



Alberta Transportation 4th Floor, Provincial Building 4920 51 Street Red Deer, Alberta T4N 6K8

Tony Penney, P.Eng. Construction Engineer

Dear Mr. Penney:

CON0022160 Central Region GRMP Instrumentation Monitoring Site C043; H619:06, km 12.206 Soil Nail Site Section C – 2022 Spring Readings

1 **GENERAL**

Two vibrating wire piezometers (VWPs) (VW46428 and VW46429) and two slope inclinometers (SIs) (SI21-C43-01 and SI21-C43-02) were read at the C043 site in the Central Region on June 23, 2022 by Mr. Guerin White, E.I.T. of Klohn Crippen Berger Ltd. (KCB). These instruments were read as part of the Central Region Geohazard Risk Management Program (GRMP). The site is located on Hwy 619:06, km 12.2, approximately 24 km southeast of Vermilion, Alberta. The approximate site coordinates are 5890276 N, 520018 E (UTM Zone 12, NAD 83). A site plan is presented in Figure 1.

The geohazard at the C043 site consists of an embankment slope failure on the north side (westbound lane) of Hwy 619:06 and a backslope failure on the south side (eastbound lane) of Hwy 619:06.

In November 2017, KCB conducted a geotechnical site investigation at the C043 site. Drilling was completed by Mobile Augers and Research Ltd. The encountered stratigraphy was as follows: low to medium plastic silty clay till overlying weathered clay shale. The encountered stratigraphy was consistent with the stratigraphy encountered during a 2013 drilling investigation.

In March 2021, a 12-m deep, 63-m long H-pile wall (H360X132) consisting of 106 piles was installed on the north side (westbound lane) of the highway. Previous remedial actions include soil nails installed through the embankment slope failure on the north side (westbound lane) of the highway in 2006.

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1.1 Instrumentation

Instrumentation installation details are tabulated in Table 1.1. Instrument locations are shown in Figure 1.

In June 2013, KCB installed one SI (SI13-01) and one standpipe piezometer (SP) (SP13 -01) in the westbound lane of Hwy 619:06. The instruments were destroyed in 2014 due to ongoing road maintenance (i.e., pavement patching).

In November 2017, KCB installed two VWPs (VW46428 and VW46429) to monitor groundwater conditions. The VWPs were installed in a borehole (BH17-C43-01) located in the south ditch of Hwy 619:06.

On March 23, 2021, KCB installed two SIs (SI21-C43-01 and SI21-C43-02) in steel pockets between the web and flanges of the 2021 H-piles to monitor deflection of the H-pile wall. SI21-C43-01 and SI21-C43-02 are located approximately one-third (21 m) from either end of the H-pile wall.

The VWPs are protected by an above-ground casing protector. However, the SIs do not have casing protectors.

The VWPs were read using an RST VW2106 vibrating wire readout.

The operable SIs were read using the same metric RST Digital MEMS Inclinometer System that has been used to the read the SIs since they were installed.

Table 1.1 Instrumentation Installation Details

Instrument	Instrument Instrument ID Type	Date Installed	UTM Coordinates ¹ (m)		Ground Surface	Stick Up	Depth	Condition
ID			Northing	Easting	Elevation ¹ (m)	(m)	(mbgs ²)	
SI13-01	SI	Jun. 20, 2013	5890277	520006	607	-0.1	14.5	Inoperable ³
SI21-C43-01	SI	Mar. 23, 2021	5890285	520009	Unknown	0.1	10.9	Operable
SI21-C43-02	SI	Mar. 23, 2021	5890285	520024	Unknown	0.1	11.4	Operable
SP13-01	SP	Jun. 20, 2013	5890277	520008	607	0.0	14.5	Inoperable ³
VW46428	VWP	Nov. 20, 2017	5890266	520007	COF	N/A	6.5	Operable
VW46429	VWP	Nov. 20, 2017			605	N/A	12.3	Operable

Notes:

¹ Coordinates and ground surfaces elevations for the instruments installed in 2017 were obtained with a handheld GPS during installation.

