ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – GRANDE PRAIRIE DISTRICT 2020 INSPECTION REPORT



Site Number	Location		Name		Hwy	km	
		noky River West	Hwy 43/Smoky River West Valley Backslopes		43:04	32	
Legal Descriptio	n		UTM Co-ordinate			1	
SE¼ 18-072-02 W	/6M	11U E 417770 N 6121417					
		Date	PF CF		Total		
Previous Inspec	tion:	30-May-2019	11	4		44	
Current Inspection:		27-May-2020	13	4		52	
Road AADT:		76	650	Year:	2019		
Inspected By:		Rishi Adhikari, ATDon Proudfoot, ThurberEd Szmata, ATNicole Wilder, ThurberDwayne Loewen, AT					
Report Attachments:		Photographs					
		✓ Plans ✓ Maintenance Items					
Primary Site Iss	ue:	the EBL of Hwy valley slope of Bezanson, Albe 3).					
Dimensions:		 Slide 1 (furthest to the west) was about 35 m wide but now Slide 1 and 2 are one slide about 115 m wide and extends about 30 m up the backslope from the EBL south ditch. The accumulated slide debris in the toe roll has now extended into the EBL south ditch (Photos 1 to 5). The backscarp is approximately 2.1 m high. Slide 2 has connected to Slide 1 and mentioned above and extends 30 m up the backslope with two sections of toe roll that have flowed into the EBL ditch (Photos 1 and 6). The backscarp is approximately 2.0 m high and there were two tension cracks that have developed into the backscarp now and connects to Slide 3. Slide 3 is about 108 m wide, extends 25 m up from the EBL south ditch margin up the backslope. The overall angle of the backslope is at about 20 degrees from the horizontal at Slide 3 (Photos 7 to 11). A portion of the toe roll is now in the EBL ditch. The backscarp ranges from 1.5 m to 3.0 m in height. Slide 4 (further to the east) has increased in size and is about 115 m wide, extends 40 m up the backslope with a distinct toe roll in the ditch (Photo 12). The backscarp varies from 1.0 m to 2.0 m high The overall angle of the backslope is about 20 degrees from the backscarp varies from 1.0 m to 2.0 m high 					
Maintenance:		horizontal at location of Slide 4. No maintenance has been performed recently at the site. To maintain ditch flow, slumped material that accumulates at the toe of the slides and overflows into the EBL south ditch will need to be periodically removed.					

Observations:	Description	Worsened?			
Pavement Distress Some longitudinal cracks were on observed in the ACP of the EBL in 2017 but it is not believed to be related to the backslope failures. No change in 2020.					
Slope Movement	All four slides have become more active this year. Slide 4 is more than double the size and there was fresh sloughed soil at all of the slides. Several tension cracks had developed into backscarps. Slide 4 has retrogressed further upslope and the backscarp is now beyond the fence line. The toe of Slide 4 is now partially blocking the highway ditch.	ব			
Erosion Some erosion and pooling of water was observed near the middle of Slide 3 and on the east side of Slide 4. The erosion near Slide 4 was approximately 1.5 m wide and 0.3 m deep.		र			
✓ Seepage	Water was panding on the outside of the teo roll of				
Bridge/Culvert Distress					
✓ Other	Toe rolls of several slides are now in EBL south ditch in several locations. To maintain ditch flow, slumped material that accumulates at the toe of the slides and overflows into the EBL south ditch will need to be periodically removed.	र			
	currently no monitoring instruments at this site				
Assessment: A review of available surficial geology maps for the site indicate that it is likely that the slumps in the backslope were initiated by a combination of loss of cohesion of the high plastic cohesive soils that compose the backslope soil stratigraphy, the overall slope steepness angles, (from 15 to 20 degrees from the horizontal) and slope height (10 m to 15 m). Thurber's experience in the Peace River indicates that this phenomenon can occur anywhere between 5 to 18 years following the exposures of the high plastic slopes to the elements for the first time. The loss of cohesion causes the formation of tension cracks in the high plastic soils. The degradation of the slopes then accelerates as surface run-off and precipitation water enters the tension cracks.					

As the backslope failures continue to grow, there is a high likelihood that additional debris material will accumulate into the EBL south side ditch obstructing groundwater flow causing water to pond at the toe of the slope. Ponded water will eventually saturate the slide debris material and exacerbate the landslide activity as a result and could infiltrate in to the highway subgrade, which could lead to a rapid deterioration of the pavement structure.

The slides are also progressing upslope and now are encroaching into privately owned land at the crest of the valley slope above the highway.

The most appropriate repair scenario should involve an overall flattening of the backslope angle to an angle of 4H:1V, or flatter, combined with the excavation/rebuilding of the backslope failures with imported soils and the installation of slope drains.

Recommendations:	Cost
Short Term The maintenance Contractor and the MCI should regularly inspect backslopes for backscarp retrogression and debris accumulation in EBL ditch.	Maintenance
Medium to Long Term The overall slope angle should be flattened, failed material in the backslope should be excavated prior to placing any new fill. The fill should be placed from the existing	\$400,000 to \$600,000

toe upwards to help buttress the existing slope. Subdrains should also be installed	(Cost of land
prior to placing the fill to drain the toe area of the slope.	Acquisition no
	included)



































