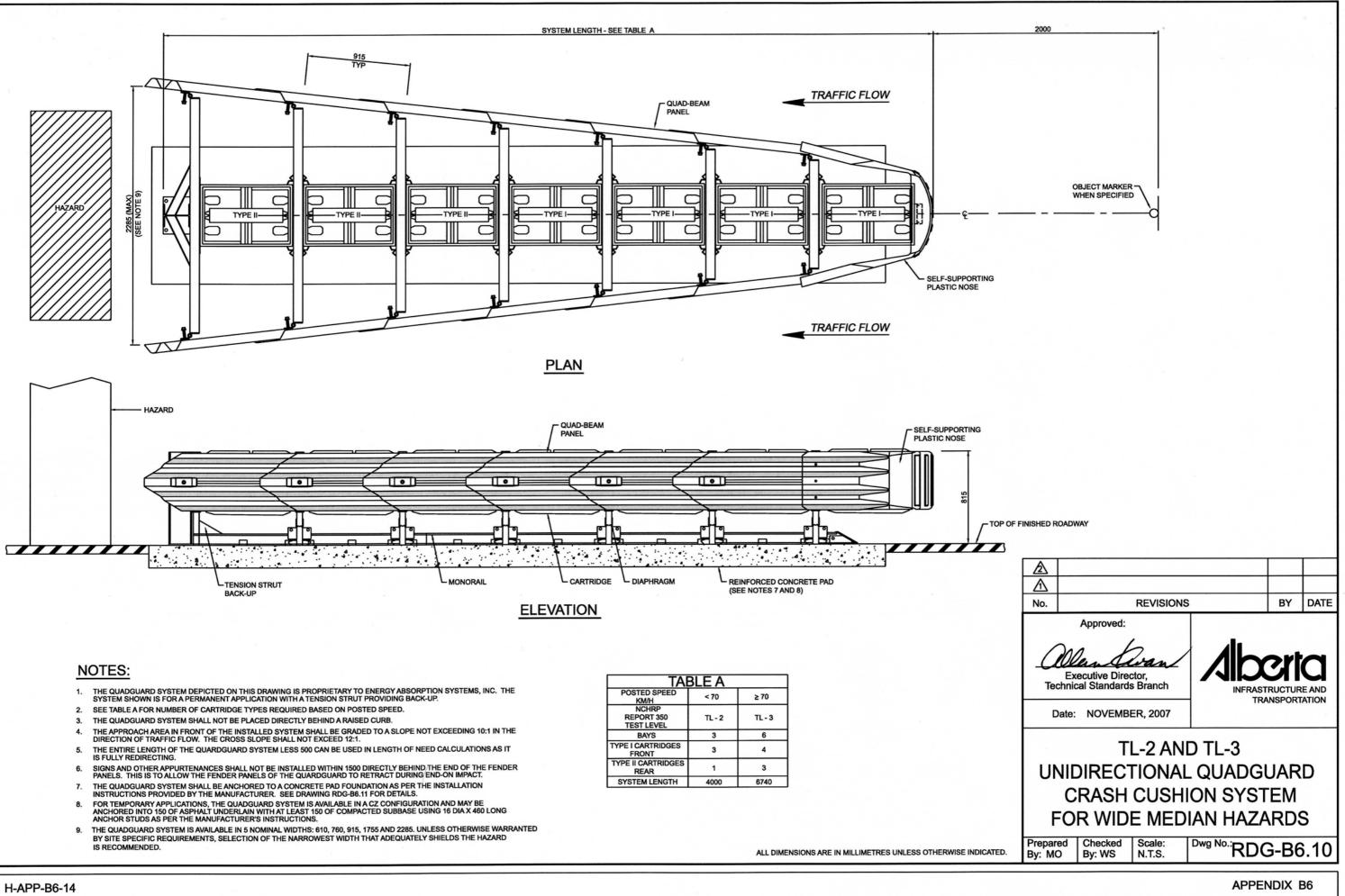
APPENDIX B6



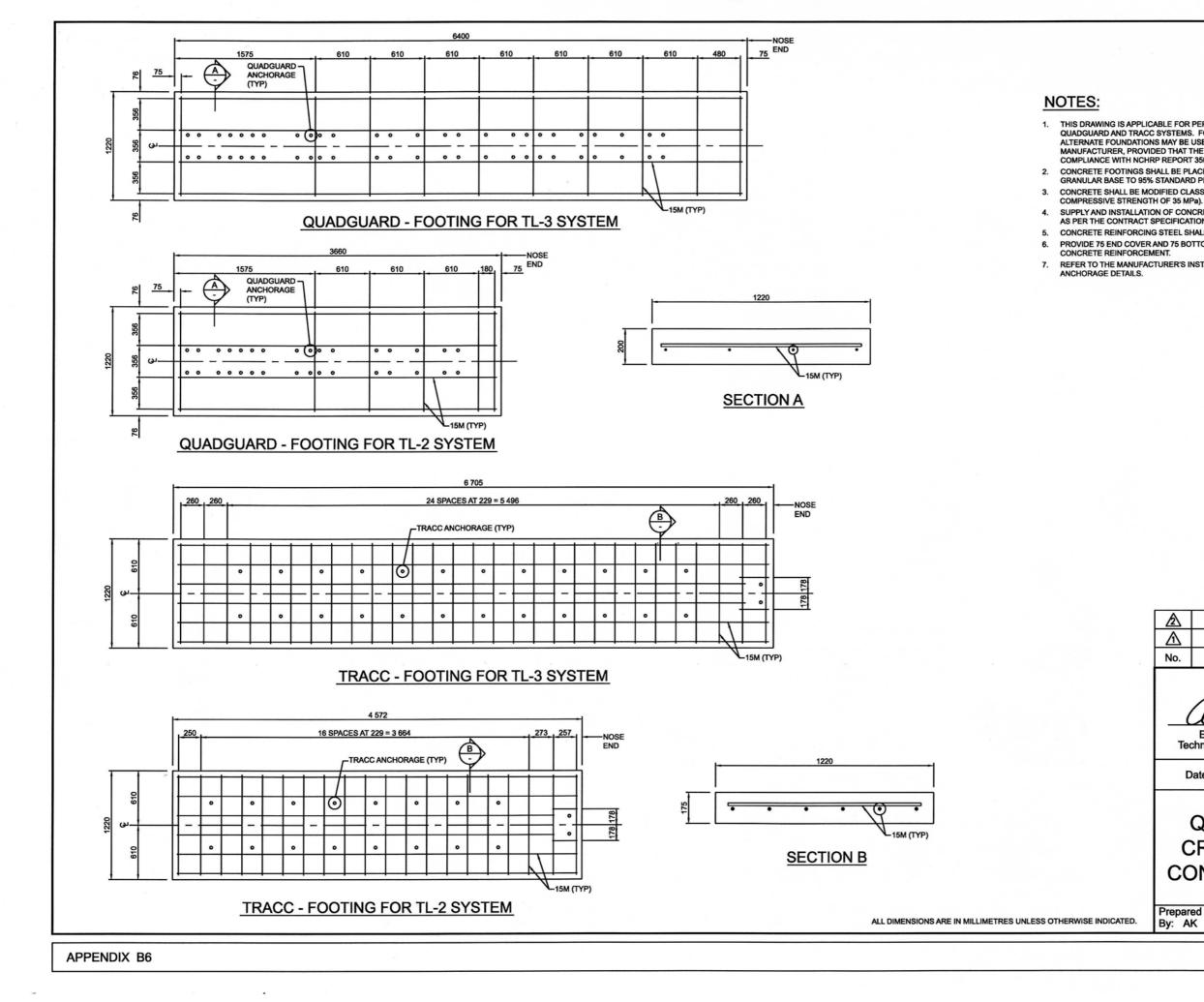
H-APP-B6-11







TA	BLEA		
POSTED SPEED KM/H	< 70	≥ 70	
NCHRP REPORT 350 TEST LEVEL	TL - 2	TL - 3	
BAYS	3	6	
TYPE I CARTRIDGES FRONT	3	4	
TYPE II CARTRIDGES REAR	1	3	
SYSTEM LENGTH	4000	6740	