² Meters below ground surface (mbgs).

³ SI13-01 and SP13-01 were destroyed in 2014 due to ongoing road maintenance (i.e., pavement patching).

2 INTERPRETATION

2.1 General

For the operable SIs, the cumulative displacement, incremental displacement, and displacement-time data was plotted in the A-direction (i.e., the direction of the A0-grooves). The A0-grooves in the SIs are aligned approximately with the direction of maximum movement, in the downslope direction.

For the VWPs, the recorded porewater pressures were converted to an equivalent water/piezometric elevation and plotted relative to ground surface elevation and each instruments tip elevation.

The SI and piezometer plots are included in Appendix I, and a summary of the SI and piezometer data is provided in Table 2.1 and Table 2.2, respectively.



Table 2.1Slope Inclinometer Reading Summary

	Date					Donth of		Movement (mm)		Rate of Movement (mm/year)		
Instrument ID	Initialized	Previous Maximum Cumulative Movement Recorded	Previous Reading	Most Recent Reading	Ground Surface Elevation (m)	Depth of Movement (mbgs ¹)	Direction of Movement	Maximum Cumulative	Incremental Since Previous Maximum Cumulative	Previous Maximum	Most Recent Reading	Change from Previous Reading
SI21-C43-01	Apr. 22, 2021	Jun. 11, 2021	Jun. 11, 2021	Jun. 23, 2022	Unknown	0.0 - 7.0	A-Direction	4.6	3.0	11.7	2.9	-8.8
SI21-C43-02	Apr. 22, 2021	Jun. 11, 2021	Jun. 11, 2021	Jun. 23, 2022	Unknown	0.0 - 7.0	A-Direction	4.9	4.0	6.2	3.9	-2.3

Notes:

¹ Meters below ground surface (mbgs).

Table 2.2 Vibrating Wire Piezometer Reading Summary

Instrument ID / Serial No.		Date		Ground Surface Elevation (m)	Tip Depth (mbgs ¹)	Water Level			
	Installed	Previous Reading	Most Recent Reading			Previous Reading (mbgs ¹)	Most Recent Reading (mbgs ¹)	Change from Previous Reading (m)	
VW46428	Nov. 20, 2017	Sep. 10, 2020	Jul. 23, 2022	605	6.5	4.5	4.5	0.0	
VW46429	Nov. 20, 2017	Sep. 10, 2020	Jul. 23, 2022	605	12.3	5.3	5.5	-0.2	

Notes:

¹ Meters below ground surface (mbgs).



2.2 Zones of Movement

Before the instrument was destroyed in 2014 due to ongoing road maintenance, movement was being recorded in SI13-01 at an approximate depth of 2 m below ground surface, at the interface between the silt and clay till.

Distributed movement is being recorded in the SIs installed in the H-pile wall (SI21-C43-01 and SI21-C43-02) from top of casing to an approximate depth of 7 m below ground surface. The H-pile wall is 12 m deep, and SI21-C43-01 and SI21-C43-02 are approximately 10.9 and 11.4 m deep, respectively.

2.3 Interpretation of Monitoring Results

Previous assessments of the slope failure suggest that movement is likely occurring in response to periods of heavy or prolonged rainfall, resulting in higher groundwater conditions or a near surface zone of saturation within the north embankment slope for short periods of time. However, this is not supported by the most recent piezometer data. The water levels recorded in the VWPs have been relatively steady (± 0.3 m) since the instruments were installed in 2017. It is possible that the reading frequency of the instruments (currently annually, previously bi-annually) does not capture short-term fluctuations (i.e., increases and decreases) in water level that are occurring in response to periods of heavy or prolonged rainfall or freshet infiltration. More data is needed to assess.

Even after installation of soils nails in 2016, the slope continued to deform slowly (as indicated by pavement cracking and settlement) with the site requiring frequent pavement patches. Subsequently, an H-pile wall was installed in 2021.

