

July 15, 2025

Alberta Transportation and Economic Corridors Main Floor, Provincial Building 9621 – 96th Avenue Peace River, Alberta T8S 1T4

Robert Senior Construction Technologist

Dear Mr. Senior:

CON0022166 Peace Region (Grande Prairie District – South) GRMP Instrumentation Monitoring Site GP034; H40:38, km 21.016 Slide South of Kakwa River Section C – 2025 Spring Readings

1 GENERAL

Two slope inclinometers (SIs) (SI17-2 and SI17-3) and four pneumatic piezometers (PNs) (PN17-2A/B, and PN17-6A/B) were read at the GP034 site in the Peace Region (Grande Prairie District – South) (GP South Region) on June 3, 2025,, by Evan Hergott, E.I.T. and Min Hou, E.I.T. of Klohn Crippen Berger Ltd. (KCB). These instruments were read as part of the GP South Region Geohazard Risk Management Program (GRMP). The site is located on Hwy 40:38, km 21.016. The approximate site coordinates are 6028755 N, 399896 E (UTM Zone 11, NAD 83). A site plan is presented on Figure 1.

The geohazard at the GP034 site consists of a landslide in the fill and foundation of an approximate 25-m high side-hill highway embankment, approximately 500 m upslope (east) from the Kakwa River.

Previous remedial actions completed at the GP034 site include the enlargement of an existing toe berm with drainage in late 2020. Foundation movements and cracking of the asphalt in response to toe berm construction resulted in work being halted. Significant asphalt cracking continued into the spring and fall of 2021.

In 2017, a geotechnical site investigation, which included installing instruments, was conducted at the site by a previous consultant. The encountered stratigraphy was as follows: clay fill, overlying rafted clay shale, overlying clay till, and overlying bedrock consisting of siltstone and sandstone.



1.1 Instrumentation

KCB has been reading the instruments at this site since the spring of 2021. Instrumentation installation details are tabulated in Table 1.1. Instrument locations are shown on Figure 1. Any instruments not included in Table 1.1 or shown on Figure 1 are assumed to be inoperable and are not presented or discussed herein. It is noted that the ground surface elevations provided for the instruments were measured prior to construction and need to be re-surveyed.

In 2017, 4 SIs and 12 piezometers were installed at the site by a previous consultant to monitor movement and groundwater conditions, respectively. Some of these instruments are now inoperable (e.g., sheared or damaged), as detailed in Table 1.1 (see table notes). Recommendations for replacement are made in Section 3.

The instruments are protected by above-ground casing protectors.

The operable SIs were read using the same metric RST Digital MEMS Inclinometer System that has been used to read the SIs since KCB took over the readings in June 2021. The operable PNs were read using an RST C109 pneumatic piezometer readout.

Table 1.1 Instrumentation Installation Details¹

Instrument	Instrument	Date Installed	UTM Coordin	ates (m)	Ground Surface	Stick Up	Depth	Condition
Туре	ID	Date installed	Northing	Easting	Elevation ² (m)	(m)	(mbgs³)	Condition
SI	SI17-2	Feb. 08, 2017	6028705	399849	865.9	0.9	16.8	Operable
	SI17-3	Feb. 07, 2017	6028714	399799	854.6	0.7	18.0	Operable
31	SI17-5	Feb. 06, 2017	6028627 399835		870.3	0.7	15.5	Inoperable ⁴
	SI17-6	Feb. 09, 2017	6028647/2 ¹	399786	857.3	0.8	17.5	Inoperable ⁴
	PN17-1A	F 07 2047	6028699	200070	965.4	N/A	4.6	Inoperable 6
PN	PN17-1B	Feb. 07, 2017		399878	865.4	N/A	8.0	Inoperable ⁶
	PN17-2A	Fab 00 2017	6028705	399849	005.0	N/A	7.9	Operable
	PN17-2B	Feb. 08, 2017			865.9	N/A	10.0	Operable
	PN17-3A	Fab 07 2017	6020714	399799	051.6	N/A	4.1	Inoperable ⁷
	PN17-3B	Feb. 07, 2017	6028714		851.6	N/A	10.2	Inoperable ⁷
	PN17-4A	Fab 00 2017	6020620	399864	070.3	N/A	5.0	Inoperable ⁶
	PN17-4B	Feb. 08, 2017	6028620		870.3	N/A	9.0	Inoperable ⁶
	PN17-5A	Fab 00 2017	6028627	399835	070.4	N/A	12.0	Inoperable ²
	PN17-5B	Feb. 06, 2017			870.1	N/A	14.9	Inoperable ²
	PN17-6A	Fab 00 2017	C020C47/21	399786	057.3	N/A	6.0	Operable
	PN17-6B	Feb. 09, 2017	6028647/2 ¹		857.3	N/A	12.0	Operable

Notes:

 $^{^1}$ Instrument installation details taken from reports and data files prepared or provided by the previous consultant(s) or TEC. Instrument coordinates and stick ups (where applicable) were confirmed by KCB with a handheld GPS (accuracy of \pm 5 m) and tape measure, respectively. Note the northing for SI17-6 and PN17-6A/B is reported as 6028642 and 6028647 in the files provided by the previous consultant and TEC, respectively.