1. THIS DRAWING IS APPLICABLE FOR PERMANENT APPLICATIONS OF QUADGUARD AND TRACC SYSTEMS. FOR TEMPORARY APPLICATIONS, ALTERNATE FOUNDATIONS MAY BE USED AS RECOMMENDED BY THE MANUFACTURER, PROVIDED THAT THE INSTALLED SYSTEM MAINTAINS COMPLIANCE WITH NCHRP REPORT 350 FOR TEST LEVEL 3 (TL-3). 2. CONCRETE FOOTINGS SHALL BE PLACED ON WELL COMPACTED

GRANULAR BASE TO 95% STANDARD PROCTOR MAXIMUM DRY DENSITY. 3. CONCRETE SHALL BE MODIFIED CLASS C (MINIMUM 28-DAY

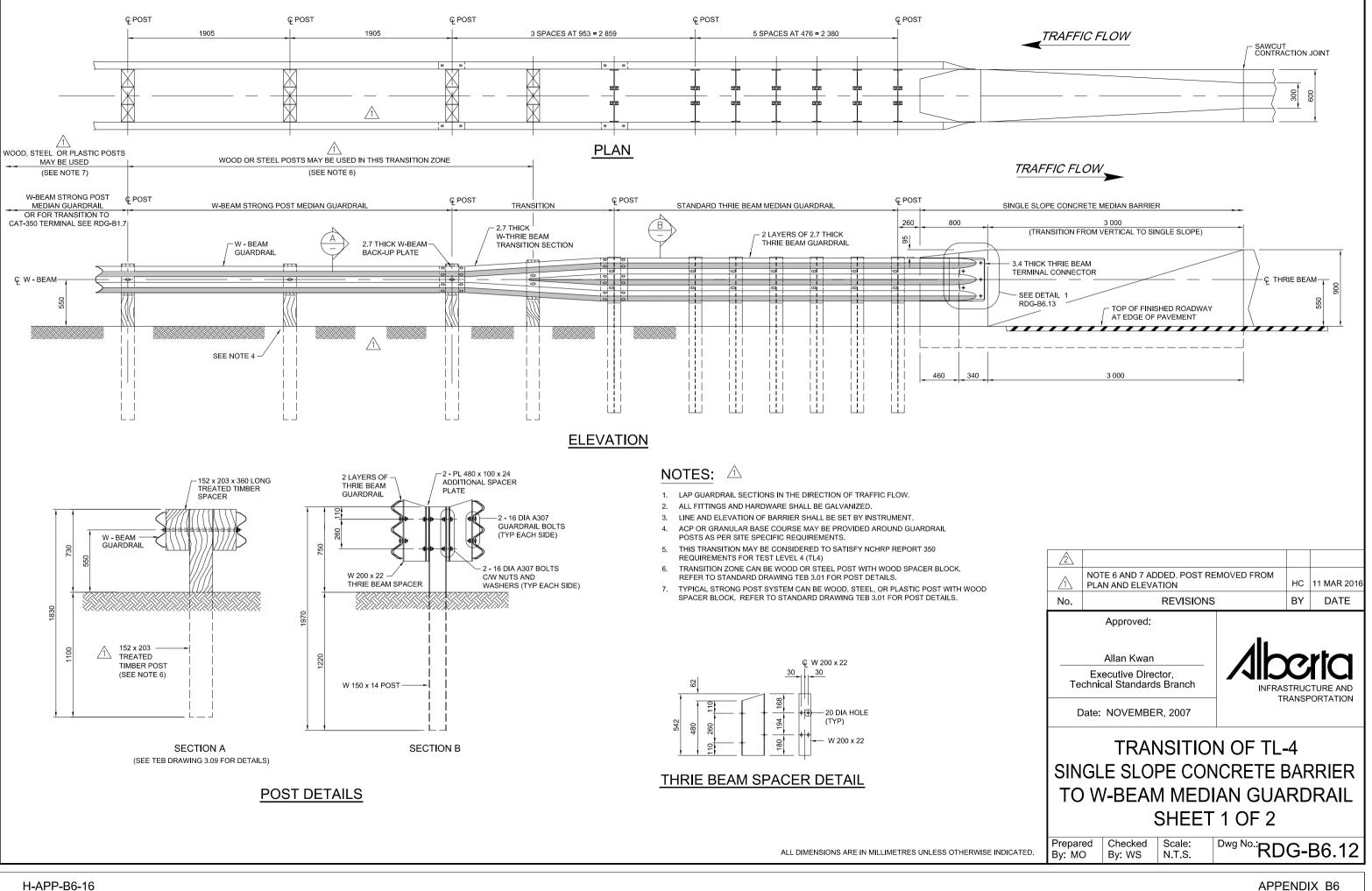
4. SUPPLY AND INSTALLATION OF CONCRETE AND REINFORCING STEEL AS PER THE CONTRACT SPECIFICATIONS.

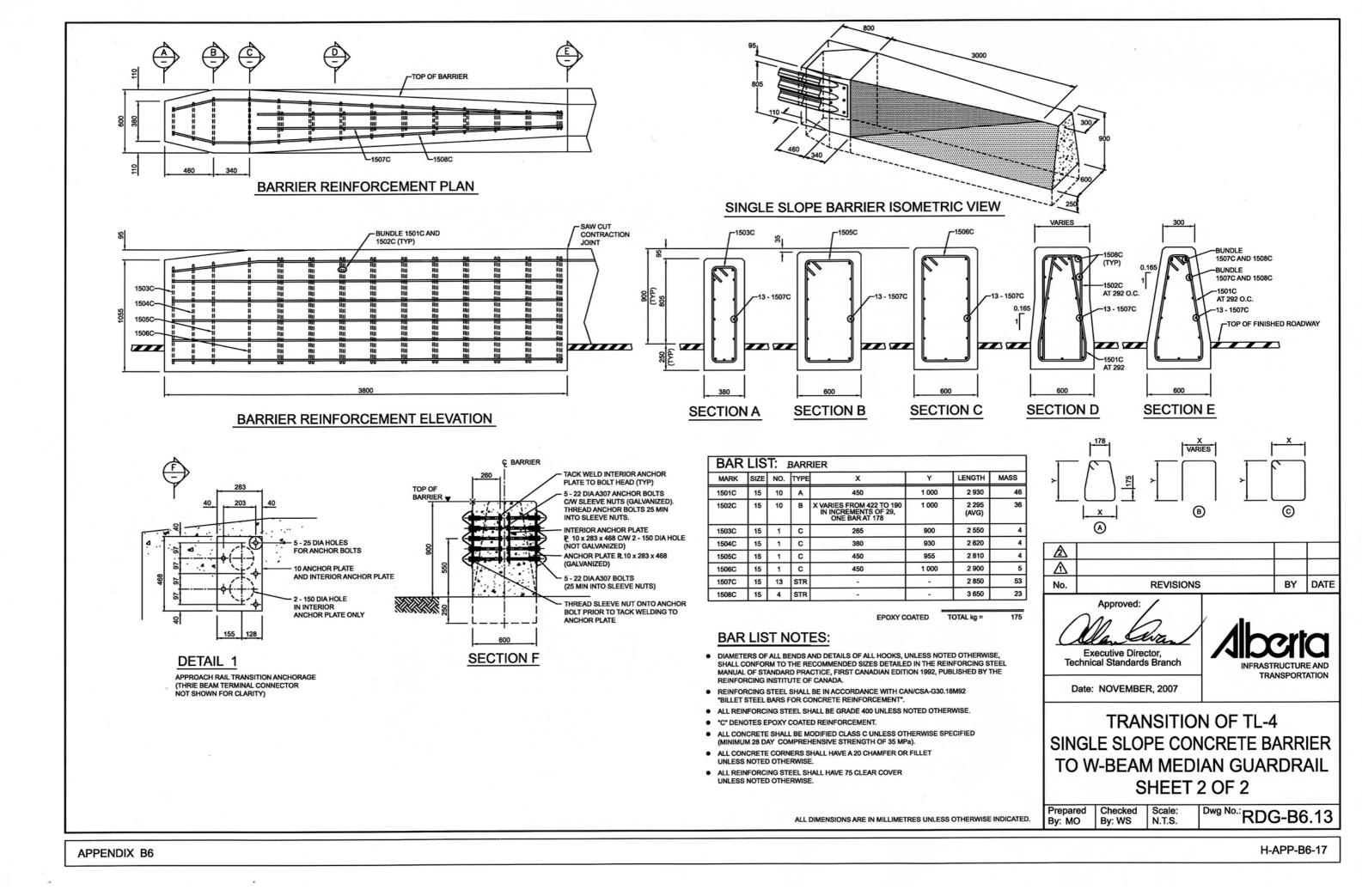
5. CONCRETE REINFORCING STEEL SHALL BE EPOXY COATED.

