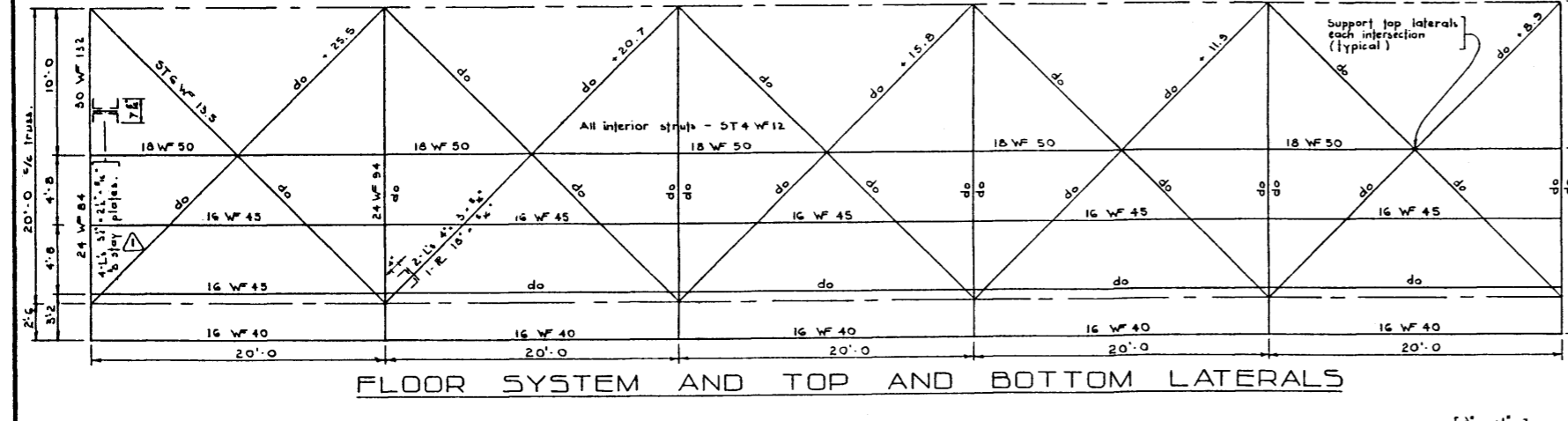
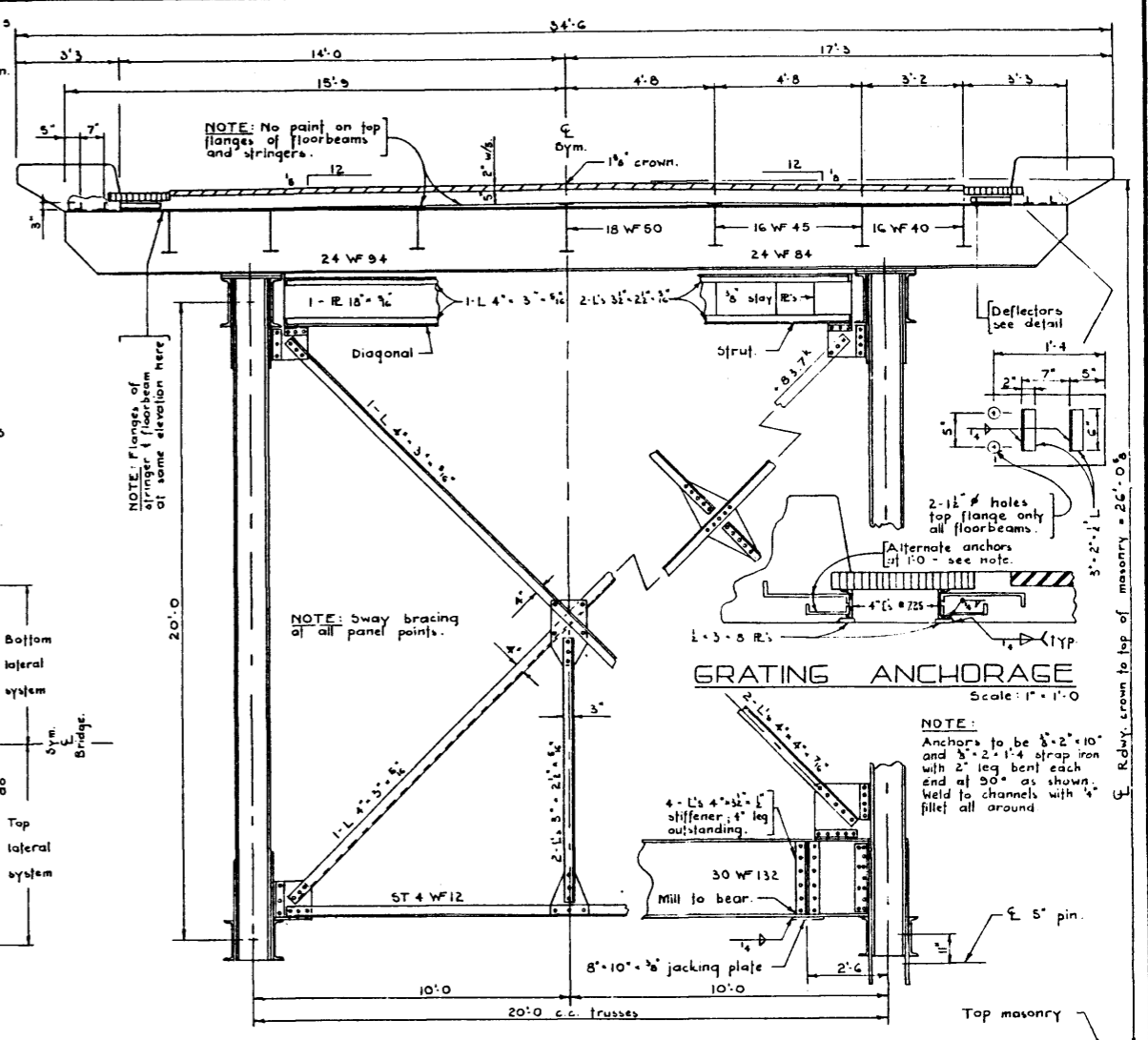