² Ground surface elevations were measured prior to construction and need to be re-surveyed.

³ Meters below ground surface (mbgs). Bottom reading depth for operable SIs, and tip depth for piezometers. Either bottom reading or casing depth for inoperable SIs.

⁴ SI17-5 and SI17-6 have sheared at an approximate depth of 7.3 m and 6.7 m below ground surface, respectively. Instruments last read in October 2017 and October 2018, respectively.

⁶ PN17-1A/B and PN17-4A/B were destroyed in 2020 during construction. Instruments last read in October 2020.

⁷ PN17-3A/B and PN17-5A/B are inoperable (readings would not stabilize). Instruments last read in November 2020, September 2022, July 2019, and June 2022, respectively.

2 INTERPRETATION

2.1 General

For the operable SIs, the cumulative displacement, incremental displacement, and displacement-time data was plotted in the A-direction (i.e., the direction of the A0-grooves) and the X-direction (i.e., the direction of maximum movement obtained at a skew angle from the A0-grooves). SI17-2 and SI17-3 have skew angles of 15° and 35°, respectively, measured clockwise from the direction of the A0-grooves.

For the operable PNs, the recorded porewater pressures were converted to an equivalent water/piezometric elevation and plotted relative to ground surface elevation and the tip elevation for each instrument.

The SI and piezometer data plots are included in Appendix I, and a summary of the SI and piezometer data is provided in Table 2.1 and Table 2.2, respectively. The SI data plots presented herein include data for readings taken with both the previous consultants' and KCB's SI reading equipment.

2.2 Zones of Movement

Discrete movement (i.e., movement occurring on a defined failure plane) is being recorded in:

- SI17-2 between an approximate depth of 9.3 m and 10.8 m below ground surface (approximately elevation 856.6 m to 855.1 m) at the bottom of a clay till foundation unit overlying bedrock (siltstone); and
- SI17-3 between an approximate depth of 13.0 m and 14.5 m below ground surface (approximately elevation 841.6 m to 840.1 m) in a clay foundation unit overlying bedrock (clay shale).

Shallow distributed movement is also being recorded in the upper 4.5 m of SI17-3 in the toe berm fill placed in late 2020.

Table 2.1 Slope Inclinometer Reading Summary

	Date				Ground	Depth of	Direction of	Movement (mm)		Rate of Movement (mm/year)		
Instrument ID	Initialized	Previous Maximum Cumulative Movement Recorded	Previous Reading	Most Recent Reading	Surface Elevation ¹ (m)	Movement (mbgs ²)	Direction of Movement, Skew Angle ³	Maximum Cumulative	Incremental Since Previous Maximum Cumulative	Previous Maximum	Most Recent Reading	Change from Previous Reading
SI17-2	Feb. 08, 2017	Oct. 18, 2024	Oct. 18, 2024	Jun. 03, 2025	865.9	8.3 – 10.8	X-Direction, 15°	51.3	1.5	211.6	2.4	0.5
SI17-3 Feb. 08, 2017	Fab 00 2017	Oct. 18, 2024	Oct 19 2024	lun 02 2025	954.6	0.0 - 4.5	X-Direction, 35°	257.8	1.7	5,519.4	2.7	2.0
	Oct. 18, 2024	Oct. 18, 2024	Jun. 03, 2025	854.6	13.0 – 14.5	X-Direction, 35°	77.4	1.3	866.9	2.0	-2.0	

Notes:

Table 2.2 Pneumatic Piezometer Reading Summary

Instrument ID	Serial No.	Date			Ground Surface	Tip Depth	Water Level			
		Installed	Previous Reading	Most Recent Reading	Elevation ¹ (m)	(mbgs ²)	Previous Reading (mbgs ²)	Most Recent Reading (mbgs²)	Change from Previous Reading (m)	
PN17-2A	37443	Feb. 08, 2017	Oct. 18, 2024	Jun. 03, 2025	865.9	7.9	4.9	6.4	-1.5	
PN17-2B	37439		Oct. 18, 2024	Jun. 03, 2025		10.0	6.6	9.6	-3.0	
PN17-6A	37444	Feb. 09, 2017	Oct. 18, 2024	Jun. 03, 2025	857.3	6.0	0.3	1.6	-1.3	
PN17-6B	37433		Oct. 18, 2024	Jun. 03, 2025		12.0	7.6	7.6	0.0	

Notes:

¹ Ground surface elevations were measured prior to construction and should be surveyed.

² Meters below ground surface (mbgs).

³ Skew angle of the X-direction measured clockwise from the A-direction. The azimuths of the A0-grooves in the SIs were measured by KCB with a magnetic compass in spring 2022.

¹Ground surface elevations were measured prior to construction and should be surveyed.

² Meters below ground surface (mbgs).

2.3 Interpretation of Monitoring Results

Slope Inclinometers

The rate of foundation movement being recorded in SI17-2 and SI17-3 was relatively slow and steady (less than 5 mm/year) between February 2017 (installation/initialization) and October 2020 but increased up to approximately 212 mm/year and 867 mm/year, respectively, when toe berm construction began in late 2020. Since January 2021, the rate of foundation movement recorded in SI17-2 and SI17-3 has been decreasing and is currently less than 3 mm/year. It is noted that the rate of movement recorded in SI17-2 and SI17-3 does not appear to have increased in response to a 5.2 magnitude earthquake that occurred near the site in February 2025.