The depth of movement being recorded in the pile-wall SIs appears to be occurring approximately 4 m above the base of the H-pile wall and approximately 2 m below the depth of movement previously recorded in SI13-01. This indicates the H-pile wall has intercepted the failure surface and is continuing to deflect, transferring load to depths below the failure plane as the piles stabilize the slide mass.

The June 2022 data obtained from the pile-wall SIs indicates that the top of the H-pile wall has deflected up to approximately 5 mm since installation. In the spring of 2021, shortly after construction of the H-pile wall, the maximum rate of movement recorded in these SIs was between approximately 6 mm/year and 12 mm/year. The rate of movement has since decreased and is now less than 5 mm/year. KCB anticipated that the rate of movement shortly after installation would be highest and would decrease as the H-pile wall picked up load stabilizing the sliding mass. However, last year and the spring of 2022 were relatively dry. Increased movement may occur in response to periods of heavy or prolonged rainfall, resulting in higher groundwater conditions More data is needed to assess long-term trends for these instruments.



3 RECOMMENDATIONS

3.1 Future Work

All operable instruments should continue to be read once per year (spring) until movements attenuate, and the rate of pavement cracking and deformation decreases.

The site should continue to be inspected by the Maintenance Contract Inspector (MCI) and as part of the Central Region GRMP Section B inspections, until movement rates attenuate.

3.2 Instrument Repairs and Maintenance

No instrument repairs or maintenance is required.



4 CLOSING

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Please contact the undersigned if you have any questions or comments regarding this report.

Yours truly,

KLOHN CRIPPEN BERGER LTD.

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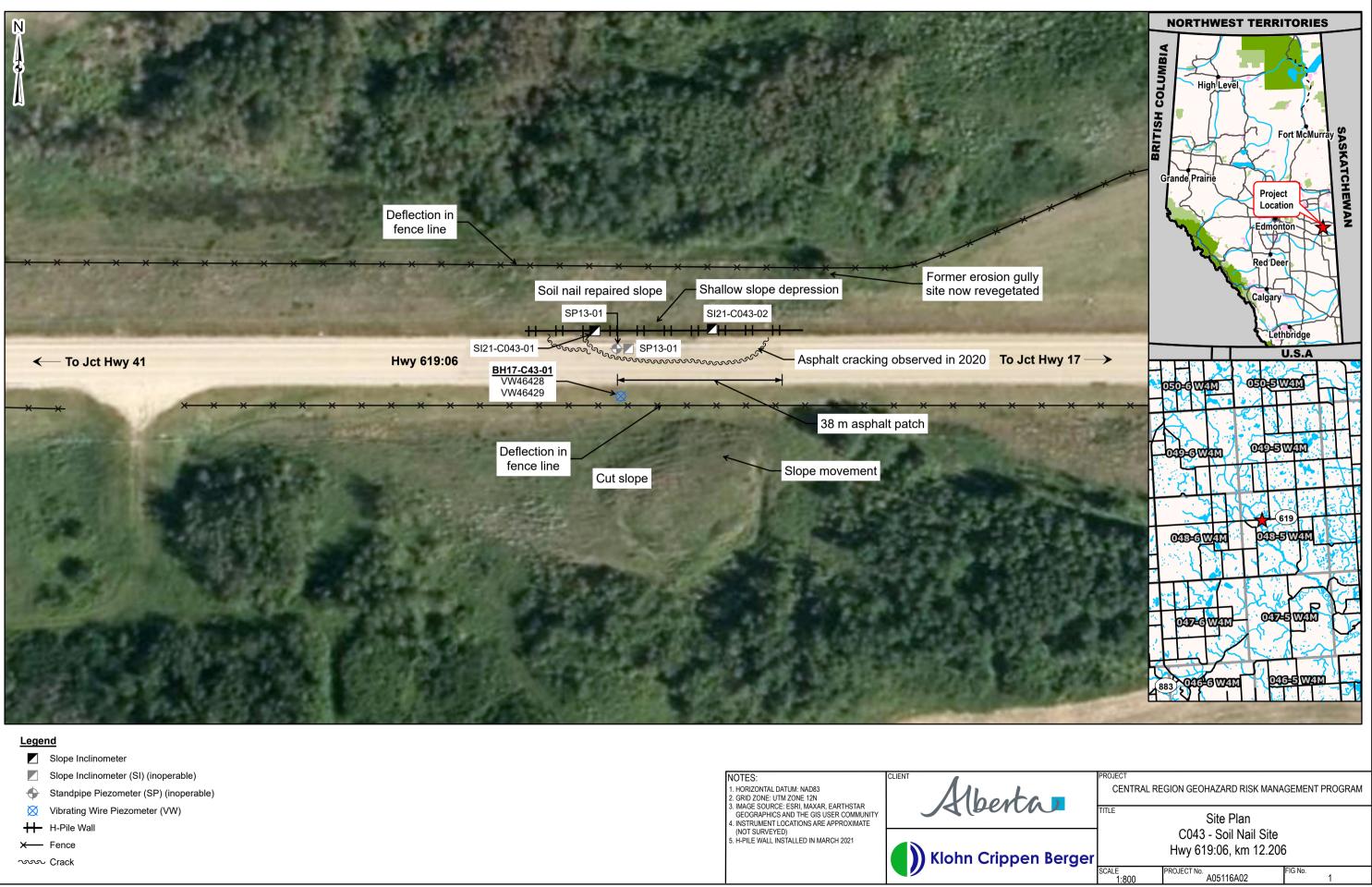
ATTACHMENTS Figure