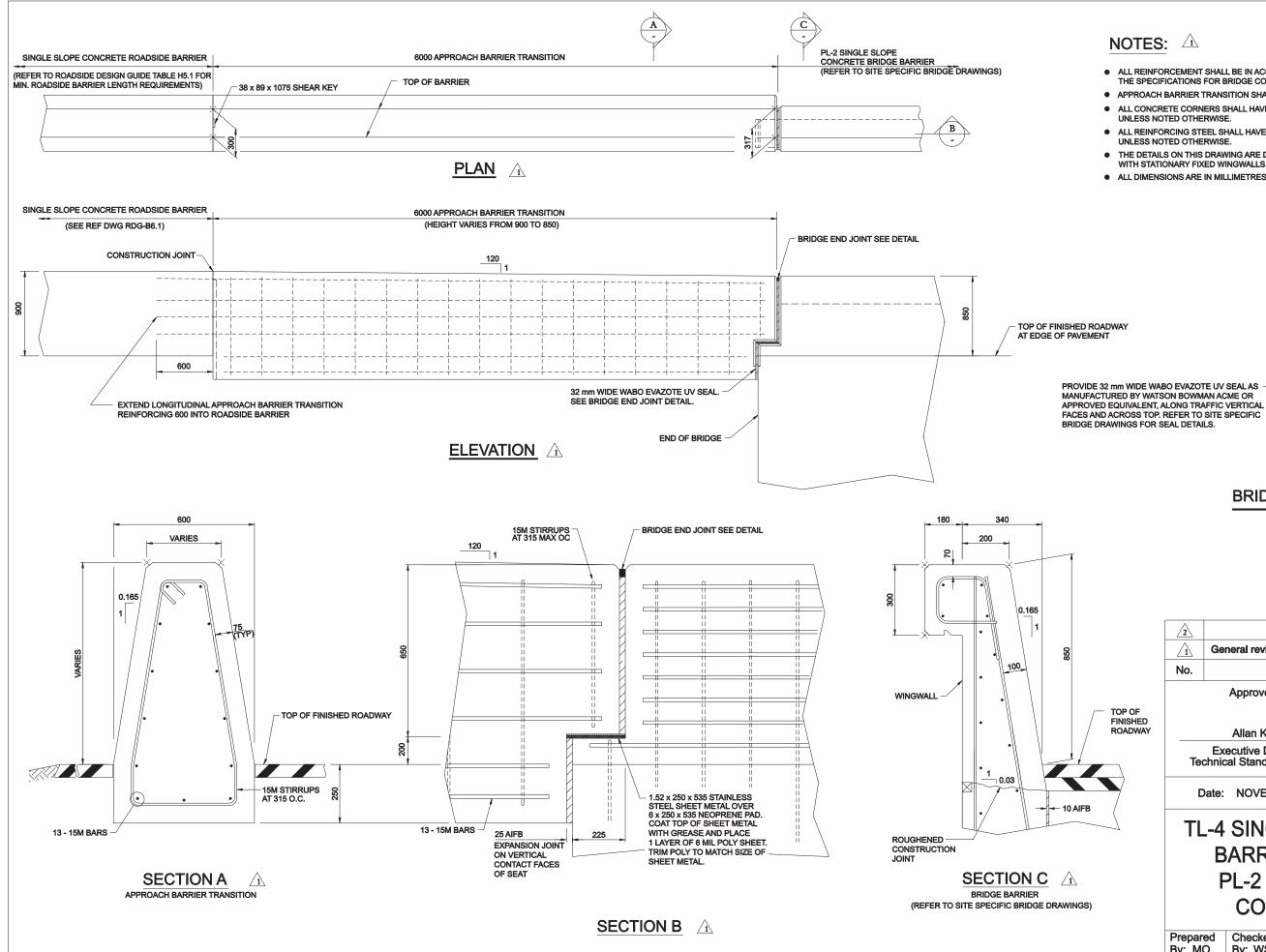
PROVIDE 75 END COVER AND 75 BOTTOM COVER FOR ALL CONCRETE REINFORCEMENT.

7. REFER TO THE MANUFACTURER'S INSTALLATION INSTRUCTIONS FOR ANCHORAGE DETAILS.

	\mathbb{A}										
	No.			REVISIONS	5	BY	DATE				
			Approved:								
	Executive Director, Technical Standards Branch										
	Da	ate:	NOVEMB	ER, 2007		NOPORI	ALION				
	QUADGUARD AND TRACC CRASH CUSHION SYSTEMS CONCRETE PAD FOUNDATION										
).	Prepare By: AK		Checked By: WS	Scale: N.T.S.		G-B6	.11				
	H-APP-B6-15										

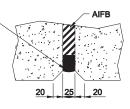






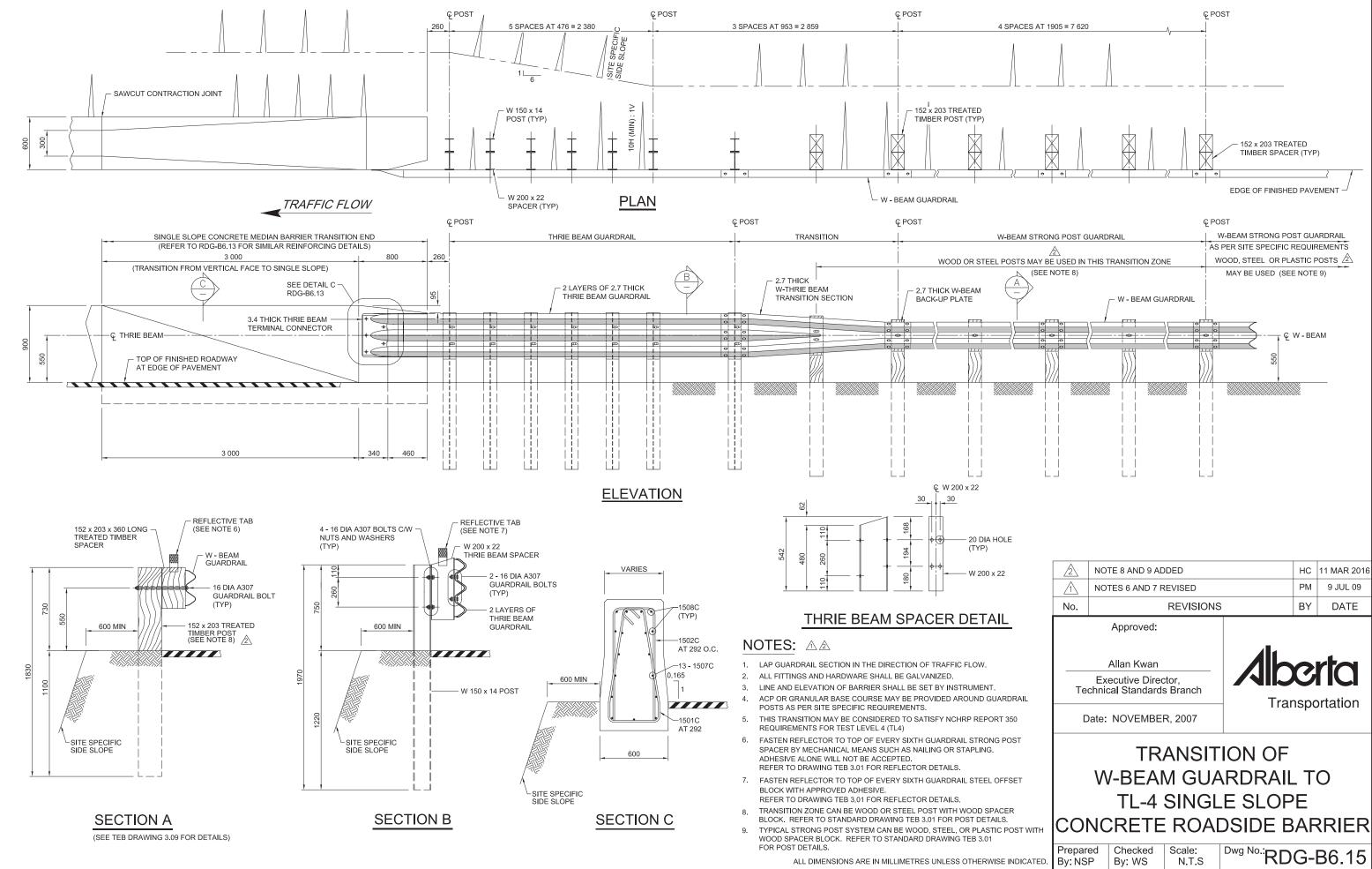
NOTES: 🛆

- ALL REINFORCEMENT SHALL BE IN ACCORDANCE WITH SECTION 5 OF THE SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- APPROACH BARRIER TRANSITION SHALL BE HPC CONCRETE.
- ALL CONCRETE CORNERS SHALL HAVE A 20 CHAMFER OR FILLET UNLESS NOTED OTHERWISE.
- ALL REINFORCING STEEL SHALL HAVE 50 CLEAR COVER UNLESS NOTED OTHERWISE.
- THE DETAILS ON THIS DRAWING ARE DEVELOPED FOR USE ON BRIDGES WITH STATIONARY FIXED WINGWALLS.
- ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE INDICATED.