The shallow fill movement being recorded in SI17-3, since toe berm construction in late 2020, is believed to be causing the cracks observed in the pavement surface and along the highway embankment and may be an extension upwards along the backscarp, associated with deeper movement. The rate of shallow movement recorded between late 2020 and September 2021 is difficult to assess due to the SI casing being extended in November 2020 and the SI reading equipment being changed four times between November 2020 and June 2021. The maximum rate of movement recorded during toe berm construction was approximately 5,519 mm/year. Since September 2021, the rate of shallow fill movement recorded in SI17-3 has been decreasing and is currently less than 3 mm/year.

Increased movement, which could impact the highway, may occur in response to precipitation and freshet infiltration, resulting in higher groundwater conditions.

<u>Piezometers</u>

Before toe berm construction, relatively steady porewater pressures were recorded in PN17-2A/B (located below highway shoulder, outside toe berm footprint) and PN17-6A/B (located below toe berm crest, downslope of highway). Following toe berm construction in late 2020:

- PN17-6A: Porewater pressures recorded in this instrument increased approximately 4.7 m between October 2020 and June 2021 before decreasing approximately 4.0 m between June 2021 and June 2022. This increase and decrease are most likely due to construction-induced porewater pressure response followed by post-construction porewater pressure dissipation, respectively. Since June 2022, porewater pressures recorded in PN17-6A have been fluctuating (approximately 1.3 m to 2.9 m reading to reading, with the fall reading higher than the spring reading and sometimes near ground surface) likely in response to seasonal variations in freshet and precipitation infiltration.
- PN17-6B: This instrument did not stabilize between November 2020 and September 2022. Since September 2022, porewater pressures recorded in this instrument have been relatively steady and approximately 1.5 m to 2.5 m higher than readings obtained before toe berm construction in late 2020.

• PN17-2A/B: Porewater pressures recorded in these instruments were relatively steady before construction, and have been fluctuating from tip elevation (i.e., dry) to approximately 3.0 m and 3.4 m above tip elevation, respectively, after construction likely due to seasonal variations in precipitation and freshet infiltration. Overall, porewater pressures recorded after construction are approximately 1 m to 2 m higher than those recorded before construction.

Currently, the porewater pressures recorded in PN17-2A/2B/6A/6B are between approximately elevation 849.7 m and 859.5 m, which is just below to above the zone of movement recorded in SI17-3 between approximately elevation 850.1 m and 854.6 m. Increases in groundwater level could trigger increased movement, which could impact the highway.

Groundwater discharge has been observed on the upper slope of the highway embankment between PN17-2A/B and PN17-5A/B, approximately 3 m below pavement surface. The groundwater discharge is above the water level recorded in PN17-2A/B and previously recorded in PN17-5A/B (instruments now inoperable), and the bottom elevation of the trench drain located below the ditch on the other side of the highway. This indicates that the existing piezometers are not reflective of groundwater conditions across the site.

3 RECOMMENDATIONS

3.1 Future Work

All operable instruments should continue to be read once per year (spring). Spring readings should be completed after late-May or early-June, due to the risk of water inside the instrument casings being frozen earlier in the year.

The site should continue to be inspected by the Maintenance Contract Inspector (MCI) and as part of the GP South Region GRMP Section B inspections.

3.2 Instrument Installs, Repairs, and Maintenance

PN17-1A/B and PN-4A/B were destroyed in 2020 during construction, and PN-3A/B and PN-5A/B have also become inoperable (i.e., readings do not stabilize). Replacement of these instruments or installation of new instruments near the location of observed groundwater discharge should be considered if site conditions deteriorate and/or repair work were to be resumed.

4 **CLOSING**

This report is an instrument of service of Klohn Crippen Berger (KCB). The report has been prepared for the exclusive use of Alberta Transportation and Economic Corridors (Client) for the specific application to the Peace Region (Grande Prairie District – South) Geohazard Risk Management Program (Contract No. CON0022166), and it may not be relied upon by any other party without KCB's written consent.

KCB has prepared this report in a manner consistent with the level of care, skill and diligence ordinarily provided by members of the same profession for projects of a similar nature at the time and place the services were rendered. KCB makes no warranty, express or implied.

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- 5. This report is electronically signed and sealed and its electronic form is considered the original. A printed version of the original can be relied upon as a true copy when supplied by the author or when printed from its original electronic file.

Yours truly,

KLOHN CRIPPEN BERGER LTD.

Courtney Mulhall, M.Sc., P.Eng. Geotechnical Engineer

Evan Hergott, E.I.T. Civil Engineer-in-Training

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CM/EH:bb

Cc: Chris Gräpel, M.Eng., P.Eng.