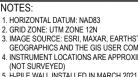
Appendix I Instrumentation Plots



FIGURE







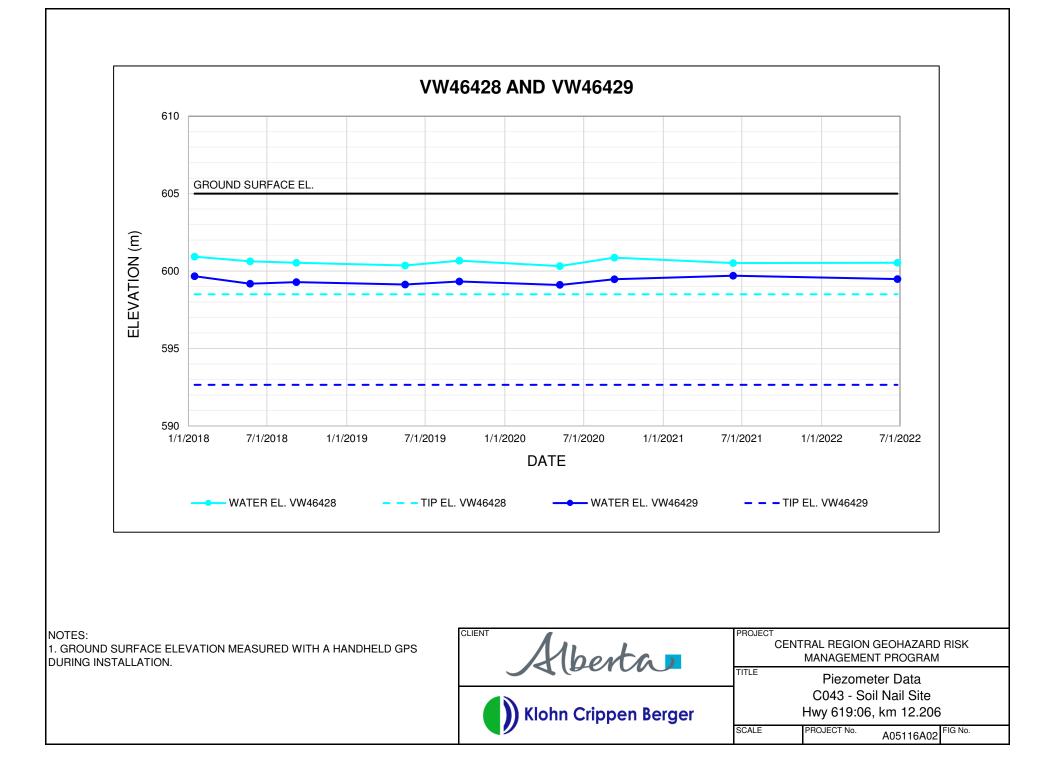


