ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – SWAN HILLS 2020 INSPECTION



Site Number	Location			Name		Hwy	km				
SH008-1	2 km E of	Wati	no Bridge	Watino East Hill		49:08	18.80-19.05				
Legal Description				UTM Co-ordinat							
NE26-77-24-W5M				11U E 462,307			N 6,173,153				
			Date	PF	CF		Total				
Previous Inspect	Previous Inspection:		2-Jun-2019	12	2		24				
Current Inspection		3	-Jun-2020	12	2		24				
Road AADT:			87		Year:		2020				
Inspected By:		Roc Ed 3	Rocky Wang, TRANS Ken Froese, Thurber Ed Szmata, TRANS								
Report Attachments:			Photographs								
	Report Attachments.		Plans Maintenance Items								
Primary Site Issue:			Rotational, retrogressive failures in 10 m high backslope.								
Dimensions:			250 m length of backslope slumping on south side of highway.								
Date of Remediation:			 1993: Highway re-routed to this current alignment. 1994: Assessment of subdrains in north slope found them to be working well. 1999: Design undertaken for French drains in south backslope slump area but not constructed. 2001: Gravel placed to buttress upper portion of Slump A and lower portion of Slump C and placed 3 m wide riprap lining in ditch. 2003: Rip-rap-lined channel constructed on lower half of Slump B. 								
Maintenance:			 2004: Additional stone added to south ditch east of Slump C. 2006: Rip-rap placed in north ditch. 2015: Slumped material in the south ditch removed. 2019: Site regraded to open up south ditch; north sideslope also regraded 								
Observations:					Worsened?						
Pavement D	Pavement Distress										
Slope Movement			Four separ- backslope.	south							
Erosion			Slumped material removed from the ditch, leaving bare soil exposed.								
Seepage			Seepage noted in the backslope and accumulating in grabens.								
Bridge/Culvert Distress		ss	Outlet of dra								
✓ Other			Two pipeline the backslop								
Instrumentation:											
None.											

Assessment:

This site is located at the crest of the Smoky River valley; however, the backslope slumping appears to be the result of seepage from exposed sandy/silty layers in the backslope rather than deep-seated instability due to valley movements. At the present time, there is no impact to the highway surface as the depth of failure is contained within the height of the backslope. Records in Alberta Transportation files indicate that this area was deemed to have a factor of safety of 1.25 at a 3H:1V inclination. South slopes to the west and east of this localized area are apparently stable although not quite as high; the north slope is of a similar height and also appears stable. This may indicate a concentration of weaker soils on the south side of this cut, that the drainage measures implemented in the north backslope at the time of construction have been effective, or that the direction of horizontal groundwater flow is a contributing factor.

In the spring of 2019, the maintenance contractor excavated the toe rolls to improve ditch drainage. The material was wasted higher up on the slopes. The contractor also added two swales on the slope to assist with drainage from the sag ponds further up the slope. The grading work obscured some of the slide features and may also contribute to local instability at the toe due to the overall steepening of the lower portion of the backslope. However, it solved the ditch drainage issue. In 2020, there was deterioration of the toe of this regrading with some sloughing and erosion observed. The upper portions of the fill are starting to revegetate.

The uppermost scarp of Slump A did not appear to have regressed since 2016; however, signs of fresh vertical movement were observed in 2017 and 2018. Further movement was observed in 2019 including a new tension crack about 2 m further upslope on the east flank of the slump and minor regression at line B. The tension crack had widened as of 2020 and some material had fallen in the slide. In 2017, one of the pins in line B was removed from the slide mass and placed 5 m further back of pin B1. The encroachment into the ditch had increased in 2017 and 2018; regrading in 2019 has pushed the toe further back into the slope. Slump B appears relatively unchanged since 2018 although a mid-slope toe appeared more defined. The slumps further west (D and E) appeared to be relatively unchanged since 2016 although additional cracks were mapped in 2018 (they may have been present before but not recorded) and toe rolls heights were measured in 2019. Seepage is accumulating at the tops of some of the slump blocks which will further reduce the strength of the slumped material and lead to further movement. With the continued seepage and the continued removal of material from the ditch (whether eroded by water or excavated to maintain ditch flow), the slumps will likely retrogress as toe support is lost. The rate of regression of the top of the slumps is difficult to estimate and it is recommended that a remediation option be developed along with a minimum horizontal offset to the pipelines that would trigger its construction.