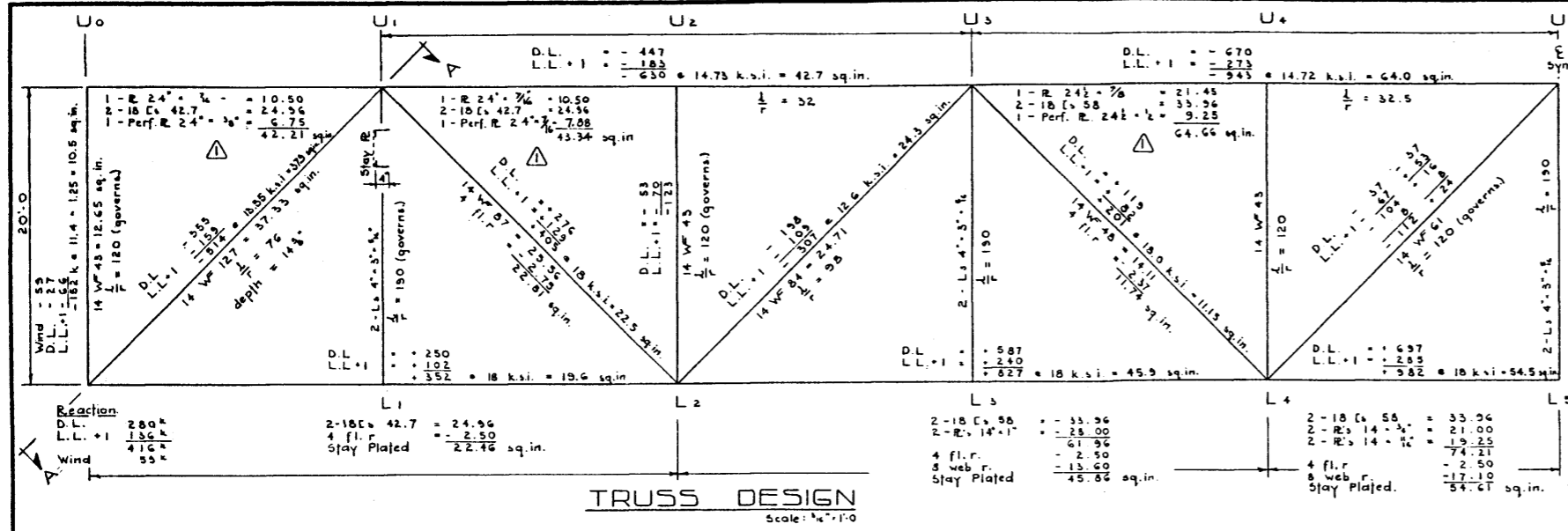