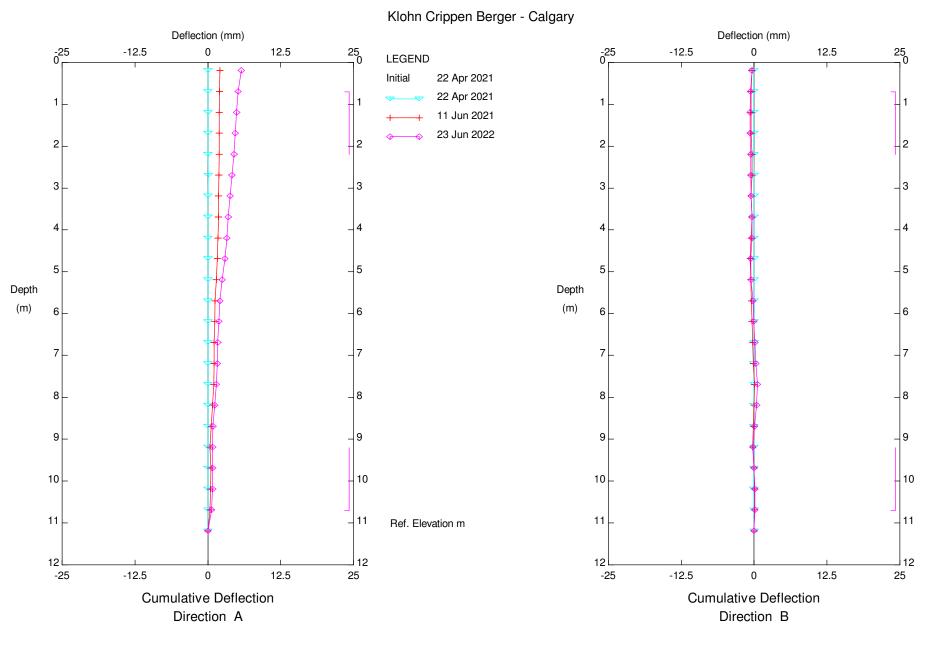
APPENDIX I

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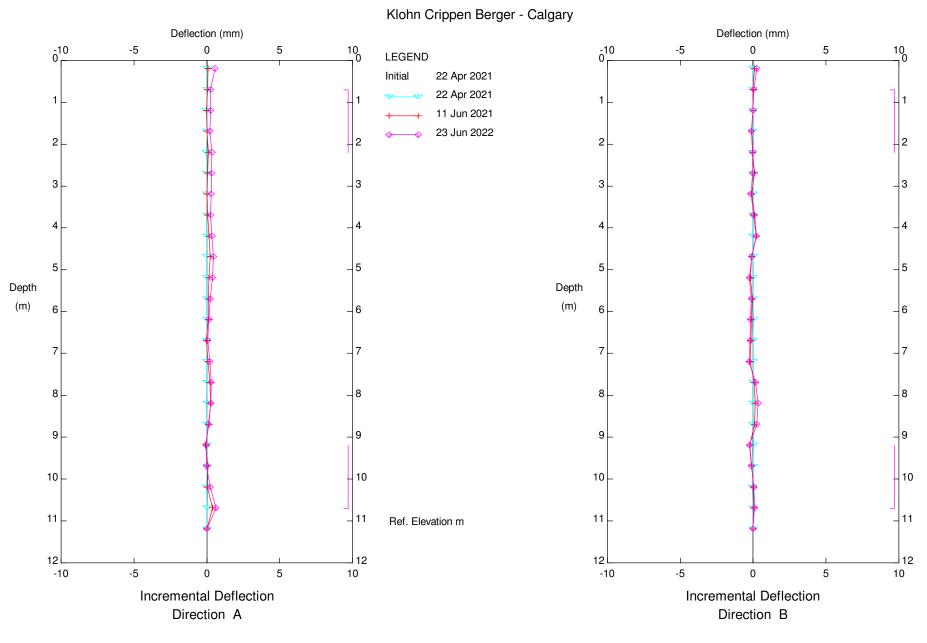
Instrumentation Plots