BRIDGE END JOINT DETAIL A

2										
	Ger	neral revision	S		C.M.	15/JUL/11				
No.			5	BY	DATE					
		Approved:								
		Allan Kwan ecutive Direc al Standards	ctor, s Branch		STRUCI	TURE AND DRTATION				
Τl	TL-4 SINGLE SLOPE CONCRETE BARRIER TRANSITION TO PL-2 STANDARD BRIDGE CONCRETE BARRIER									
Prepar By: M		Checked By: WS	Scale: N.T.S.)G-l	B6.14				
	APPENDIX B6									



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MISCELLANEOUS DRAWINGS

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Appendix B7

Miscellaneous Drawings

TABLE OF CONTENTS

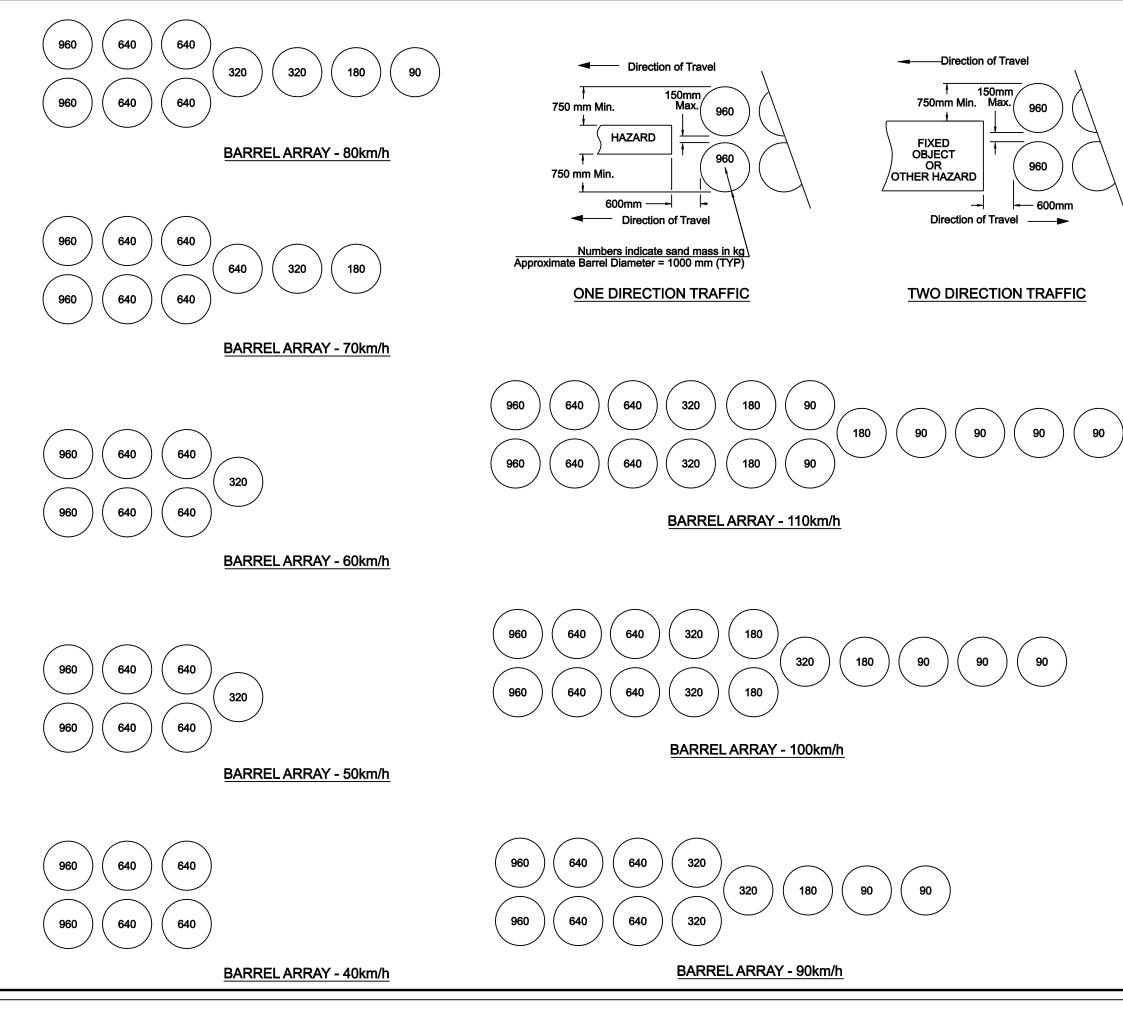
Dwg. No.

Drawing Title

Page Number

TEB 3.19	Sand Barrel Cushion System	H-APP-B7-1
TEB 3.51	Guide Post Delineation for Guardrail	H-APP-B7-3
TEB 1.81	Typical Breakaway Wood Post	H-APP-B7-4
RDG-B7.1	TL-3 W-Beam Guardrail Placement at Minor Structures and Box Culverts	H-APP-B7-5
RDG-B7.2	TL-4 Single Slope Concrete Barrier Transition at Median Light Standard – Sheet 1 of 2	H-APP-B7-6
RDG-B7.3	TL-4 Single Slope Concrete Barrier Transition at Median Light Standard – Sheet 2 of 2	H-APP-B7-7
RDG-B7.4	Placement and Protection of Overhead Sign Supports for Divided Roads – Sheet 1 of 2	H-APP-B7-8
RDG-B7.5	Placement and Protection of Overhead Sign Supports for Divided Roads – Sheet 2 of 2	H-APP-B7-9

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General notes:

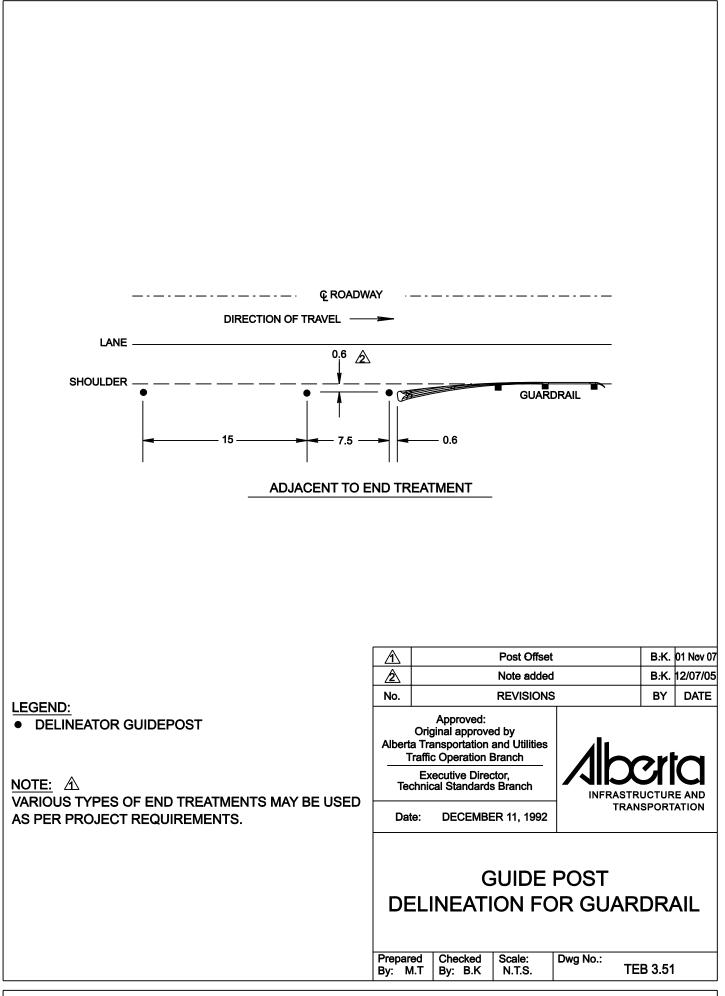
- 1. Only crash tested components meeting NCHRP 350 requirements shall be used. The systems currently available are:
 - a. Energite Inertia Barrier System by Quixote Transportation Safety Inc.
 - b. Fitch Sand Barrel System by Quixote Transportation Safety Inc.
 - c. TrafFix Impact Attenuator Sand Barrels by TrafFix Devices Inc.
- 2. The sand mass shall be clearly marked on each barrel.
- 3. For permanent installations, the Fitch System shall be used.
- 4. The systems shall be installed strictly in accordance with manufacturer's recommendations.
- 5. The sand barrel systems are non-directive and break up during impact. The vehicle speed is slowed by transfer of it's momentum to the sand, allowing for safe, steady deceleration. Sand and plastic parts from the system will scatter in the direction of impact.
- 6. Fill sand shall conform to ASTM C-33 washed concrete sand or approved equal. Moisture content of sand shall be three percent or less to minimize caking. The sand shall be mixed with an appropriate percentage of rock salt when use during freezing temperature is expected.
- 7. Barrels shall be set as far from the traveled way as possible to minimize the number of brush or nuisance hits.
- 8. Barrel layout shall conform with the configuration for the appropriate posted highway speed.
- 9. In the case of work zone installations, the design speed shall be at least equal to the speed posted through the work zone.

