ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION-GRANDE PRAIRIE 2022 INSPECTION REPORT



Site Number	Location		Name		Hwy	km	
GP029 Saddle (Bu River Cros		urnt) sing	RR771 Slide (Church Camp)		2:70	11.881	
North Valle		ey Slope	UTM Co. ordinata	-			
			UIM Co-ordinates				
1100 /44-077-00-0001VI		110 E 393076 IN 6166469				19	
		Date	PF	CF	T	Total	
Previous Inspection:		28-May-2020	11	6		66	
Current Inspection:		17-May-2022	11	6		66	
Road AADT:		32	260 Year:		2	021	
Inspected by:		Ed Szmata, ATDon ProudforMax Shannon, ATNicole WildeKristen Tappenden, ATBarry MeaysAustin Dillman, ATKen Szmata, AT		Don Proudfoot Nicole Wilder, Barry Meays,	ot, Thurber ', Thurber , Thurber		
Report Attachments:		Photograph	าร				
		✓ Plans			ance Items		
Primary Site Iss Dimensions:	valley slope on the west side of the Saddle (Burnt) River. The highway was widened further east in 2013 to accommodate the construction of a new northbound passing lane. A portion of the passing lane was likely constructed over the backscarp of an existing landslide on the west valley slope. River erosion at the toe of valley slope undermines the global stability of the overall valley slope in combination with artesian conditions observed to occur at this site. The cracks observed on the ACP surface of the Hwy 2:70 indicated that the landslide backscarp extended beyond the SBL shoulder and into the west highway ditch. The lateral extent of the cracks observed in the ACP indicated that the affected area is about 240 m in width. Previously several ACP patches were observed in a small section of the SBL as well as a section of both NBLs over the previously distressed areas near the intersection between Highway 2:70 and						
Maintenance:		Township Road 771 in 2019. The site on all lanes was chip sealed in 2021 immediately prior to when we would have inspected the site in 2021. Hence why the 2021 inspection was postponed to 2022.					
Observations:			Description	11 121 2	Wors	ened?	
 Pavement Distress Cracks are shop placed in 2021 observed on the observed in the south through a The cracks in the width. A crack running paralle 		wing through the o with opening width e shoulder of SBL e pavement extend all lanes and into the east shoulder all extended about 5 to a crack directly	chip seal that w up to 80 mm we The scarp cra ded 200 m furth the east should re about 20 mm 0 m further sou v east of the cra	as ere ck ner er. in uth ck	۲		

	extension. There is also a dip in the road that is		
Slope Movement	evident when cars drive over the surface.The readings of the SIs installed in 2017 indicatedthe presence of a deep slip surface at a depth about45 m below the existing ground surface. There werealso movement zones observed between 17 m and33 m depth below ground surface. Only SI17-5 didnot show movement; however, this may be installedtoo shallow.	v	
Erosion			
Seepage	Ponded water observed mid-slope northeast of SI-2 appeared to have more water.	2	
Bridge/Culvert Distres	S		
C Other			
Instrumentation:			
SI-1 (Destroyed)	Installed at about 20 m downslope (east) of Hwy 2:70 as a part of the 2007 geotechnical investigation program. This SI was likely destroyed during the construction of the northbound climbing lane in 2013. Readings of this SI indicated a slip surface at an approximate elevation of 622.6 m (about 22 m below ground surface).		
SI-2	Installed about 60 m downslope of the Hwy 2:70 NBL the middle of the Saddle (Burnt) River west valley showed a rate of slope movement of: 1.8 mm/yr wi 6.4 m to 9.5 m below the existing ground surface of 2022 readings. A movement zone within depths of 3 showed a rate of slope movement of 1.5 mm/yr during readings.	at approximately / slope. This SI thin depths from luring the spring 04.4 m to 35.1 m of the spring 2022	
SI-3	Installed about 120 m downslope of the Hwy 2:70 NB 20 m west of the crest of the top of bank of the low Saddle (Burnt) River. This SI showed a rate of movem within depths from 4.5 m to 7 m in spring 2022. 0.5 mm/yr within depths of 9.4 m to 10.6 m in spring 2	L, approximately er terrace of the ent of: 5.6 mm/yr The SI showed 2022.	
PN-2	Installed at the same location as SI-2 with the tip a 629.66 m. The measured piezometric surface was ground surface (B.G.S) indicating an increase in approximately 1.3 m since the fall 2021 readings.	t an elevation of 6 0.02 m above water level of	
PN-3	Installed at the same location as SI-3. Piezome damaged. The last water level measured in 2014 wa	eter leads were s at an elevation	
SI17-1	 Installed about 12 m downslope of the Hwy 2:70 NBL, opposite Township Rd 771. This SI previously showed a rate of movement 4.1 mm/yr within depths from 25 m to 26.3 m and 16.7 mm/yr with depths of 31.8 m to 33 m. However, this SI has since sheared off 33.2 m below the ground surface. 		
SI17-2	Installed about 55 m downslope of the Hwy 2:70 NBL and 43 m downslope of SI17-1. This SI showed a rate of movement of: 3.1 mm/yr within depths from 17.2 m to 19.7 m. The SI showed a zone of movement with rate of movement of 1.0 mm/yr within depths of 21.5 m and 23.3 m and showed a rate of movement of: 8.0 mm/yr within depths of 48.3 m to 50.8 m in spring 2020. However, this SI has since sheared off at 50.6 m below the ground surface.		
5117-3	75 m downslope of SI17-2. This SI showed a rate	of movement of:	