ATTACHMENTS

Figure

Appendix I Instrumentation Plots

Site GP034; H40:38, km 21.016 Slide South of Kakwa River Section C – 2025 Spring Readings

FIGURE

Legend

- Pneumatic Piezometer (PN)
- ✓ Slope Inclinometer (SI)
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Klohn Crippen Berger

PEACE REGION (GRANDE PRAIRIE DISTRICT-SOUTH)
GEOHAZARD RISK MANAGEMENT PROGRAM

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Site Plan GP034 - Slide South of Kakwa River

Hwy 40:38, km 21.016

PROJECT No. A05116A01

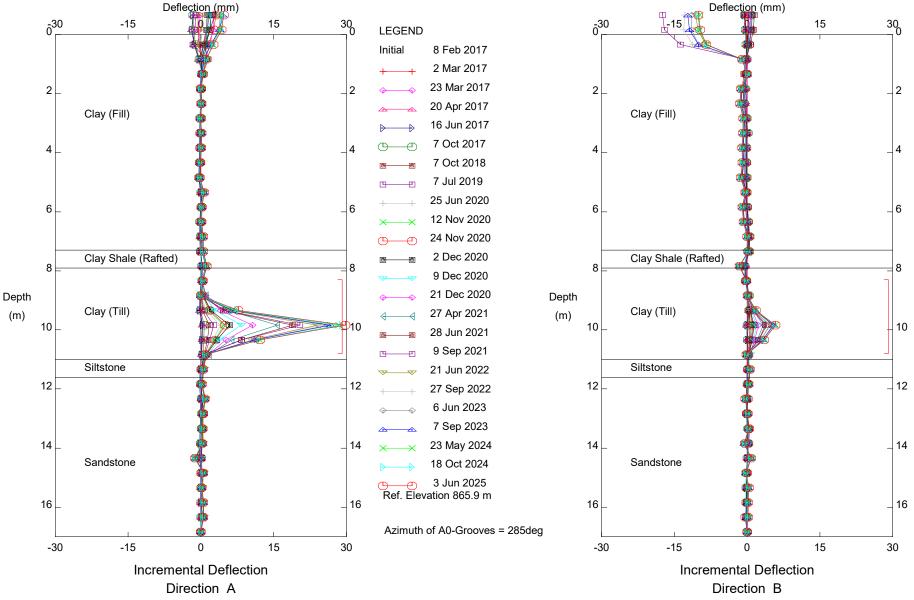
Site GP034; H40:38, km 21.016 Slide South of Kakwa River Section C – 2025 Spring Readings

APPENDIX I

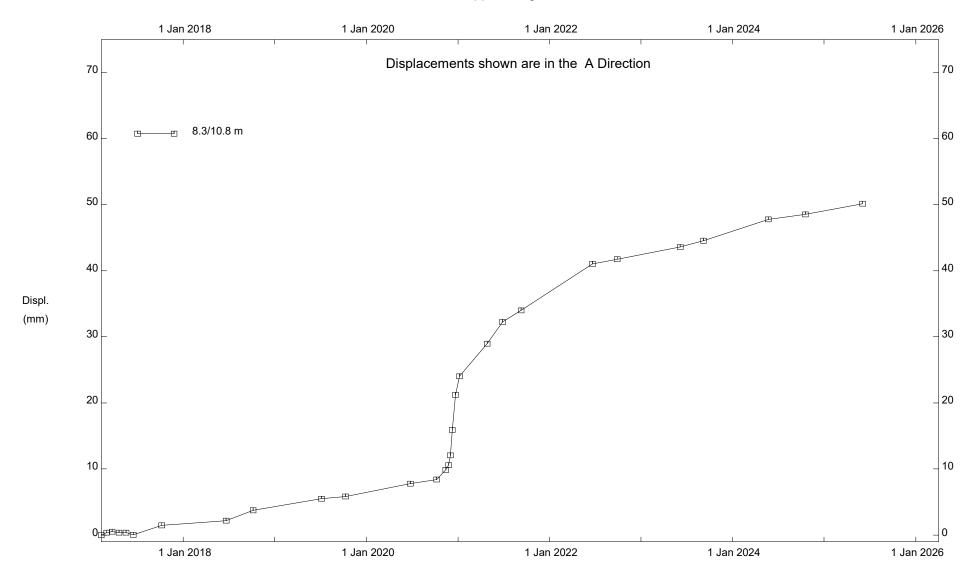
Instrumentation Plots

Klohn Crippen Berger - Edmonton Deflection (mm) Deflection (mm) ō⁷⁵ -37.5 ₀75 37.5 75 __0 LEGEND 8 Feb 2017 Initial 2 Mar 2017 23 Mar 2017 2 2 2 20 Apr 2017 Clay (Fill) Clay (Fill) 16 Jun 2017 7 Oct 2017 7 Oct 2018 7 Jul 2019 25 Jun 2020 6 6 12 Nov 2020 24 Nov 2020 Clay Shale (Rafted) 2 Dec 2020 Clay Shale (Rafted) 8 9 Dec 2020 21 Dec 2020 Depth Depth Clay (Till) Clay (Till) 27 Apr 2021 (m) (m) 10 10 10 10 28 Jun 2021 9 Sep 2021 Siltstone Siltstone 21 Jun 2022 12 12 12 12 27 Sep 2022 6 Jun 2023 7 Sep 2023 23 May 2024 14 14 14 Sandstone 18 Oct 2024 Sandstone 3 Jun 2025 Ref. Elevation 865.9 m 16 16 16 16 Azimuth of A0-Grooves = 285deg -37.5 37.5 -37.5 37.5 -75 75 -75 0 75 **Cumulative Deflection Cumulative Deflection** Direction A Direction B

GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-2
Alberta Transportation



GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-2
Alberta Transportation



GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-2

Alberta Transportation

Klohn Crippen Berger - Edmonton Deflection (mm) Deflection (mm) ₀75 -37.5 $\bar{0}^{75}_{-}$ 37.5 75 __0 LEGEND 8 Feb 2017 Initial 2 Mar 2017 23 Mar 2017 2 2 2 20 Apr 2017 Clay (Fill) Clay (Fill) 16 Jun 2017 7 Oct 2017 7 Oct 2018 7 Jul 2019 25 Jun 2020 6 6 12 Nov 2020 24 Nov 2020 Clay Shale (Rafted) 2 Dec 2020 Clay Shale (Rafted) 8 9 Dec 2020 21 Dec 2020 Depth Depth Clay (Till) Clay (Till) 27 Apr 2021 (m) (m) 10 10 10 10 28 Jun 2021 9 Sep 2021 Siltstone Siltstone 21 Jun 2022 12 12 12 12 27 Sep 2022 6 Jun 2023 7 Sep 2023 23 May 2024 14 14 14 Sandstone 18 Oct 2024 Sandstone 3 Jun 2025 Ref. Elevation 865.9 m 16 16 16 skew = 15deg Azimuth of A0-Grooves = 285deg 37.5 -37.5 37.5 -75 -37.5 75 -75 0 75

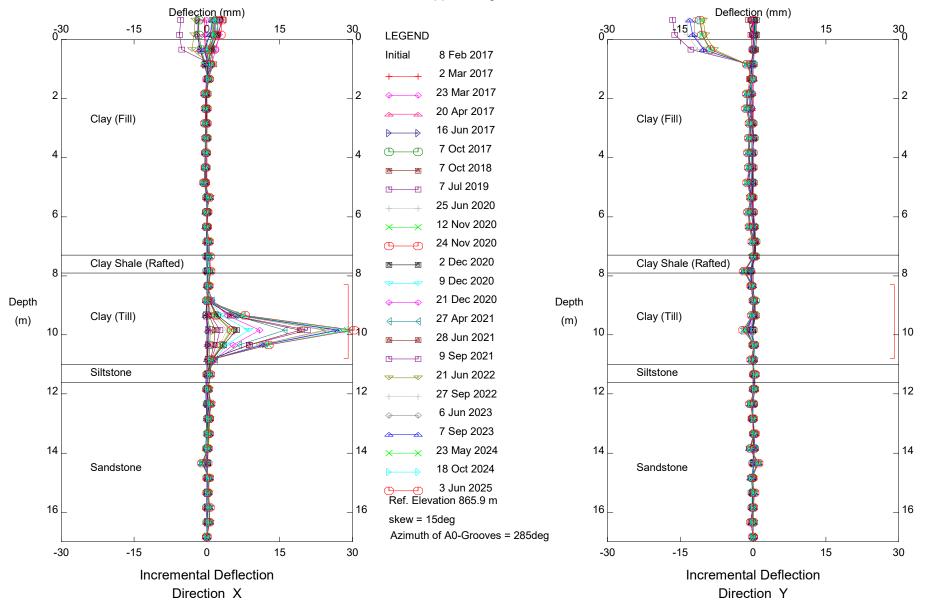
GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-2
Alberta Transportation