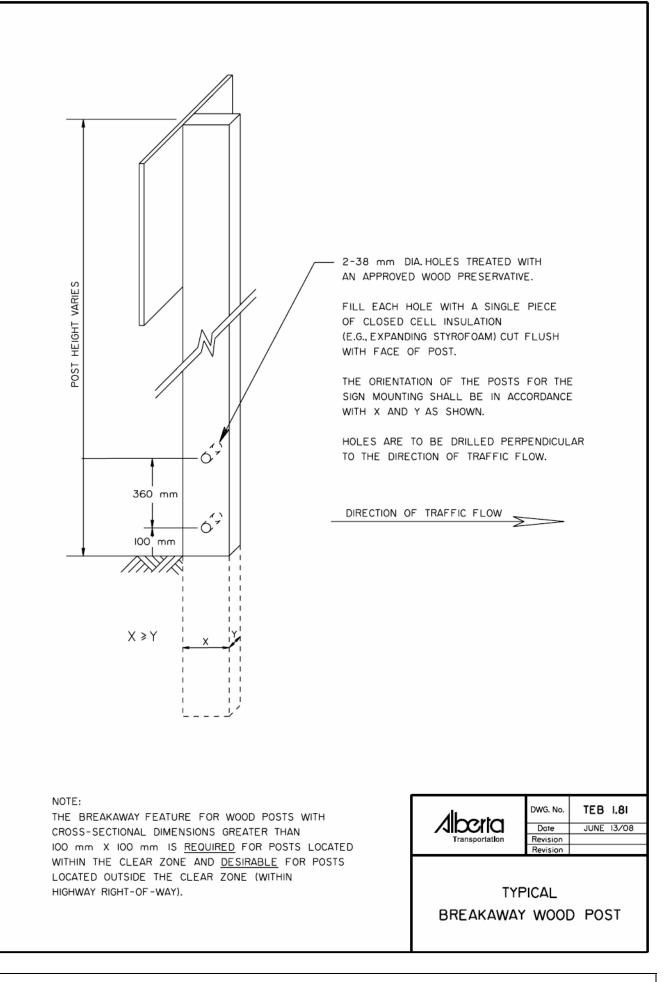
▲ REQUIRES DEPARTMENT APPROVAL ON NEW INSTALLATIONS

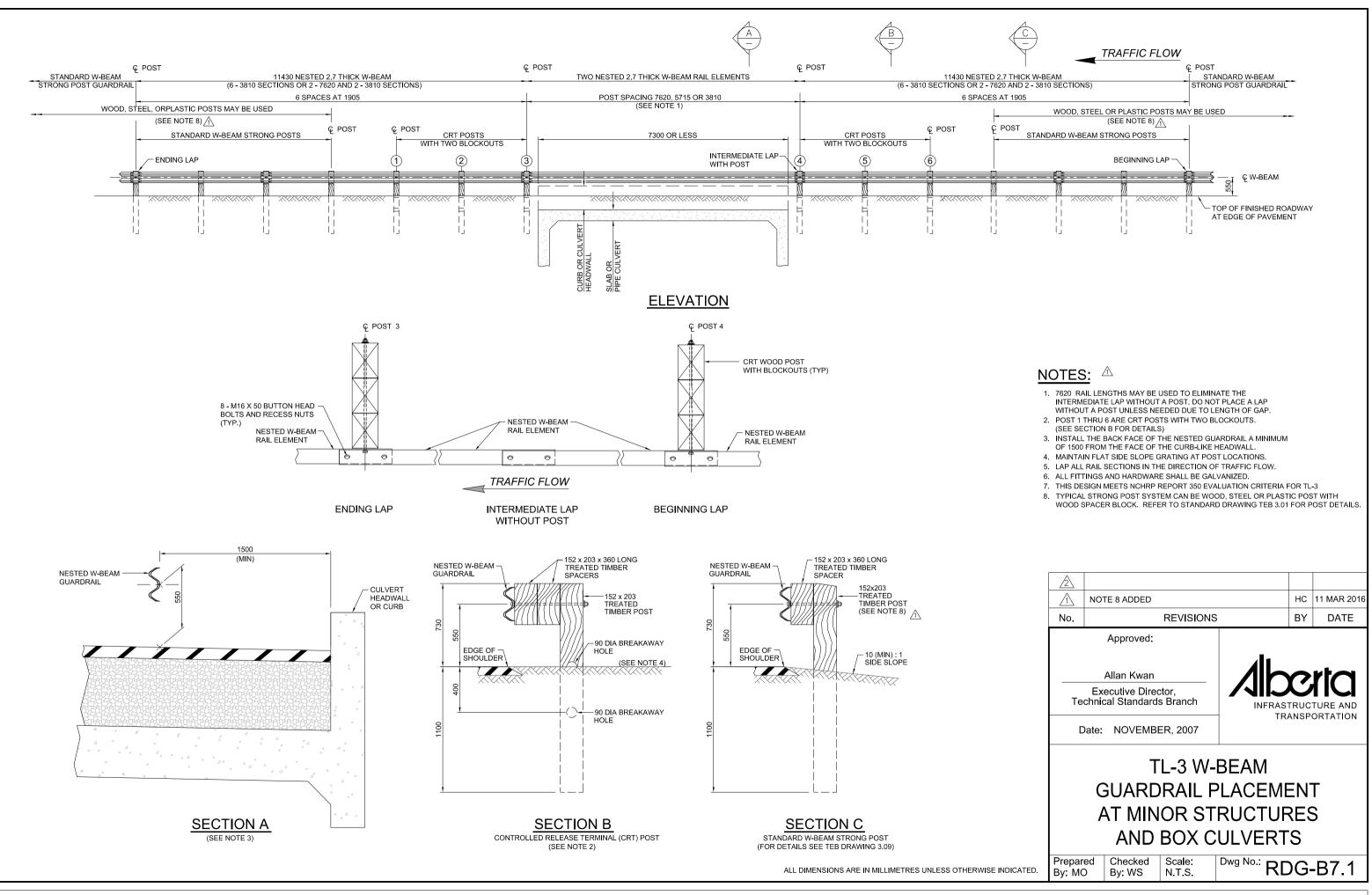
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\triangle	AP	PROVAL NO	TE ADDED		P.M.	02 JUĿ 13				
No.			REVISION	3	BY	DATE				
	Exe	Approved: iginal signed Allan Kwan ecutive Direc al Standards	ctor, s Branch	INFRAS	TRUCT	JRE AND RTATION				
Date: NOVEMBER 23, 2004 SAND BARREL CUSHION SYSTEM										
Prepar By: N	ed I.T.	Checked By: R.Y.	Scale: N.T.S.	Dwg No.:	TEB 3.	19				

