

## ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – PEACE-HIGH LEVEL 2020 INSPECTION

Site Number	Location	Name	Hwy	km			
PH60	East Hill	35+680 to 36+180 Site 3	2:60	35.95			
Legal Description	1	UTM Co-ordinates					
W28 & E29-083-2	1 W5M	11V E 483450	N 623139	95			

	Date	PF	CF	Total	
Previous Inspection:	3-Jun-2019	13	4	52	
Current Inspection:	9-Jun-2020	13	4	52	
Road WAADT:	4580		Year:	2019	
Inspected By:	Ed Szmata, TRANS Rocky Wang, TRANS		Don Proudfoot, TEL Tyler Clay, TEL		
Report Attachments:	Photographs				
	✓ Plans ✓ Maintenance Items			ce Items	

Primary Site Issue:	Large landslide referred to as Site 3 previously encompassed highway in the 1980s. The upslope area was mitigated by major crest unloading. Mitigated downslope of roadway by the construction of large toe berms. The area is still potentially unstable and ongoing shallow and deep-seated movements are occurring on the downslope side. The shallow slump (first observed in 2013) upslope of highway at 35+900 continues to slowly retrogress. Deep active gully erosion and headcutting is also occurring within engineered berm below roadway which is progressing slowly towards roadway. A new shallow landslide developed on the northeast facing backslope within a through-cut area at 35+835 since the 2015 inspection. Large slide movement and earth flow activity occurred in Spring 2016 between 35+700 to 35+900 approximately 150 m to 200 m below the roadway. The debris from these mass movements partially blocked the Heart River.
Dimensions:	Site 3 is 350 m wide; extends 200 m upslope of the roadway to just below the crest of the valley and between 200 m and 450 m downslope of the roadway to the Heart River. The 2013 shallow slump at 35+900 is 35 m wide and developed 100 m from the roadway. The new shallow landslide at 35+835 was 30 m wide at the main scarp and 40 m long extending to the south roadway ditch. The earth flow within the gully at 35+700 affects an area approximately 70 m wide (note there are two gully head sources) and 180 m long. The composite slide/flow at 35+900 is approximately 65 m wide and 165 m long with an exposed main



scarp several meters high (estimated from 2016		satellite imagery).			
Maintenance:	No maintenance activity since 2011.				
Observations:	Description	Worsened?			
Pavement Distress					
✓ Slope Movement	Retrogressive slumping along Heart River is actively continuing, which encompasses the lower portions of the large toe berm constructed downslope of the roadway to stabilize the initial landslide in this area. Areas have minor ongoing movement but no significant retrogression/lateral expansion of the main 2016 slide area which is offset approximately 180 m from highway (Photo 60-04). Overgrown scarp near 35+750 shows no sign of recent movement. Shallow slide/flow on the backslope at 35+830 had no further expansion and was not blocking ditch (Photo 60-05).				
✓ Erosion	Ongoing erosion at the culvert outlet and gullying immediately below roadway at 35+700 where there is active slumping on the gully walls (Photos 60-01 and 60-02). The erosion and gullying significantly expands approximately 125 m downslope of the highway where trees and thicker vegetation becomes sparse. Ongoing minor retrogression but no significant expansion since 2019 and gully sidewalls are intermittently stable (Photo 60-03). Minor ditch erosion is ongoing on both sides of the roadway between 35+850 and 36+000. At 36+050, gully erosion is ongoing on the upper portion of the toe berm below the roadway (Photos 60-06).				
C Seepage					
✓ Bridge/Culvert Distress	Erosion ongoing at culvert outlet at 35+700 which is believed to be at least partially blocked (Photo 60-01).				



C Other						
Instrumentation:						
SI12 - No discernible movem	ent.					
SI13 – Creep (0.5 mm/yr) ove	er 2.2 m to 4.7 m depth.					
SI88 - No discernible movem	ent.					
SI89 – 10 mm/yr over 11.8 m cumulative movement of 107	SI89 – 10 mm/yr over 11.8 m to 13.6 m depth (-3.4 mm/yr since the Fall 2019 readings), with total cumulative movement of 107 mm					
SI91 – Sheared at 13.1 m be	low top of casing as of Spring 2019					
Assessment:						
Assessment: Small deep-seated movements are occurring within the fill embankment downslope of roadway indicating low Factors of Safety. The Spring 2019 instrument readings indicate an accelerating trend of annual movement rate since 2016 from an average of approximately 7 mm/yr between the years 2013 to 2016 to approximately 10 mm/yr since 2017 at SI 89. Prior to 2016, the average annual movement rate was approximately 3 to 4 mm/yr (not including initial movement picked up within the same year following the SI installation in 1996). Additionally, SI 91, located in the base of the valley slope was sheared off at 13.1 m depth in the recent Spring 2019 readings. Surficial observations of active movement have not been observed in these areas, likely due to the relatively small magnitude, but the risk level for this site has been adjusted accordingly due to the relatively small magnitude, but the risk level for this site has been adjusted accordingly due to the concerning trend of increased movement rates. A significant volume of material has been displaced from the valley toe in localized areas due to gullying and mass soil flow events and could be a factor in this trend. Increased movement rates at SI 89 may be driven by a loss of toe material at the erosion gully to the south. In general, the shallow slumping upslope of roadway is active but currently has limited to no effect the highway. The landslide within the backslope at 35+835 does not pose a risk to the roadway; however, as displaced material continues to move into the ditch; water flow could be blocked and require maintenance. The mass movement near the valley base below 35+900 is expected to retrogress upslope and could cause instabilities that could potentially impact the highway in the future as toe support is lost. No slope instrumentation exists above this area to monitor potential impacts to the highway if movement were to occur. The erosion gullies at 36+075 and 35+700 do not threaten the highway in their current state but a						
Recommendations:		Cost				



Continue to monitor instruments twice yearly and undertake annual inspections.	
Consider redirecting surface runoff further downslope of headcutting/gullying that is occurring at 36+000 and 35+700. Consider repairing the shallow slope failures at 35+900 and 35+835. This may include removal of loose soil and backfill using granular fill; and protecting the exposed surface with liner and soil cover to allow vegetation to grow.	\$300,000



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13351-C4A



	LEGEND:	
1	HORIZONTAL CHAINAGE (37+130 GROUARD BRIDGE)	35+900
Pe		
	PHOTOGRAPH LOCATION	
	SLOPE INCLINOMETER	
	- NO MOVEMENT	SI 64
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-	NOTES:	
N	1 DRAWING MUST BE USED IN CONJUNCTION WITH THE	ATTACHED REPORT
2	REFERENCE 13351 DATED DECEMBER 2020 AND IS S	UBJECT TO THE
3		
$\downarrow$	2 AIR BASE FROM TARIN RESOURCE SERVICES LTD. 0.4	m/PIXEL (2012).
	3 SLIDE FEATURES, PHOTOGRAPHS AND CHANIANGE ARE S	SHOWN APPROXIMATE ONLY.
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	PEACE RIVER EAST I	4ILL
	HWY 2.60 (PH60) STA 35+680	TO 36+180
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		FIGURE PH60-2
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112	DECEMBER 10, 2020 THURBER FNGI	NEERING LTD.





Photo 60-01. Close-up of hanging culvert outlet (35+700). Ongoing erosion below culvert and rilling above. No major change from the 2019 condition.













Photo 60-06. Headcutting and erosional gullying occurring below roadway (36+050).