Cumulative Deflection

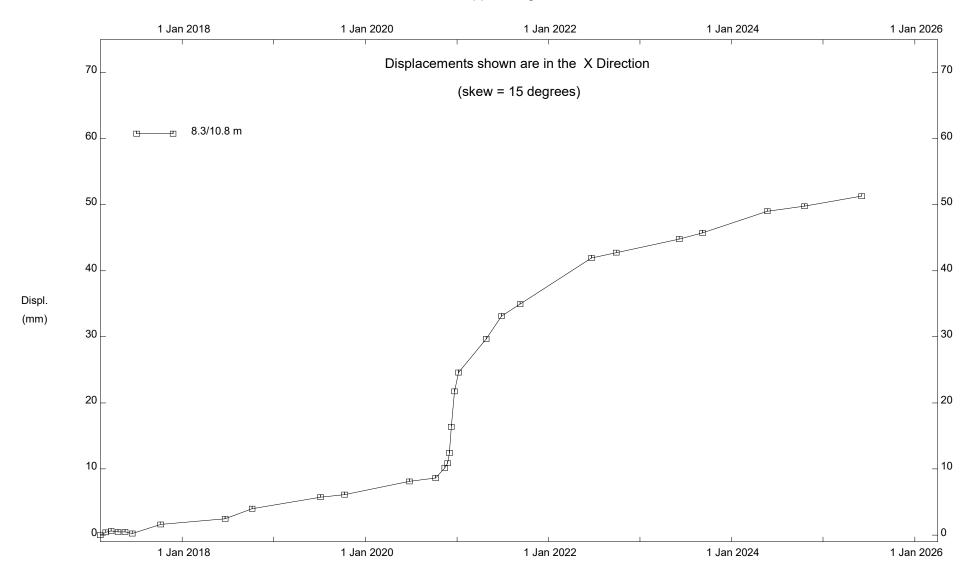
Direction Y

Cumulative Deflection

Direction X

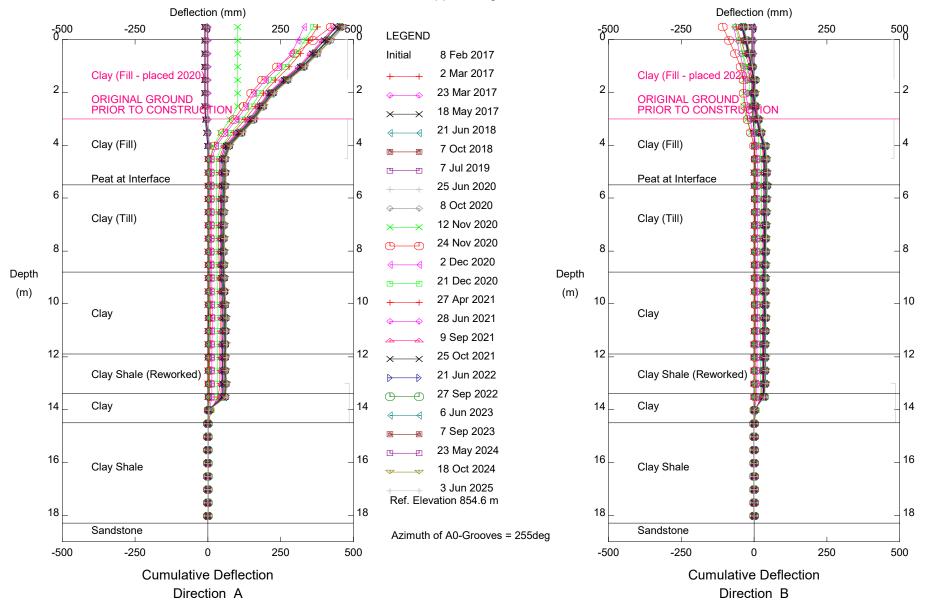


GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-2
Alberta Transportation

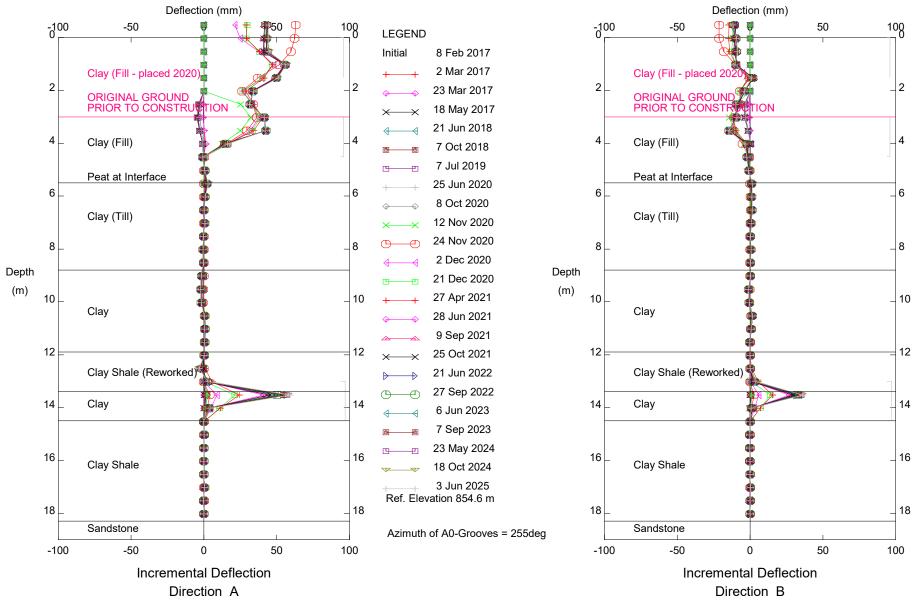


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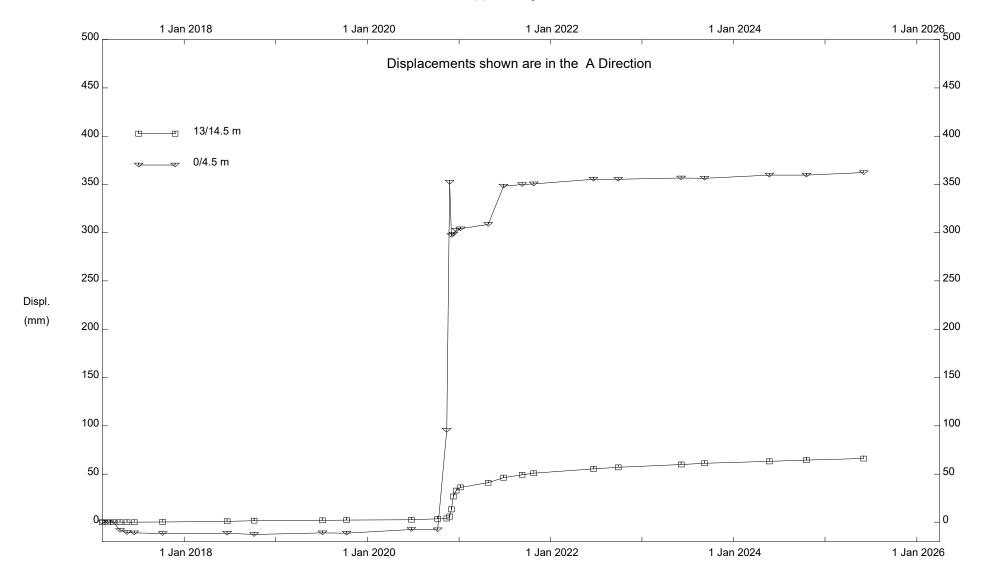