DESIGNED BY: Henry H. Hendrickson
 CHECKED BY: Douglas W. Swicki
 DATE: 22 September 1950
 DATE: 12 October 1950



JACKING BEAM
 Jacking load = 280 k
 Moment = 280 x 2.5 = 700 k-ft.
 5 req'd @ 15.4 x 1.5 k.s.i. = 364 cu.in.
 30 W 132 = 379.7 cu.in.
 S req'd = 22.4

INTERIOR FLOOR BEAM
 Shear D.L. = 27 k
 L.L. = 57 k
 84 k @ 11 k.s.i. = 7.64 sq.in.
 Moment D.L. = 26
 L.L. = 284
 5 req'd @ 310 k @ 18 k.s.i. = 207 cu.in.
 24 W 94 [web area = 12.4 sq.in. Section mod. = 220.9 cu.in. L.L. deflection = 0.24 in.]

END FLOOR BEAM
 Shear D.L. = 14 k
 L.L. = 54 k
 68 k @ 11 k.s.i. = 6.5 sq.in.
 Moment D.L. = 13 k-ft
 L.L. = 252
 5 req'd @ 282 k @ 18 k.s.i. = 189 cu.in.
 24 W 94 [web area = 11.3 sq.in. Section mod. = 196.3 cu.in. L.L. deflection = 0.26 in.]

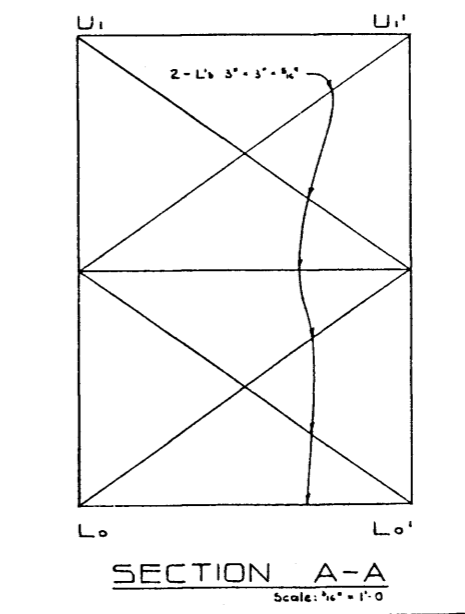
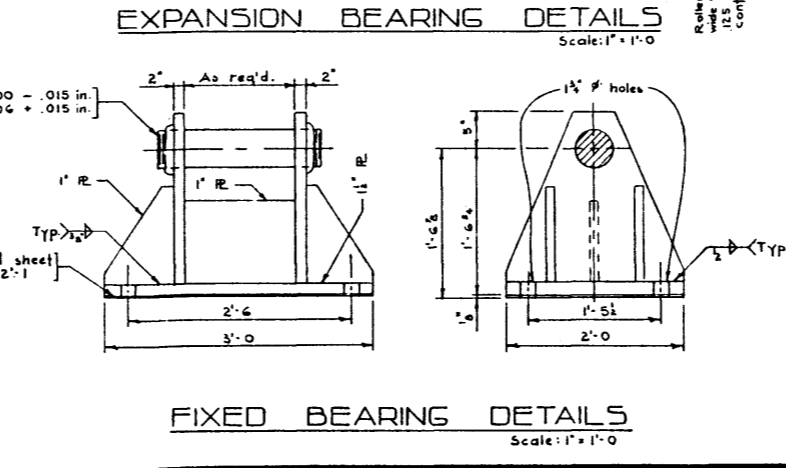
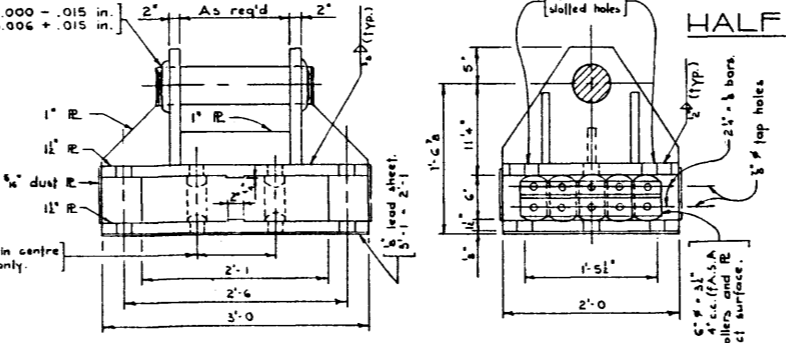
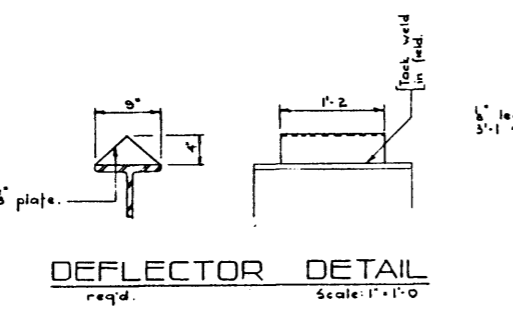
INTERIOR STRINGERS
 Shear D.L. = 4.6
 L.L. = 23.0
 27.6 k @ 11 k.s.i. = 2.5 sq.in.
 Moment D.L. = 23.4 k-ft
 L.L. = 87.0
 5 req'd @ 110.0 k @ 18 k.s.i. = 73.4 cu.in.
 16 W 43 [web area = 3.9 sq.in. Section mod. = 72.4 cu.in.]

