

GENERAL NOTES

MINIMUM PILE BEARING CAPACITY						
END SPAN	m	6	8	10	12	14
PILE BEARING CAPACITY	(kN)	300	340	380	420	460

- ALL DRAWING REFERENCES REFER TO CURRENT DRAWINGS.
- ALL DIMENSIONS ARE GIVEN IN mm UNLESS NOTED OTHERWISE.
- ROADWAY ELEVATIONS SPECIFIED ELSEWHERE ARE GIVEN TO TOP OF THEORETICAL CROWN ON CENTRELINE ROADWAY.

GEOTECHNICAL CONSIDERATIONS

- IT IS THE RESPONSIBILITY OF OTHERS USING THESE SUBSTRUCTURE DRAWINGS TO OBTAIN GEOTECHNICAL INFORMATION APPLICABLE TO THE SPECIFIC SITE AT WHICH THE SUBSTRUCTURE WILL BE CONSTRUCTED. A SITE SPECIFIC GEOTECHNICAL INVESTIGATION COMPLETED BY A QUALIFIED PROFESSIONAL ENGINEER SHOULD BE UNDERTAKEN PRIOR TO THE DESIGN PHASE IN ORDER TO ESTABLISH THE SUITABILITY OF THE DESIGN ASSUMPTIONS LISTED ON THESE PLANS, AND TO PROVIDE SUPPLEMENTARY INFORMATION WHERE REQUIRED.
- ADDITIONALLY, A GEOTECHNICAL INVESTIGATION IS DEEMED NECESSARY IN ORDER TO ESTABLISH THE SOIL CHARACTERISTICS AT THE SITE - THE PRESENCE AND THE EXTENT OF BOULDERS, HARD OR SOFT FORMATIONS, WATER TABLES, ARTESIAN CONDITIONS AND OTHER VARIABLES - AS THESE COULD SIGNIFICANTLY IMPACT THE CONSTRUCTION OF THE SUBSTRUCTURE AND ITS REQUIRED PERFORMANCE AND STRENGTH IF NOT IDENTIFIED AND ADDRESSED IN A TIMELY MANNER.

HYDROTECHNICAL CONSIDERATIONS

- IT IS THE RESPONSIBILITY OF OTHERS USING THESE SUBSTRUCTURE DRAWINGS TO COMPLETE A SITE SPECIFIC HYDROTECHNICAL ASSESSMENT IN ORDER TO ESTABLISH THE NEED FOR RIPRAP, TO DETERMINE THE SUITABILITY OF THE DESIGN ASSUMPTIONS LISTED ON THESE PLANS, AND TO SUPPLEMENT THEM WHERE REQUIRED. CONSULTATION WITH A QUALIFIED PROFESSIONAL ENGINEER WITH HYDROTECHNICAL EXPERTISE PRIOR TO THE DESIGN PHASE IS RECOMMENDED.

DESIGN

- **DESIGN SPECIFICATION:** CAN/CSA-S6-06
- **DESIGN VEHICLE:** CL-800, HIGHWAY CLASS A FOR FATIGUE
- **DEAD LOAD:** ABUTMENT SELF WEIGHT, GIRDER SELF WEIGHT AND WEARING SURFACE ALLOWANCE AS PER STANDARD SL-510 GIRDER DRAWING S-1749
- **EARTH PRESSURE:** THE FOLLOWING LIMITING ASSUMPTIONS WERE USED FOR SUBSTRUCTURE DESIGN. THESE PLANS SHOULD NOT BE USED WHEN ACTUAL SITE CONDITIONS RESULT IN MORE SEVERE LOAD EFFECTS OR LESS EFFECTIVE RESISTANCE:
 - UNIT WEIGHT OF SOIL $\gamma = 22 \text{ kN/m}^3$
 - COEFFICIENT OF ACTIVE PRESSURE, $k_a = 0.50$ (NON-FACTORED)
 - COEFFICIENT OF PASSIVE PRESSURE, $k_p = 2.10$ (NON-FACTORED)
 - MAXIMUM ABUTMENT HEIGHT: 2.5 m
 - MAXIMUM ABUTMENT SKEW: 45°
 - DEPTH BELOW ABUTMENT PILE FILL LINE TO EFFECTIVE PILE FIXITY = 2.0 m
- **SEISMIC:** $Z_a = 1$, $a = 0.05$ $Z_v = 1$, $v = 0.05$
- **THERMAL:** MAXIMUM MEAN DAILY TEMPERATURE = 35°C
MINIMUM MEAN DAILY TEMPERATURE = -45°C
- **BRAKING FORCE:** CONSIDERED

