## LEGEND

Area of cracking /
settlement $\square$
Slope Inclinometers


Note: Scale and Instrument locations are approximate.



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Area of cracking /
settlement $\square$
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## Bale, Bryan

From:
Sent:
To:
Cc:
Subject:
Attachments:

Clay, Tyler
Monday, July 18, 2011 5:31 PM
Ross Dickson
Bidwell, Andrew; Bale, Bryan
S38-Callum Creek, Hwy 22 Site Conditions (July 2011)
July2011_CallumCk_Figures.pdf

## Hello Ross,

This email is intended to serve as a summary of the recent instrument data and observations from the July 14, 2011 visit to the S38-Callum Creek site on Highway 22.

General Observations:
The highway appeared to have a recent overlay patch (within the last month or so) along the section where settlement and cracking had been previously observed (approximately $70-100 \mathrm{~m}$ length). There were some fresh cracks through the new overlay on the west side of the highway shoulder, approximately 20 m in length with aperture less than 50 mm (see attached photos). There was also roughly 10 mm or less of settlement of a portion of the road within the overlay area (see attached photos). The shape and position of the settled area was consistent with past observations of arcshaped cracks and settlement. There was also some cracking visible on the east side of the road near SI 11-02 for approximately $10-15 \mathrm{~m}$ and with an aperture less than 20 mm .

Instrument Data (see also attached data plots):

- SI 11-01: Since the April 12, 2011 initialization this SI has been read twice (May $2^{\text {nd }}$, July $14^{\text {th }}$ ). Prior to the July $14^{\text {th }}$ readings there were no confirmed movement zones. The July $14^{\text {th }}$ data shows a confirmed downslope movement zone at approximately 12 m depth with 31 mm displacement at $43^{\circ}$ bearing.
- SI 11-02: Since the April 12, 2011 initialization this SI has been twice (May $2^{\text {nd }}$, July $14^{\text {th }}$ ). Prior to the July $14^{\text {th }}$ readings there were no confirmed movement zones. On July $14^{\text {th }}$, this SI was found to be pinched off at approximately 9.40 m below ground surface.
- VW 10-6697: This piezometer tip is installed approximately 14 m BGS (below ground surface). This instrument has recorded small pressures since it was installed, i.e. the equivalent groundwater table would be only slightly above the piezometer tip.
- VW 10-6710: This piezometer tip is installed approximately 6 m BGS . This instrument has recorded a pressure approximately 37 kPa higher than the previous reading in May 2011. This current pressure corresponds to an equivalent piezometric elevation 4.55 m above the tip elevation, i.e. roughly 1.5 m BGS.


## Comments and Recommendations

This site has active and significant landslide movement, based on the continued settlement and cracking visible in the road surface through recent overlay patches and the SI data. The depth of the movement zones in the Sl's roughly corresponds to a layer of higher plastic clay with possible slickensides that was identified during the instrument installation drilling. The data from the vibrating wire piezometers indicates that there are potentially zones of higher water pressure within the slope. These water pressures could be a contributing factor that is driving the active movement at this site.

## Recommended short-term actions:

- Continue placing overlays and/or milling down and resurfacing the asphalt to maintain a trafficable surface through the slide area but without accumulating an excessive load from repeated overlays (i.e. loading the upper portion of the landslide area).
- Maintain signage (e.g. "Bump Ahead") along with reduced speed limit as appropriate for the road surface conditions.
- There is a possibility that additional or accelerated landslide movement could lead to the loss of one of both lanes of the highway and the construction of a temporary detour lane in the upslope road ditch may be required.

Recommended medium to long-term actions:

- Select and design a repair measure for this site based on the information from the borehole drilling and instrument installations. The most practical and reliable repair measure for this site would likely be a pile wall similar to the one constructed at the other landslide area a short distance northbound from this site. Drainage measures (e.g. horizontal drains and/or impermeable lining of the upslope road ditch to reduce water infiltration into the slope) may also be beneficial. The pros/cons and cost/benefit of these and other repair options could be assessed with a slope stability analysis.

Please let us know if you require any further details re. the information above.
Regards,
Tyler

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