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GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-3
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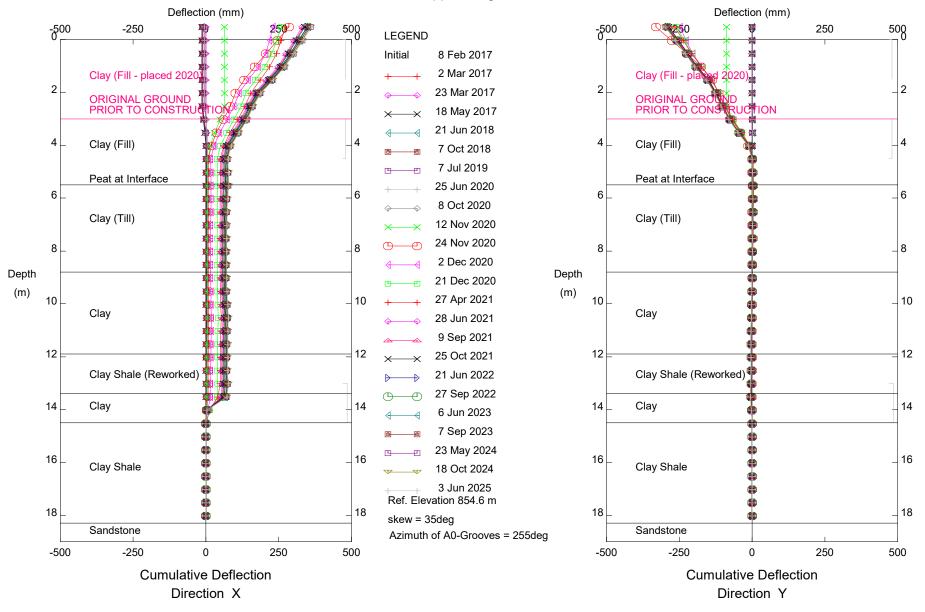


GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-3
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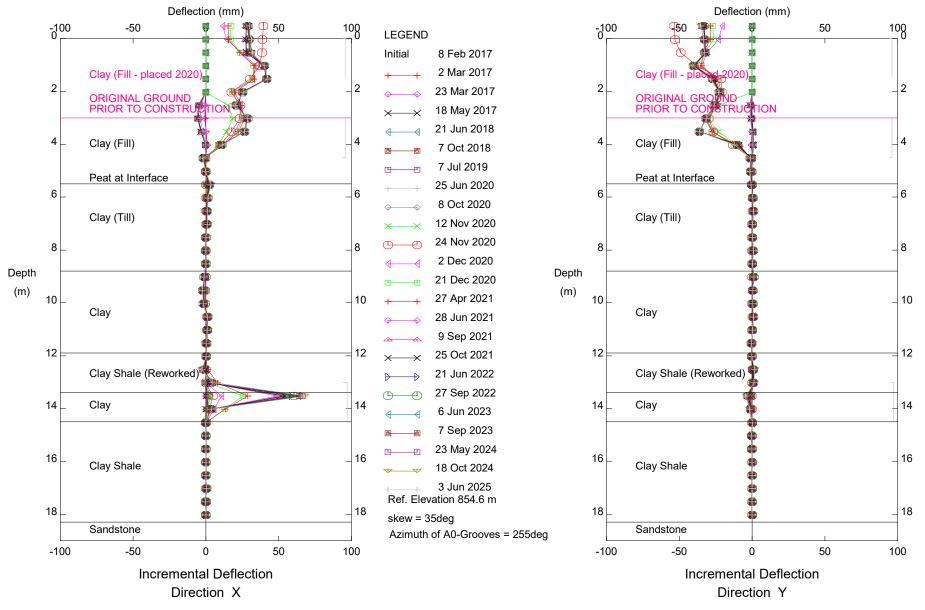