CONSTRUCTION

- ALL CONSTRUCTION WORK SHALL CONFORM TO THE CURRENT SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- TREATED TIMBER SHALL BE PROPERLY HANDLED TO AVOID BRUISING, BREAKING OR PENETRATION OF OUTER FIBRES. LIFTING TOOLS SHALL BE APPLIED ONLY ON ENDS OF TREATED TIMBER PIECES. ALL CUTS AND BRUISES SHALL BE CAREFULLY TRIMMED. COMPATIBLE WOOD PRESERVING COMPOUND SHALL BE APPLIED TO ALL CUTS, BRUISES, FIELD CUTS AND BOLT HOLES IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
- STRUCTURAL PLATE CORRUGATED STEEL (SPCS) PLATES SHALL BE ARRANGED SO THAT NO VERTICAL SEAMS INTERFERE WITH PILE LOCATIONS.
- PILES SHALL BE DRIVEN TO THE TIP ELEVATIONS SHOWN ON THE DRAWINGS, OR LOWER, TO ACHIEVE AT LEAST THE SPECIFIED MINIMUM BEARING CAPACITY. BEARING CAPACITIES SHALL BE ESTIMATED USING THE BEARING FORMULA PRESENTED IN THE CURRENT SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- AFTER PILE DRIVING OPERATIONS HAVE STARTED, THE CONSULTANT MAY REVISE THE REQUIRED PILE TIP ELEVATIONS, PROVIDED THE MINIMUM SPECIFIED BEARING CAPACITY AND MINIMUM PENETRATION HAVE BEEN ACHIEVED.
- ALL BEARING PILES SHALL PENETRATE TO AT LEAST 5 m BELOW STREAMBED.

BACKFILL

- THE CONTRACTOR SHALL BACKFILL ABUTMENTS WITH CARE AND IN ACCORDANCE WITH SPECIFICATIONS.
- THE CONTRACTOR SHALL MAINTAIN OVERALL BACKWALL STABILITY, LIMIT DEFLECTIONS DURING BACKFILLING AND ENSURE PROPER GIRDER INSTALLATION.
- IF IT IS NECESSARY TO PARTIALLY BACKFILL PRIOR TO ANCHOR PILE SYSTEM INSTALLATION, THE CONTRACTOR SHALL PROVIDE A RESTRAINT SYSTEM AND MONITOR WALL DEFLECTIONS WITH ADEQUATE INSTRUMENTATION DURING BACKFILLING.
- THREADED RODS SLACK SHALL BE TAKEN OUT PRIOR TO BACKFILLING.

GIRDER INSTALLATION

- REFER TO TYPE SL-510 GIRDER DRAWINGS (LATEST REVISION) AS APPLICABLE:

6 m S-1723 AND S-1724	12 m S-1729 AND S-1730
8 m S-1725 AND S-1726	14 m S-1731 AND S-1732
10 m S-1727 AND S-1728	ALL SPANS S-1749

- GIRDERS SHALL BE CONNECTED TOGETHER WITH 20 mm ϕ A325 BOLT ASSEMBLIES, C/W DROP-IN WASHERS TO FILL THE GAP BETWEEN GIRDERS, TORQUED TO 400 Nm. GIRDERS SHALL NOT TOUCH EXCEPT THROUGH DROP-IN WASHERS. BOLTS AND WASHERS TO BE HOT-DIPPED GALVANIZED.
- CONNECTOR AND LIFTING HOOK POCKETS SHALL BE FILLED WITH A DEPARTMENT APPROVED CONCRETE PATCHING MATERIAL.