H-APP-B7-1

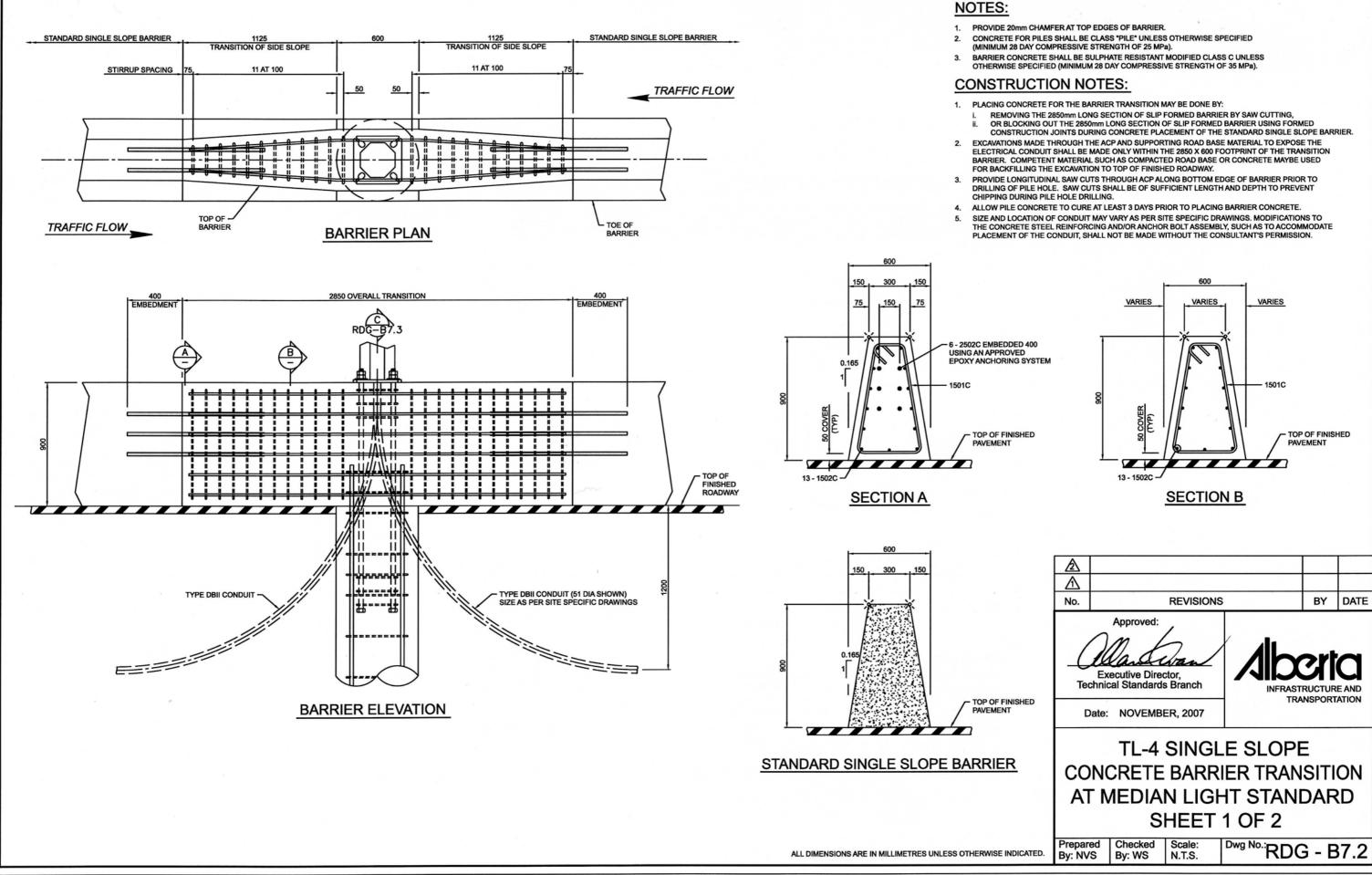
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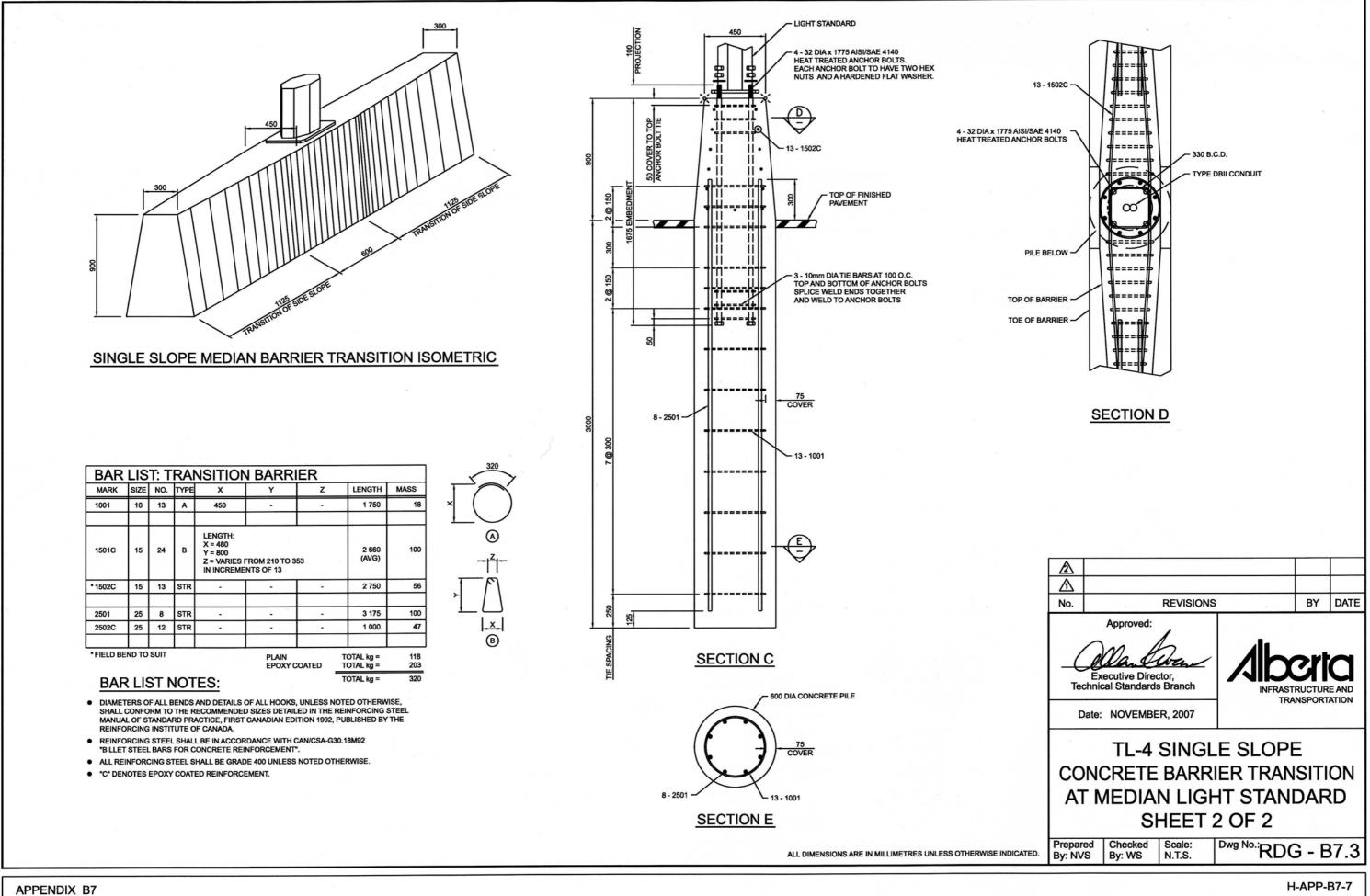