	58.2 mm/yr within depths from 42.2 m to 44.6 m in Fall 2019; however, this SI has since sheared off at 43.6 m below the ground surface.
SI17-4	Installed about 112 m downslope of the Hwy 2:70 NBL and 25 m north of SI-3. This SI showed a rate of movement of: 17.1 mm/yr within depths from 37.4 m to 39.2 m and a rate of movement of: 59.9 mm/yr within depths of 43.5 m to 45.3 m in Fall 2018; however, this SI has since sheared off at 45.1 m below the ground surface.
SI17-5	Installed about 12 m west of Hwy 2:70 SBL and about 80 m upslope from SI-2. This SI has shown no discernable movement since installation.
PN17-1A PN17-1B	Installed at same location as SI17-1 with PN17-1A and PN17-1B tips at elevations of 633.9 m and 610.5 m (11.7 m and 35.0 m B.G.S.), respectively. Both PN17-1A and PN17-1B are not functioning now.
PN17-2A PN17-2B PN17-2C	Installed at same location as SI17-2 with PN17-2A, PN17-2B and PN17-2C tips at elevations of 628.4 m, 598.5 and 587.9 m (10.2 m, 40.1 m, and 50.8 m B.G.S.), respectively. PN17-2A showed an increase in water level of 0.77 m, PN17-2B showed an increase in water level of 0.64 m and PN17-2C showed an increase in water level of 0.28 m since the fall 2021 readings.
PN17-3A PN17-3B PN17-3C	Installed at same location as SI17-3 with PN17-3A, PN17-3B and PN17-3C tips at elevations of 603.8 m, 591.3 and 581.4 m (25.7 m, 38.2 m, and 48.1 m B.G.S.), respectively. PN17-3A has not been functioning since installation. PN17-3B and PN17-3C both showed an increase of 0.50 m and 0.56 m, respectively in water levels since the fall 2021 readings, respectively.
PN17-4A PN17-4B PN17-4C	Installed at same location as SI17-4 with PN17-4A, PN17-4B and PN17-4C tips at elevations of 606.3 m, 590.9 and 582.7 m (24.9 m, 40.3 m, and 48.5 m B.G.S.), respectively. PN17-4A and PN17-4B showed decreases in water levels of 0.07 m and 0.21 m, respectively and PN17-4C showed an increase in water level of 0.49 m since the fall 2021 readings. PN17-4C shows the ground water level to be 7.53 m above ground surface.
PN17-5A PN17-5B	Installed at same location as SI17-5 with PN17-5A and PN17-5B tips at elevations of 633.2 m and 612.0 m (14.2 m and 35.4 m B.G.S.) respectively. PN17-5A showed an increase in water levels of 0.28 m and PN17-5B showed a decrease in water level of 0.14 m since the fall 2021 readings.
SP17-6	Installed in the farmers field about 125 m south of SI17-5 and showed a decrease of 0.72 m in water level since the fall 2021 readings.

Assessment:

Cracks on the ACP surface have been regularly observed and patched/chip sealed yearly since 2004. The distress and cracking of the pavement observed at the site since 2014 are likely the result of the widening of the roadway and the construction of pavement over the backscarp of the existing landslide.

The depth of the slip surface previously observed in the slope inclinometer SI-1 was about 22 m below the existing ground surface, indicating that the landslide affecting the highway is a deep-seated slope failure. The new SIs installed on September 2017 indicated that the landslide failure plane is even deeper, at approximately 45 m depth. The cracks observed in the ACP indicated that the backscarp of the landslide extended into the west higway ditch beyond the SBL.

The instruments installed in 2017 confirmed that the landslide affecting the highway is in fact a deep-seated landslide driven by artesian groundwater conditions; therefore, the most feasible mitigation measure is to realign the highway further west out of the landslide area. The conventional slope stabilization measures such as toe berms and pile walls are not considered to be feasible alternatives.

A preliminary design for a highway re-alignment was carried out by Thurber and WSP to by-pass the affected area. The preliminary design was submitted to AT in July 2018.

On July 24, 2018 a call-out was performed at this site as Mr. Ken Misik, AT's MCI for this area observed a pronounced dip at the inner wheel path of the road. A copy of the call-out letter dated August 16, 2018 is included in the site binder.

Currently, a detailed design for highway re-alignment is being carried out by Thurber and WSP. A key issue for the finalization of the detailed design is related to obtaining agreements for pipeline crossing and existing oil wells along the proposed re-alignment, in addition to the acquisition of additional ROW.

Recommendations:Ballpark CostIn the short term, the development of the cracks on in the ACP surface of the
roadway should be regularly monitored. Cracks should be timely patched/sealed
to prevent surface water infiltration into the landslide backscarp.Monitoring

Closure

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Yours very truly, Thurber Engineering Ltd. Don Proudfoot, P.Eng. Principal | Senior Geotechnical Engineer

Nicole Wilder, M.Eng., P.Eng. Geotechnical Engineer



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