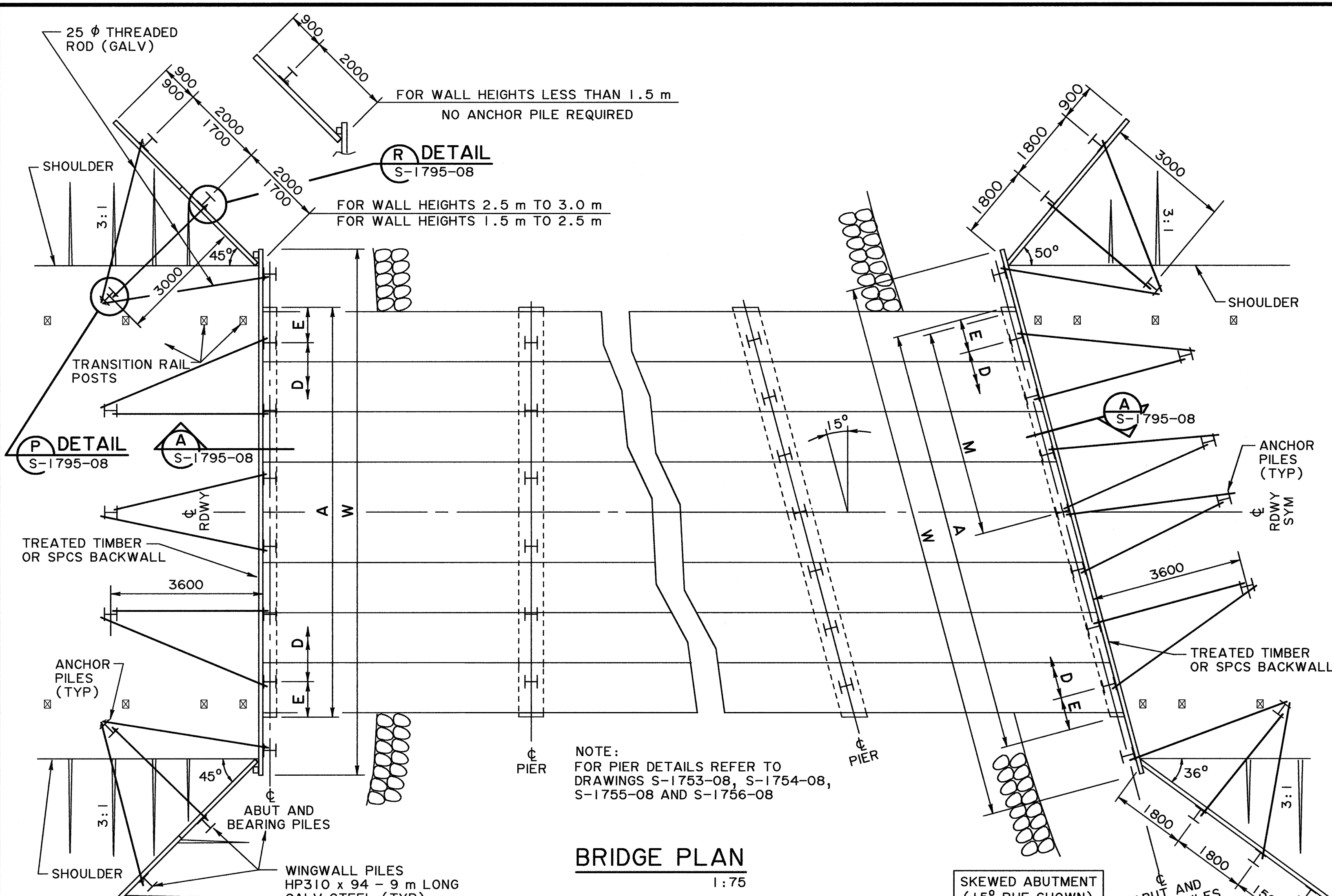
MATERIALS

- GIRDER DOWELS SHALL CONFORM TO THE REQUIREMENTS OF CSA G40.21M GRADE 300W AND HOT-DIPPED GALVANIZED AFTER FABRICATION.
- ALL STEEL PLATE AND SHAPES SHALL CONFORM TO THE REQUIREMENTS OF CSA G40.21M GRADE 350W.
- THREADED RODS SHALL BE EITHER WILLIAMS SUPER - HIGH TENSILE 'V' THREADED ROD OR ACROW RICHMOND HIGH TENSILE CONTINUOUS THREADED ROD. THREADED RODS SHALL HAVE ROLLED THREADS AND A MINIMUM YIELD STRENGTH OF 620 MPa. RODS SHALL BE HOT-DIPPED GALVANIZED.
- PILE CAPS SHALL BE HOT-DIPPED GALVANIZED. GALVANIZING MAY BE OMITTED FROM PILE SURFACES LOCATED MORE THAN 1000 mm BELOW GROUNDLINE. ALL FIELD WELDS OF GALVANIZED MATERIAL SHALL BE METALIZED.
- GALVANIZING SHALL CONFORM TO THE CURRENT CSA STANDARD G164.
- ALL WELDING SHALL CONFORM TO THE CURRENT AWS SPECIFICATION D1.5.
- WELDING IN LOW TEMPERATURES SHALL BE DONE IN ACCORDANCE WITH SECTION 13.4.1 OF THE SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- GALVANIZED SURFACES DAMAGED BY FIELD WELDING SHALL BE THOROUGHLY CLEANED AND METALIZED.
- TREATED TIMBER (TT) SHALL CONFORM TO SECTION 23 OF THE SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- STRUCTURAL PLATE CORRUGATED STEEL (SPCS) MATERIAL SHALL CONFORM TO SECTION 18 OF THE SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- GRANULAR BACKFILL SHALL BE PLACED TO A MINIMUM OF 95% PROCTOR DENSITY AND OTHERWISE CONFORM TO THE SPECIFICATIONS FOR BRIDGE CONSTRUCTION.
- BEARING PADS SHALL CONSIST OF NEOPRENE 60 HARDNESS AND SHALL CONFORM TO SECTION 18 "BEARING DEVICES" DIVISION II OF AASHTO DESIGN STANDARD.