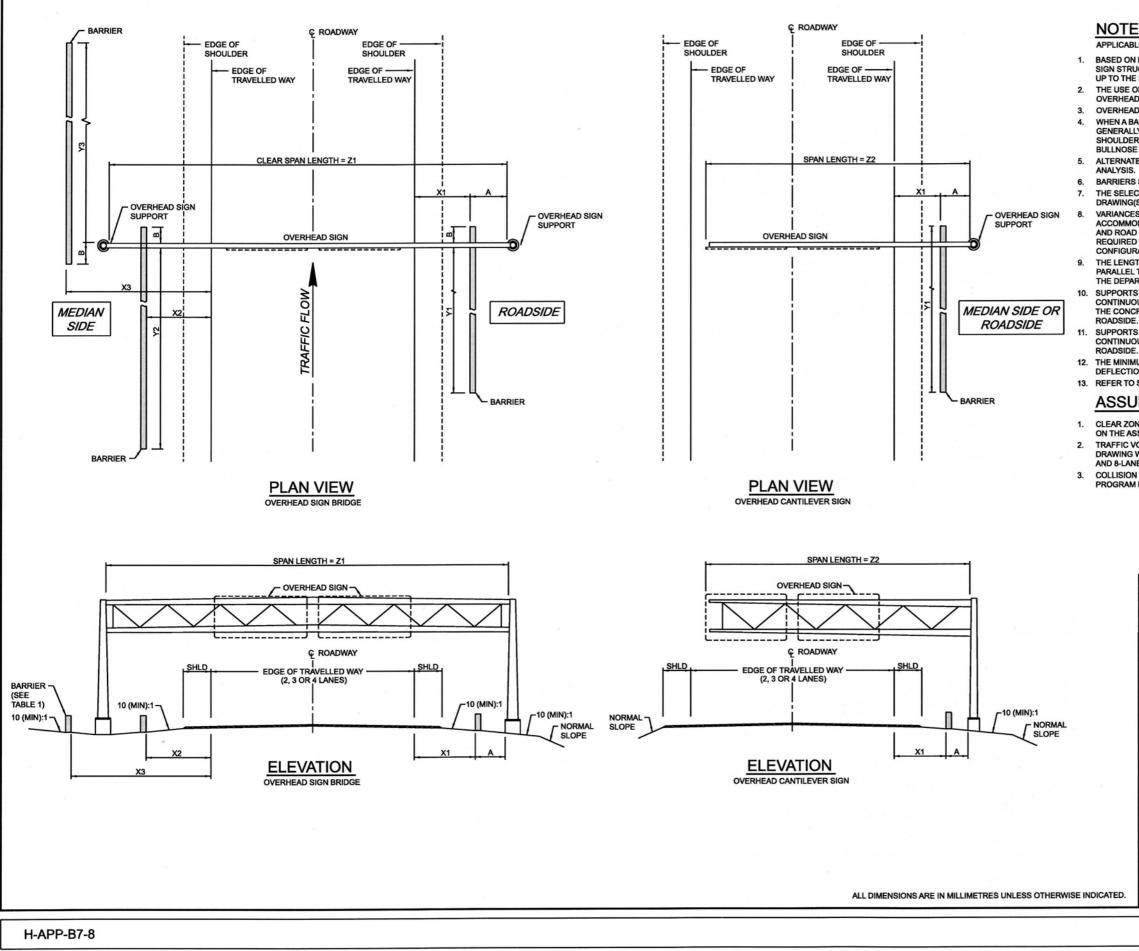


H-APP-B7-5





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NOTES:

APPLICABLE FOR URBAN AND RURAL CROSS SECTIONS

BASED ON BENEFIT/COST ANALYSIS WITH CONSIDERATIONS FOR COLLISION COSTS, CANTILEVER SIGN STRUCTURES ARE GENERALLY PREFERRED OVER OVERHEAD SIGN BRIDGE STRUCTURES, UP TO THE SPECIFIED MAXIMUM CANTILEVER SPAN LENGTH PROVIDED IN TABLES 2 AND 3. THE USE OF SAND BARRELS (SEE AIT STANDARD DRAWING TEB 3.19) IS RECOMMENDED WHEN OVERHEAD SIGN SUPPORTS ARE PLACED AT OR JUST BEYOND THE CLEAR ZONE LINE.

OVERHEAD SIGN SUPPORTS LOCATED WITHIN THE CLEAR ZONE REQUIRE BARRIER PROTECTION. WHEN A BARRIER SYSTEM IS WARRANTED, THE THRIE BEAM BULLNOSE BARRIER SYSTEM IS GENERALLY RECOMMENDED, PROVIDED THAT THERE IS SUFFICIENT SPACE IN THE MEDIAN OR SHOULDER FOR PROPER INSTALLATION. REFER TO RDG-B5.6 AND RDG-B5.7 FOR THRIE BEAM BULLNOSE GENERAL LAYOUTS.

5. ALTERNATE BARRIER SYSTEMS SHOWN IN TABLE 1 MAY BE USED IF PROVEN BY BENEFIT-COST

BARRIERS SHALL BE LOCATED AT OR BEYOND THE RECOMMENDED SHY LINE OFFSET. THE SELECTED BARRIER SYSTEM SHALL BE INSTALLED AS PER THE APPLICABLE AIT STANDARD DRAWING(S).

VARIANCES FROM TABLES 2 AND 3 MAY BE REQUIRED ON A SITE SPECIFIC BASIS TO ACCOMMODATE LIMITING FACTORS SUCH AS ROADSIDE OBSTACLES, ENCROACHING SIDESLOPES, AND ROAD CURVATURE. IN SUCH CASES, A SITE-SPECIFIC BENEFIT-COST ANALYSIS MAY BE REQUIRED TO DETERMINE THE OPTIMUM OVERHEAD SIGN SPAN AND BARRIER SYSTEM CONFIGURATION.

THE LENGTH OF NEED (LON) SHOWN IN TABLES 2 AND 3 ARE BASED ON ALIGNING THE BARRIER PARALLEL TO THE ROADWAY. THE LON MAY BE REDUCED BY FLARING THE BARRIER. REFER TO THE DEPARTMENT'S ROADSIDE DESIGN GUIDE FOR MAXIMUM FLARE RATES.

10. SUPPORTS FOR CANTILEVER SIGNS IN URBAN AREAS WITH NARROW MEDIANS WITH CONTINUOUS MEDIAN CONCRETE BARRIER ARE GENERALLY PREFERRED IF PLACED ON THE CONCRETE MEDIAN BARRIER AS OPPOSED TO THE SUPPORT BEING LOCATED ON THE

SUPPORTS FOR CANTILEVER SIGNS IN RURAL AND URBAN AREAS IN WIDER MEDIANS (WITHOUT CONTINUOUS CONCRETE MEDIAN BARRIER) ARE MORE ECONOMICAL IF PLACED ON THE

12. THE MINIMUM SETBACK DISTANCE "A" PROVIDES ALLOWANCE FOR THE MAXIMUM DYNAMIC DEFLECTION OF THE BARRIER.

13. REFER TO SECTION H.5.4.4 FOR ZONE OF INTRUSION CONSIDERATIONS.

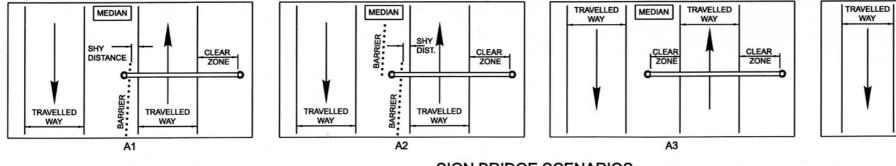
ASSUMPTIONS

1. CLEAR ZONE DISTANCES USED IN THE DEVELOPMENT OF THIS STANDARD DRAWING WERE BASED ON THE ASSUMPTION OF FILL SIDE SLOPES OF 6:1 OR FLATTER.

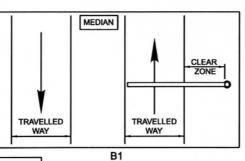
TRAFFIC VOLUMES USED FOR BENEFIT-COST ANALYSIS IN THE DEVELOPMENT OF THIS STANDARD DRAWING WERE ASSUMED TO BE 10 000 VPD, 30 000 VPD, AND 50 000 VPD FOR 4-LANE, 6-LANE, AND 8-I ANE DIVIDED HIGHWAYS, RESPECTIVELY,

COLLISION COSTS FOR BENEFIT-COST ANALYSIS WERE OBTAINED USING THE COMPUTER PROGRAM RSAP (ROADSIDE SAFETY ANALYSIS PROGRAM).