NOTE: Make centre stringer 18 W 50 (flange hole for one 1/4" hanger bolt.)

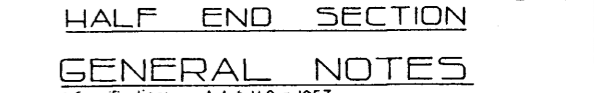
EXTERIOR STRINGERS
 Shear D.L. = 2.3
 L.L. = 23.0
 25.3 k @ 11 k.s.i. = 2.3 sq.in.
 Moment D.L. = 11.3
 L.L. = 87.0
 5 req'd @ 58.5 k @ 18 k.s.i. = 65.1 cu.in.
 A-7 16 W 40 [web area Section mod. = 4.5 sq.in. = 64.4 cu.in.]

BEARING
 Reaction D.L. = 280 k
 L.L. = 416 k
 84 k @ 11 k.s.i. = 7.64 sq.in.
 Bearing R = 416 sq.in.
 6" rollers @ 115 lin in req'd

PIN BEARING
 Bearing A req'd @ 416 / 24 = 17.3 sq.in.
 A supplied 20
 Shear A req'd @ 416 / 23.5 = 15.4 sq.in.
 Bending b req'd = 9.8 cu.in.
 b supplied = 12.3 cu.in.



NO.	DATE	DESCRIPTION	BY
1	25 Nov. 1950	Change to slay plates; top chord	H.H.H.



GENERAL NOTES

- Specifications - A.A.S.H.O - 1957
- Live load - H20-516-44
- Dead load - asphalt and concrete - 1,710
- floor steel and connections - 260
- handrail - 35
- truss and bracing - 730
- 2,735 lbs/ft/truss.
- Wind load - 350 lbs/ft for top chords.
- 200 lbs/ft for bottom chords.
- Connections:
 - Shop - all main components to have 5/8" rivets
 - all bracing to have 3/4" rivets.
 - Field - all main components to have 3/4" high strength bolts.
 - all bracing to have 1/2" high strength bolts.
- Bolts, nuts, and washers shall conform to A.S.T.M. - A325 - 58 T.
- Gussets - 1/4" min. for main members.
- 3/16" min. for bracing members only.
- Shop paint - all steel, except surfaces in contact with steel or concrete and machined surfaces to receive one coat red lead, iron oxide, oil alkyd type paint conforming to the requirements of the Canadian Government Specification Board: Spec. I-GP-140 A.
- Camber - camber for full D.L. & L.L.
- Bearings - welded using the metal arc process.
- Steel shall conform to A.A.S.H.O - M165 - 57 I (A.S.T.M. - A375 - 56 T) where welding req'd.
- Steel in pins and rollers shall conform to A.A.S.H.O - M102 - 57-1 (A.S.T.M. - A235 - 55) (Class C1).
- Structural steel shall conform to A.A.S.H.O - M94 - 57-1 (A.S.T.M. - A7-56 T).

200' DECK TRUSS STRESSES - SECTIONS - DETAILS (A-7 STEEL)

GOVERNMENT OF THE PROVINCE OF ALBERTA
 DEPARTMENT OF HIGHWAYS
 BRIDGE BRANCH, EDMONTON

FILE NO. _____ HWY. NO. _____ DWG. NO. _____
 LOCATION _____ SCALE _____ SHOWN _____
 SHEET _____ OF _____

5-728