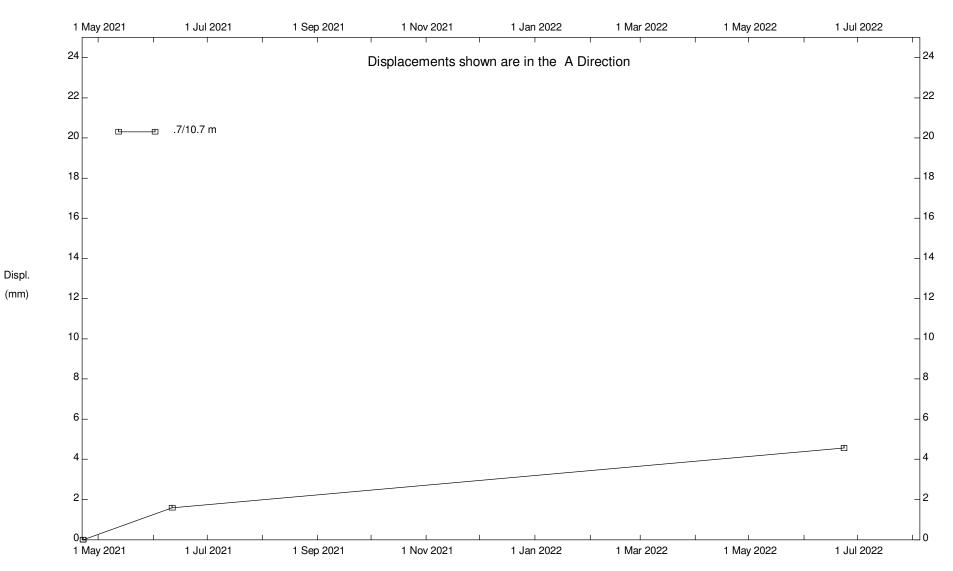








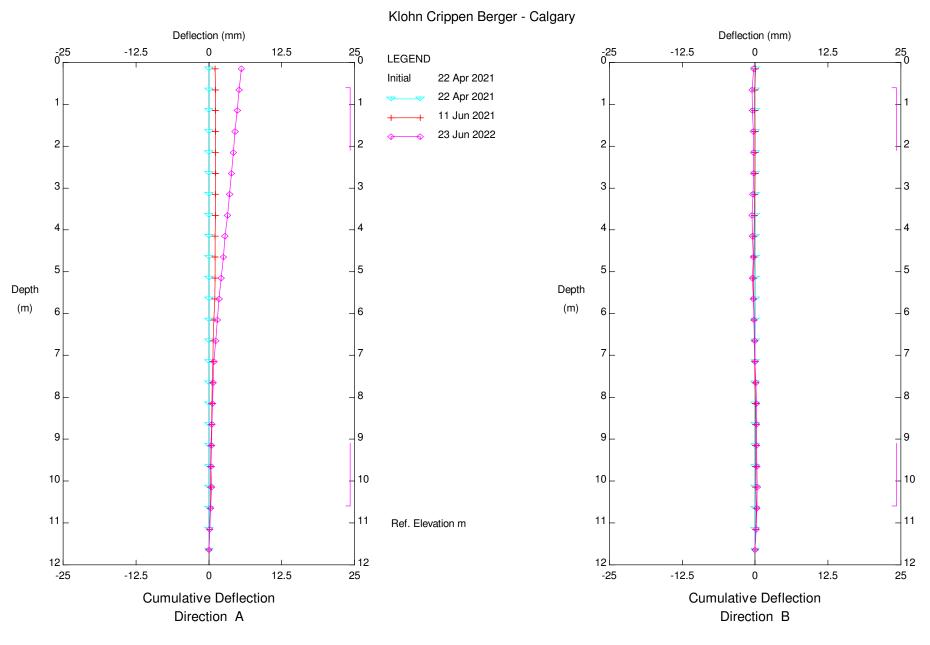




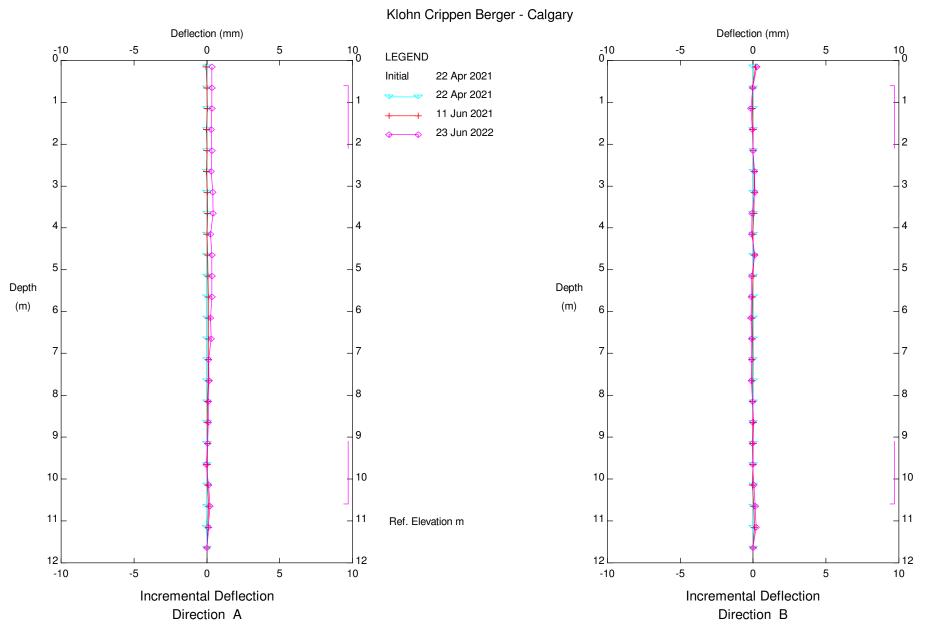
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