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No.			REVISIONS	3	BY	DATE			
	Approved: Man Cuan Executive Director, Technical Standards Branch								
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OF	C	VERH FOR S	ead s Divide Heet	D PROTEC IGN SUPF D ROADS 1 OF 2	POR	TS			
	Prepared Checked Scale: Dwg No.: RDG-B7								
				APPE	NDIX E	37			



SIGN BRIDGE SCENARIOS



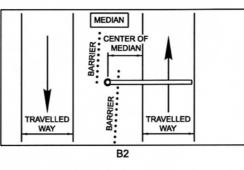


TABLE 1 - MINIMUM BARRIER SETBACK AND EXTENSION

BARRIER TYPE	SETBACK FROM HAZARD, A (mm) - (SEE NOTE 1) (SEE SHEET 1 OF 2)	EXTENSION, B (mm)
HIGH TENSION CABLE	APPROX. 2500 (REFER TO PROPRIETOR SPECS.)	10000
PRECAST CONCRETE	VARIES (UP TO 2400 mm FOR UNANCHORED)	9000
STRONG POST W-BEAM	2100	4000 *
MODIFIED THRIE BEAM	1200	4000 *
THRIE BEAM BULLNOSE	1200	4000
CIP CONCRETE	750	3000

CANTILEVER SIGN SCENARIOS

* WHEN ANCHORED WITH A CABLE ANCHOR TERMINAL (REFER TO RDG-B1.1 AND RDG-B5.1)

TABLE 2 - BAR	RIER GE	OMET	RIC PA	RAME	FERS F	OR "	RURAL "	CROSS	SECTIONS			
AIT DESIGN DESIGNATION	ROADWAY FACILITY (No. OF LANES)	DESIGN SPEED (Km/hr)	OBSTACLE OFFSET ** X1 (m)	OBSTACLE OFFSET ** X2 (m)	OBSTACLE OFFSET ** X3 (m)	LENGTH OF NEED Y1 (m)	LENGTH OF NEED Y2 (m)	LENGTH OF NEED Y3 (m)	OVERHEAD SIGN CLEAR SPAN Z1 (m)	BRIDGE SCENARIO REFERENCE	CANTILEVER SIG CLEAR SPAN Z2 (m)	SCENARIC REFERENC
RAD-410.4-80	4	80	С	С					7.4 + 2C	A3	MAX: 7.4 + C (ROADSIDE)	B1
RAD-410.4-100	4	100	C	2.4			150		9.8 + C+A	A1	MAX: 7.4 + C (ROADSIDE)	B1
RAD-410.4-110	4	110	С	2.8	2.8+ 2A+P	-	150	150	10.2 + C + A	A2	MAX: 7.4 + C (ROADSIDE)	B1
RAD-411.4-80	4	80	С	C	-	-	-		7.4 + 2C	A3	MAX: 7.4 + C (ROADSIDE)	B1
RAD-411.4-100	4	100	C	2.4	-	-	150	-	9.8 + C + A	A1	MAX: 7.4 + C (ROADSIDE)	B1
RAD-411.4-110	4	110	С	2.8	2.8+ 2A+P		150	150	10.2 + C + A	A2	MAX: 7.4 + C (ROADSIDE)	B1
RAD-412.4-120	4	120	C	C	-		-		7.4 + 2C	A3	MAX: 7.4 + C (ROADSIDE)	B1
RFD-412.4-130	4	130	С	C		-	-		7.4 + 2C	A3	MAX: 7.4 + C (ROADSIDE)	B1
RFD/RAD-616.6-130	6	130	С	С		-			11.1 + 2C	A3	MAX: 11.1 + C (ROADSIDE)	B1

** SEE NOTE 5

TABLE 3 - BARRIER GEOMETRIC PARAMETERS FOR " URBAN " CROSS SECTIONS

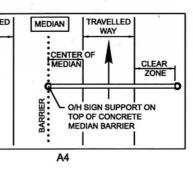
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AIT	ROADWAY	DESIGN	OBSTACLE	OBSTACLE	OBSTACLE	LENGTH	LENGTH	LENGTH	OVERHEAD SIGN	BRIDGE	CANTILEVER SIG	BN
DESIGN DESIGNATION	FACILITY (No. OF LANES)	SPEED (km/hr)	OFFSET ** X1 (m)	OFFSET ** X2 (m)	OFFSET ** X3 (m)	OF NEED Y1 (m)	OF NEED Y2 (m)	OF NEED Y3 (m)	CLEAR SPAN Z1 (m)	SCENARIO REFERENCE	CLEAR SPAN Z2 (m)	SCENARIO REFERENCI
UCD-408.4-60	4	60	-	-	-	-		-	-		•	-
UAD/UCD-409.9-70	4	70	· ·	-	-	-	-	-	-		-	-
UAD-410.4-80	4	80	C	C	•	-		-	7.4 + 2C	A3	MAX: 7.4 + C (ROADSIDE)	B1
UAD-410.4-100	4	100	C	2.4		-	150		9.8 + C + A	A1	MAX: 7.4 + C (ROADSIDE)	B1
UAD-410.4-110	4	110	C	2.8	2.8 + 2A + P	-	150	150	10.2 + C + A	A2	MAX: 9.4 (MEDIAN)	B2
UAD-411.4-80	4	80	C	С		-			7.4 + 2C	A3	MAX: 7.4 + C (ROADSIDE)	B1
UFD-411.9-80	4	80	С	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	9.9 + C	A4	MAX: 9.4 (MEDIAN)	B2
UAD-411.4-100	4	100	C	2.4	•	-	150		9.8 + C + A	A1	MAX: 7.4 + C (ROADSIDE)	B1
UFD-411.9-100	4	100	C	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	9.9 + C	A4	MAX: 9.4 (MEDIAN)	B2
UAD-411.4-110	4	110	С	2.8	2.8 + 2A + P	-	150	150	10.2 + C + A	A2	MAX: 9.4 (MEDIAN)	B2
UAD-613.1-60	6	60	С	FLARED	FLARED	-	100	100	13.6 + C	A4	MAX: 11.1 + C (ROADSIDE)	B1
UAD-613.1-80	6	80	C	FLARED	FLARED	-	100	100	13.6 + C	A4	MAX: 11.1 + C (ROADSIDE)	B1
UAD/UFD-616.6-80	6	80	C	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	14.6 + C	A4	MAX: 14.6 (MEDIAN)	B2
UAD/UFD-616.6-100	6	100	C	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	14.6 + C	A4	MAX: 14.6 (MEDIAN)	B2
UAD/UFD-616.6-110	6	110	C	2.8	-	-	150	•	11.1 + C + A	A1	MAX: 11.1 + C (ROADSIDE)	B1
UAD/UFD-616.6-110	6	110	С	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	14.6 + C	A4	MAX: 14.6 (MEDIAN)	B2
UAD/UFD-616.6-120	6	120	C	3.2	3.2 + 2A+ P	-	150	150	11.1 + C + A	A2	MAX: 11.1 + C (ROADSIDE)	B1
UAD/UFD-820.8-100	8	100	C	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	18.2 + C	A4	MAX: 18.2 (MEDIAN)	B2
UAD/UFD-820.8-110	8	110	C	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	18.2 + C	A4	MAX: 18.2 (MEDIAN)	B2
UAD/UFD-820.8-130	8	120	C	FLARED	FLARED	-	CONTINUOUS	CONTINUOUS	18.2 + C	A4	MAX: 18.2 (MEDIAN)	B2

** SEE NOTE 5

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ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHERWISE INDICATED.

APPENDIX B7



NOTES:

4.

5.

1. SETBACK DISTANCE *A* INCLUDES WIDTH OF BARRIER PLUS A DYNAMIC DEFLECTION ALLOWANCE.

2. FOR RIDGED BARRIERS, REFER TO SECTION H.5.5 OF THE ROADSIDE DESIGN GUIDE FOR ZONE OF

INTRUSION CONSIDERATIONS.

3. "P" INDICATES THE OVERHEAD SIGN SUPPORT DIAMETER.

C INDICATES SITE SPECIFIC CLEARANCE DISTANCE, REFER TO SECTION H.3.2.1 OF THE ROADSIDE DESIGN GUIDE.

FOR OBSTACLE OFFSET FROM TRAVELLED WAY, REFER TO SHEET 1 OF 2. THE OBSTACLE COULD BE AN OVERHEAD SIGN SUPPORT OR BARRIER, WHICHEVER IS CLOSER TO THE TRAVELLED WAY. IF THE OBSTACLE IS AN OVERHEAD SIGN SUPPORT, THEN x ≥ C. IF THE OBSTACLE IS A BARRIER THEN x ≤ C (GENERALLY) IN ALL CASES 'x' SHALL BE GREATER THAN THE SHYLINE OFFSET UNLESS OTHERWISE DICTATED BY UNIQUE SITE SPECIFIC CIRCUMSTANCES

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	PLACEMENT AND PROTECTION OF OVERHEAD SIGN SUPPORTS FOR DIVIDED ROADS											
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