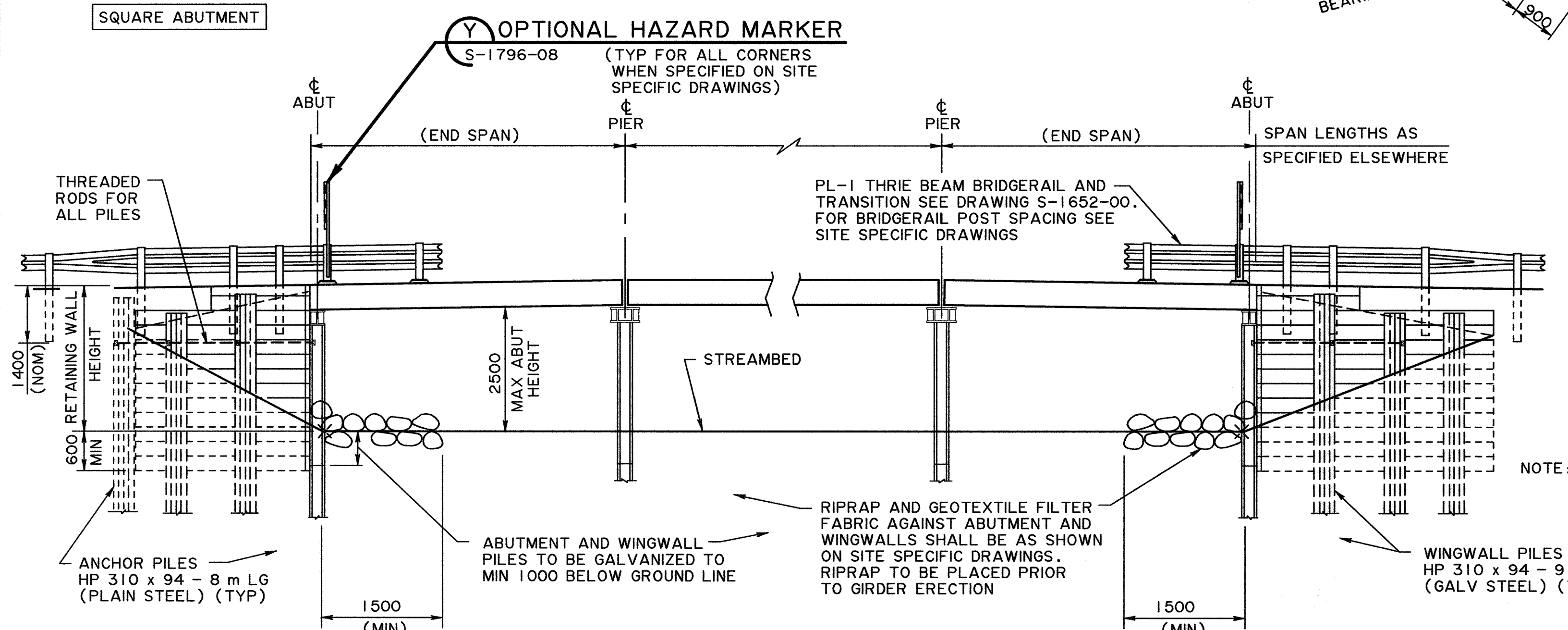
ADDITIONAL NOTES

- PIER NOTE: FOR PIER DETAILS REFER TO CURRENT STANDARD DRAWINGS S-1753-08, S-1754-08, S-1755-08 AND S-1756-08.
- ALL NOTES ON S-1753-08 SHALL APPLY EXCEPT AS MODIFIED ON THIS DRAWING.

• WORK DRAWINGS S-1793-08, S-1794-08, S-1795-08 AND S-1796-08 TOGETHER WITH SITE SPECIFIC GENERAL LAYOUT



BRIDGE PLAN
1:75



BRIDGE ELEVATION
(TREATED TIMBER SHOWN SPCS SIMILAR)
1:75

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W. POLANCA
DATE: Aug 15, 2008

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PROFESSIONAL ENGINEER ALBERTA
D. F. SPANER
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REV	DATE	REVISIONS	BY

RECOMMENDED
DIRECTOR BRIDGE ENGINEERING
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TECHNICAL STANDARDS BRANCH
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DATE: Sept 8/08

Alberta Transportation

SL PRECAST GIRDER BRIDGES WITH HIGH BACKWALL STEEL ABUTMENTS - SHEET 1

AT BAR CODE	DATE	SHEET	DRAWING
	2008-08-18	1 of 4	S-1793-08