ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – SWAN HILLS 2020 INSPECTION



Site Number	Location	Name	Hwy	km	
SH006-1	North of Swan Hills	Klumph Creek	33:14	18.1- 18.6	
Legal Description		UTM Co-ordinates			
NE28/SE33-70-9-W5M		11U E 608,502	N 6,107,1	07	

	Date	PF	CF	Total
Previous Inspection:	10-Jun-2019	10	4	40
Current Inspection:	1-Jun-2020	10	4	40
Road AADT:	670		Year:	2020
Inspected By:	Rocky Wang, TRANS Ed Szmata, TRANS Rodney Johnston, TRANS		Ken Froese, Thurber	
Report Attachments:	✓ Photographs✓ Plans✓ Maintenance Items		ltems	

Primary Site Issue:	Highway crosses active slide area approximately 700 m in width over an overall slope height of 57 m. Movement appears to be 6 m to 13 m deep in upper portion of weathered bedrock or in bottom of clay overburden at bedrock contract likely triggered initially by toe erosion by the highly-mobile Swan River. Movement is manifested on the highway at each end of the scarp: transverse cracking at the north and culvert distress at the south.	
Dimensions:	500 m of highway length with an embankment fill height between 4 m to 10 m in height (north to south).	
Date of Remediation:	1988: 40 m wide portion of west slope of embankment failed damaging both culverts which was repaired. 2001: Construction of toe berm (600 m long) with sand subdrains using excavated material (390,000 m³) from upslope areas for off-loading. Outlet of 1200 mm and 900 mm culverts at Klumph Creek repaired and extended.	
Maintenance:	2016: ACP patch placed over cracks at north end of site. 2017: Overhead powerline installed on east side of highway.	

Observations:	Description	Worsened?
▼ Pavement Distress	Crack pattern at north end of site has reflected through patch. Deterioration of driving surface observed over most of the highway length.	·
Slope Movement	Ongoing movement observed in instruments and confirmed by pavement distress; possible skin slide observed near culvert inlets in 2019.	\S
▼ Erosion	Erosion at outlet of north culvert riprap apron noted in 2014 deepened in 2017 and again in 2019.	Þ
✓ Seepage	Soft/wet areas sometimes observed along ditches in north half of site.	
	Sinkhole (3 m by 2 m) observed in 2013 has not changed in size.	

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□ Other			
Instrumentation (as of Fall 2020):			
SI10	Ongoing movement at 10.5 m depth with a current rate of about 5.9 mm/year compared to the average rate (since 2013) of about 2.2 mm/year with cumulative movement of 62 mm. The rate of movement has noticeably accelerated in the last year.		
SI18-30 to -35	14 mm and 14 Fall 2019. Prodepth and S cumulative movement zo zone at 5.4 m	movements zones have been identifier and SI18-31 (13.3 m depth) with cumulative 4 mm, respectively, which is an increase of 9 mm evious tentative movement zones identified in SI 118-33 at 16.7 m depth have resolved into accovements of 8 mm. After some initial movement, one at SI18-35 has not developed further; the pot depth appears to be resolving with 4 mm of cumul pattern has been apparent yet at SI18-34.	movements of and 8 mm since 18-32 at 15.5 m tive zones with the near-surface ential movement
SP00-2, SP00- 6A, SP00-6B	SP00-2 and SP00-6B have shown an upward trend over the last two years and are now at or above historical high water levels. The steady increasing trend at SP00-6A that was been ongoing since 2001 appears to have stabilized in Fall 2019 at a historical high.		
PN18-30 to -35	in Fall 2019 o	n 2018 piezometers increased from installation to r Spring 2020 and have now begun to decrease w al water levels.	
Damaged/ Destroyed	SI11 (5.7m de	pth), SI00-5, SI00-6, SP00-5 (unable to locate)	

Assessment:

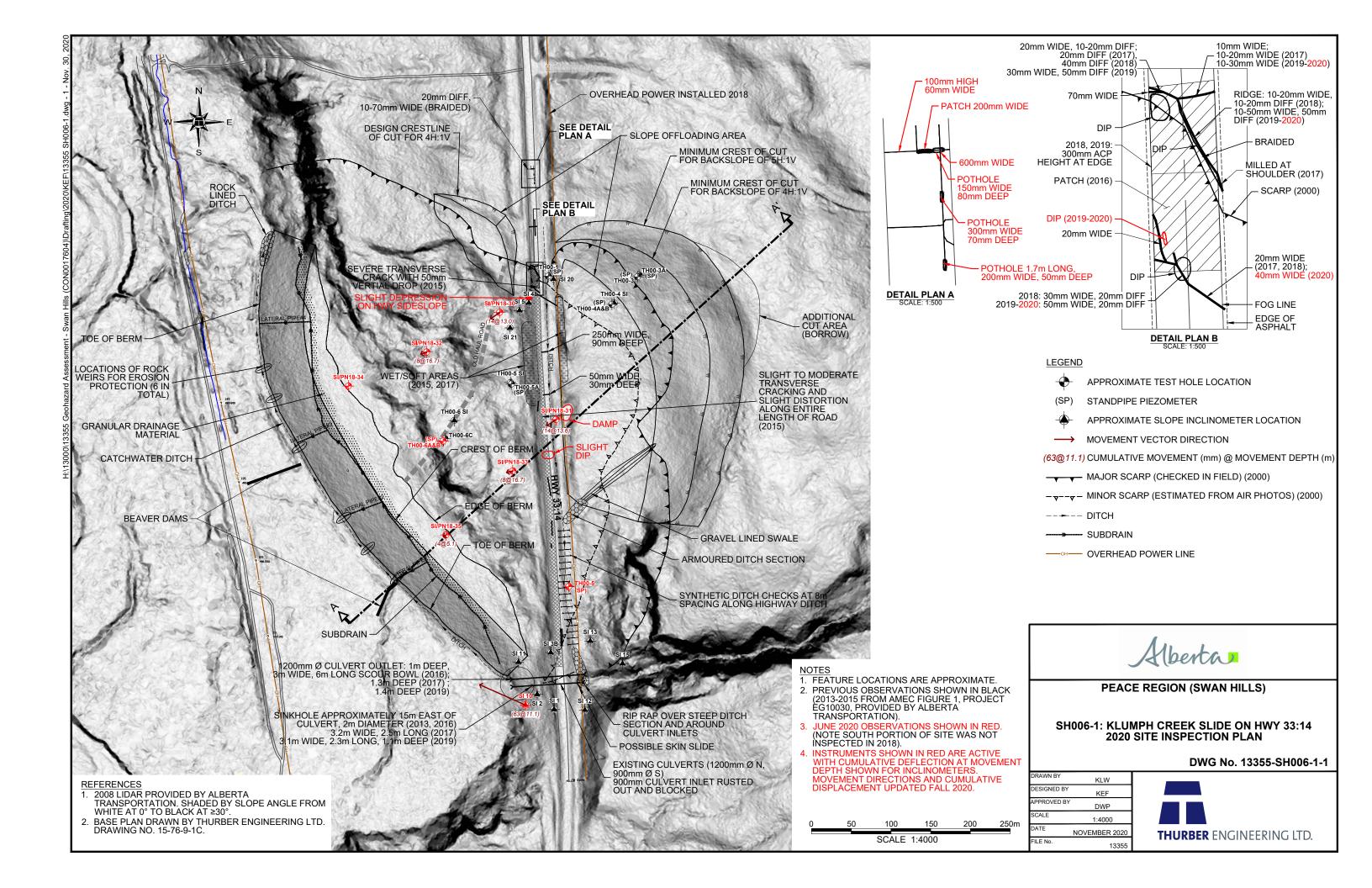
The landslide is still active albeit at a slow, creeping rate of movement. Instrumentation installed in March 2018 initially measured slow movements near the highway at about 13 m to 14 m depth and has now, in 2020, identified movement zones further downslope from the highway. Asphalt milling and patching is still required about every two to three years to maintain the roadway at the north end of the site where it crosses the landslide scarp although has not been undertaken for a couple of years and the highway surface is noticeably deteriorating. Milling should be undertaken to reduce the humps at the north end of the site. Cracks have widened slightly, and the differential height is increasing in the absence of routine maintenance. A new dip in the highway profile was observed in 2020 approximately in line with SI18-35.

