GEOHAZARD ASSESSMENT PROGRAM

NORTH CENTRAL REGION – ATHABASCA



THURBER ENGINEERING LTD. GEOTECHNICAL # ENVIRONMENTAL # MATERIALS

2009 INSPECTION

Site Number	Locatio	Location			Name				Hwy	km	
NC X	6.5 km w between H the north c	6.5 km west o between Hwy 8 the north of Lac		unction 858 to he			E	858:02	45.85		
Legal Descript		UTI	M Co-or	dinates	(NA	D 83)					
NW- 30-68-13-W		12 N 6085029 E 4						1			
		1	Date		PF	CE			Tot	al	
Previous Inspection:			Sep. 18, 2008	3	9	2			18		
Current Inspection:		Ι.	June 22, 2009		10	3	3		30		
Road AADT:			360)		Year:			2008		
Inspected By:		Tai Ro	Tarek Abdelaziz, Don Proudfoot (Thurber) Roger Skirrow, Arthur Kavulok, Jake Knudslein (TRANS)								
Report Attachments:		•	Photographs Plans Maintenance							ltems	
Primary Site Issue: Dimensions:			An active landslide occurred after a heavy rainfall event, causing severe pavement distress on the eastbound lane of the highway About 65 m along the highway and 22 m perpendicular to the								
			highway centerline								
Date of any remediation:											
Maintenance:			September 2008, September 2009 (after June, 2009 site visit), and probably in October 2009 according to recent correspondence with TRANS.								
Observations:			Description							Worse?	
Pavement Distress			New pavement overlay on the highway eastbound and westbound lanes. Up to 15 mm wide cracks on the EBL. 5-10 mm wide cracks on the WBL. 5 mm dip noted on the EBL at the western limit of the slide								
Slope Movement			Distortion of the fence and the muskeg ditch on the south side of the highway since 2008; graben counterscarp crack width has increased by 125 mm since 2008 visit; titling trees by the fence							~	
Erosion											
✓ Seepage			Water ponding in the ditch between the toe of the south side slope and the graben counterscarp crack								
Bridge/Culvert Distress											
✓ Other			The beaver dams located further to the east of the highway were cleared to enhance the surface drainage at the site								
Instrumentatio	on: None										

Assessment (Refer to attached Figure):

The site conditions appeared to have deteriorated due to the continued slide movement. This is evident from re-opening of cracks on highway, the severe distortion along the muskeg ditch, the fence, and widening of the graben counterscarp crack by additional 125 mm since last year, which are all indicative of the accelerated movement of the lower slide block. The infiltration of surface water (trapped in the highway ditch) into the open graben counterscarp crack probably contributed to movement acceleration. The appearance of new cracks on the WBL reflects the tendency of the slide to retrogress to the WBL of the highway. The site features indicate that the slide is probably a shallow translational landslide extending to the south of the fence.

Recommendations:

According to recent correspondence with Mr. Kavulok in November 2009, the highway conditions deteriorated significantly after the site visit and the highway was re-patched in September, 2009. However, severe cracking and pavement distress were noted on the EBL in October 2009. It is understood that re-patching the highway was planned for November 2009.

In the short term, the MCI should continue to seal any open cracks on the highway surface and re-patch the highway as required to improve the quality of ride to a minimum level. Smoothing out the ditch and filling the graben counterscarp crack is important to enhance surface drainage and reduce the likelihood of further surface water infiltration into the crack. It is also recommended to undertake a geotechnical investigation program (consisting of installing 2SIs and 2PNs, preliminary survey and slope stability analyses) to understand subsurface and ground water conditions, and explore potential remedial measures.

After the implementation of the geotechnical investigation program, possible long term remedial measures might include either of the following:

- (1) Excavation and Replacement of slide mass at a ball park cost of \$750,000, excluding land acquisition. This will require some slope flattening, gravel fill for drainage, and a shear key.
- (2) Construction of a toe berm at a ball park cost of \$500,000, excluding land acquisition.
- (3) Construction of a pile wall at a ball park cost of \$700,000.

Improvements to surface drainage are likely required with each of the above options.