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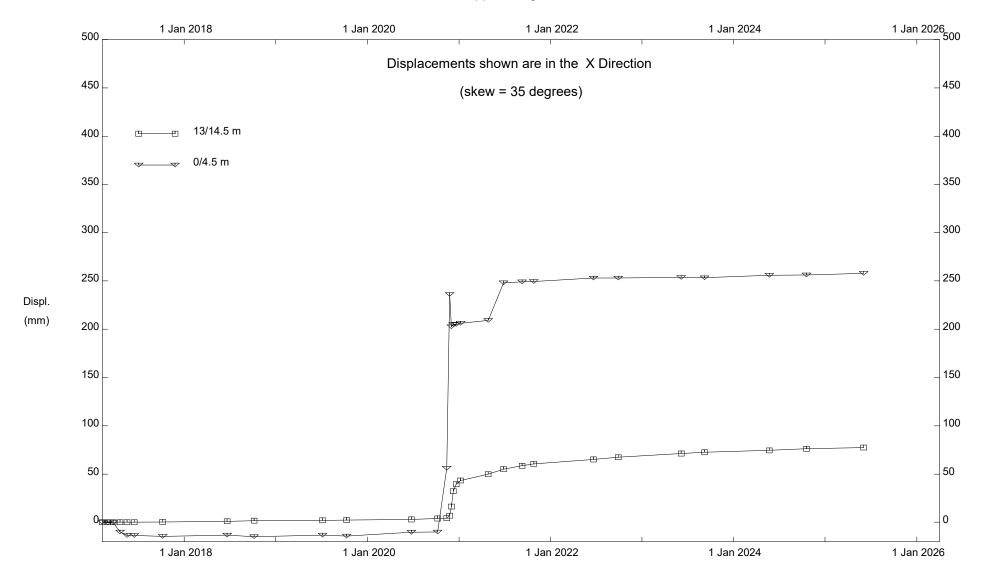
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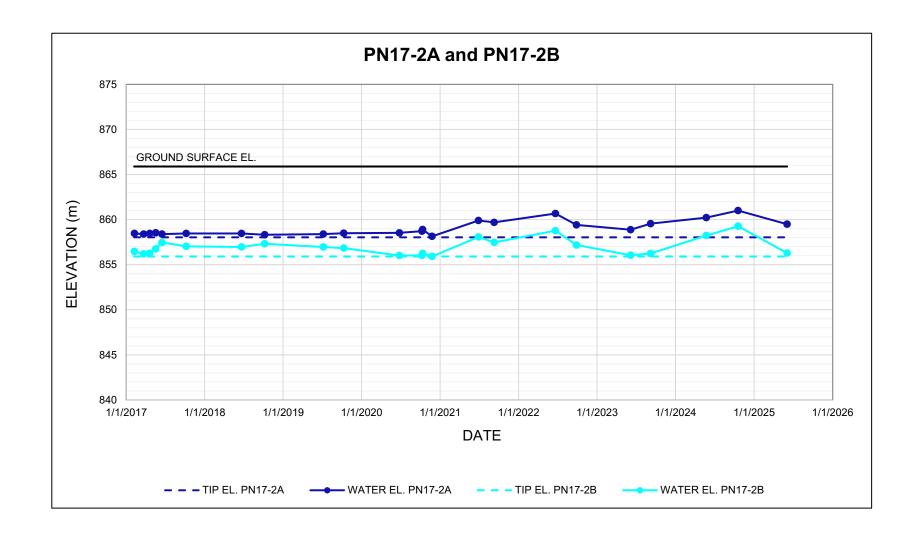


GP034; H40:38, Slide S. of Kakwa River, Inclinometer SI17-3
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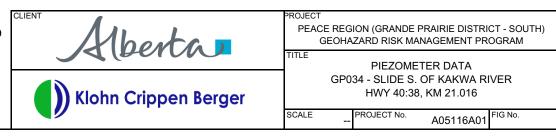
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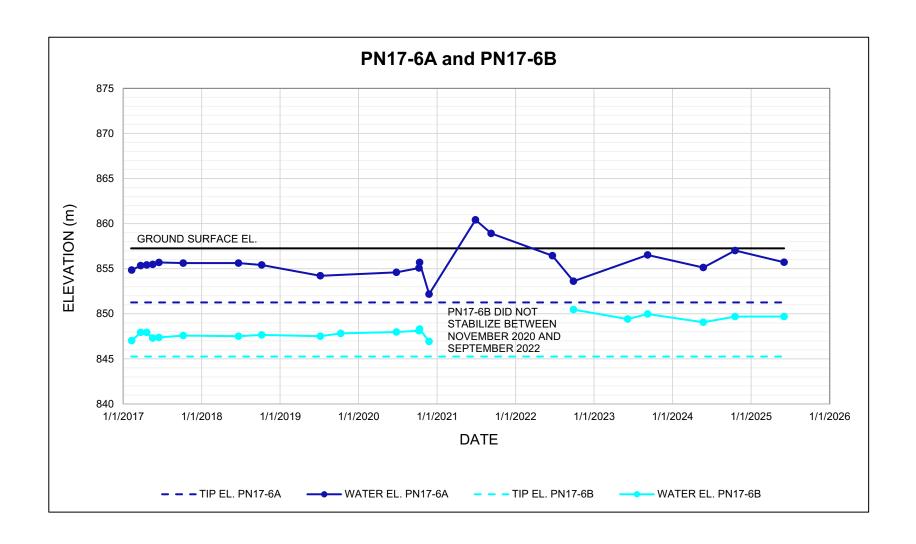
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NOTES:

- 1. PIEZOMETER DATA OBTAINED BEFORE JUNE 28, 2021, PROVIDED TO KLOHN CRIPPEN BERGER LTD. BY ALBERTA TRANSPORTATION AND ECONOMIC CORRIDORS ON JUNE 25, 2021.
- 2. GROUND SURFACE ELEVATION MEASURED PRIOR TO CONSTRUCTION AND NEEDS TO BE UPDATED.





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