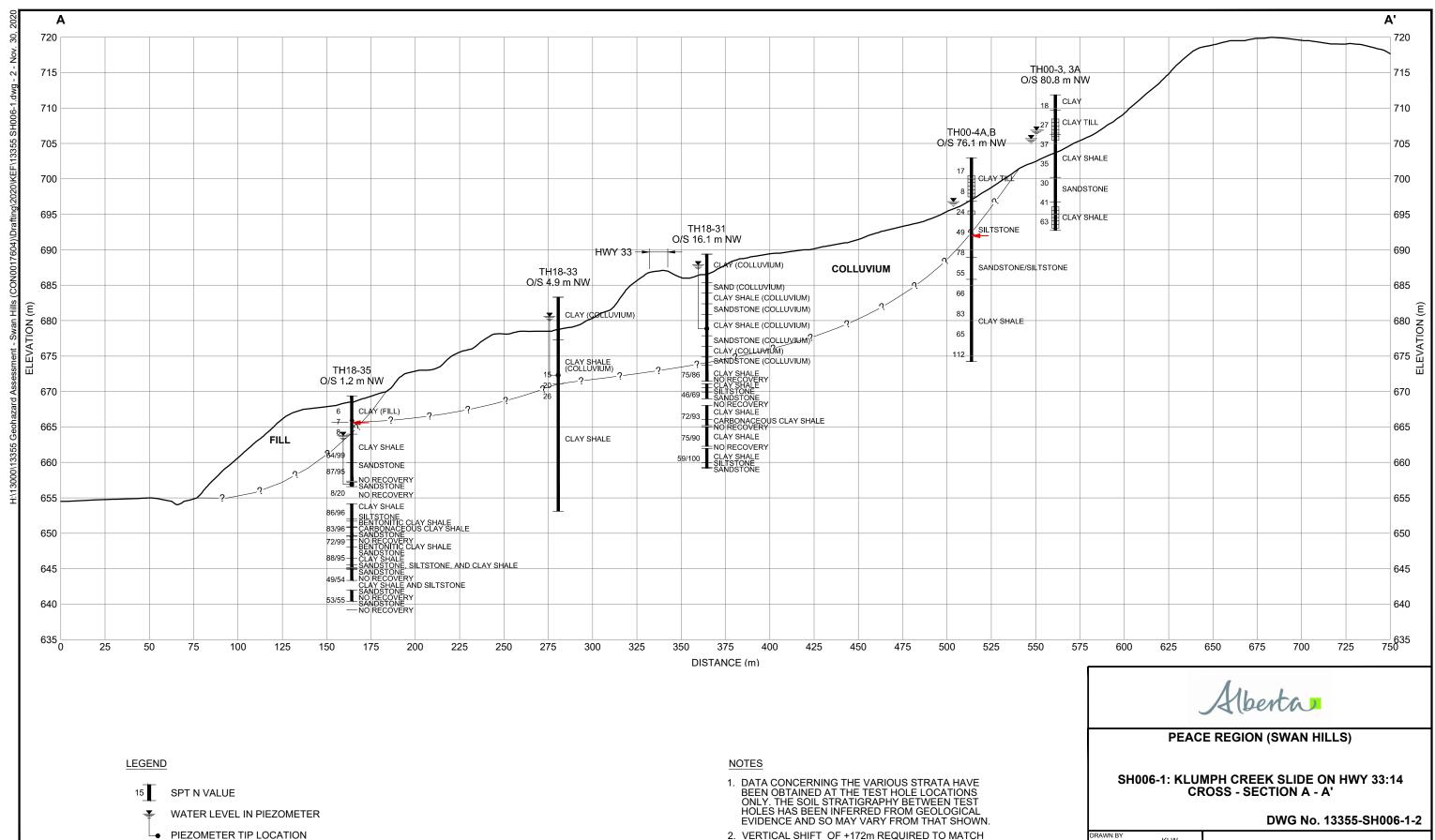
Recommendations:

Short-term road maintenance (patching and milling to provide a safe, smooth surface) should continue as required and the annual GeoHazard inspection and twice-annual instrumentation readings should continue as scheduled.

The recommended instrumentation was installed in early 2018 and are starting to reveal ongoing creep movement patterns. Given the large size of the landslide and slow movements, periodic patching of the asphalt at each flank appears to be the current most cost-effective method of dealing with the landslide movements. However, continued monitoring of the site (visual and instrumentation) is recommended to manage the risks.

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STANDPIPE PIEZOMETER SCREENED INTERVAL

→ ZONE OF MOVEMENT IN SLOPE INCLINOMETER

 VERTICAL SHIFT OF +172m REQUIRED TO MATCH ORIGINAL GROUND (FROM THURBER DRAWING 15-76-9-1, MAY 2000) TO GEODETIC ELEVATION.

DRAWN BY	KLW
DESIGNED BY	KEF
APPROVED BY	DWP
SCALE	H 1:2000 V 1:50
LAST UPDATED	AUGUST 201
FILE No.	1335





Photo 1 – Looking south from east shoulder at main scarp crack and patch at north end of site.



Photo 2 – Looking south from west shoulder at main scarp crack and patch at north end of site.

Note powerline installed on east side of highway since 2017 visit.

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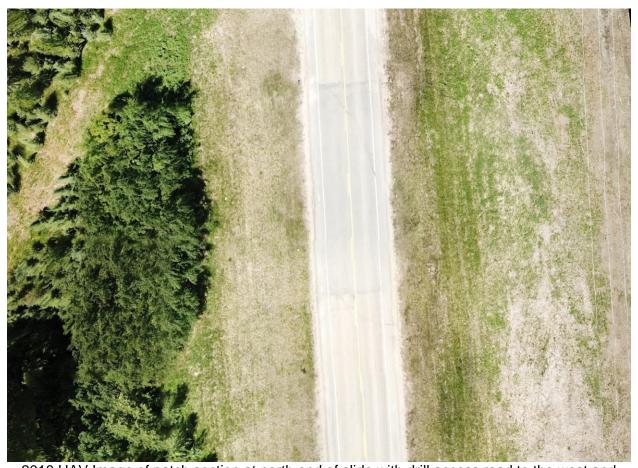


Photo 3: Looking north at hump over main scrap crack at north end of the site.



Photo 4: Looking south at crack at south end of patch at north end of the site.

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2018 UAV Image of patch section at north end of slide with drill access road to the west and new powerline on the east.

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