C043; H619:06; Soil Nail Site, Inclinometer SI21-01

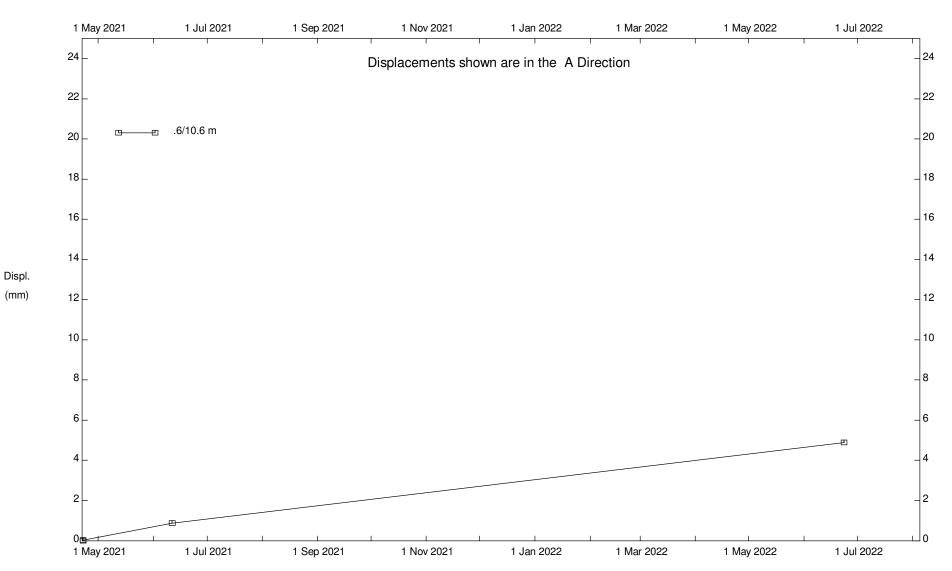
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