An underground utility locate was undertaken in 2018 to identify the locations of the pipelines at the top of the slope. The TransCanada pipeline (TCPL) was closest at an offset 3.9 m south of Pin A1 and 0.1 m south of Pin B3. The East Peace Gas Co-op natural gas line is located further south of the TCPL line.

Recommendations:

Short-Term:

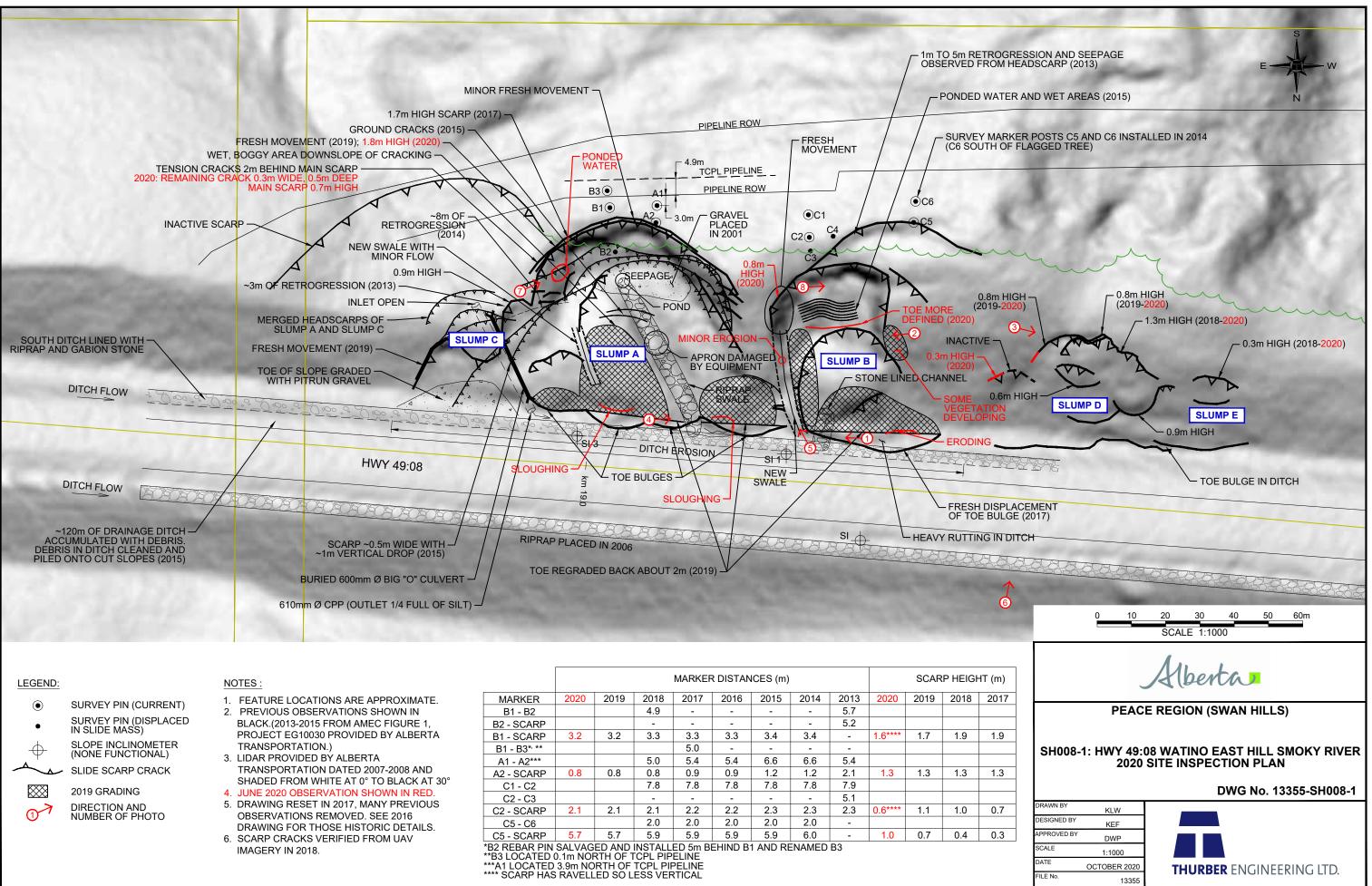
- Remove material from the ditch when required to maintain flow but do not place the excavated material back onto the slide mass as it will load the slide. Augment the ditch bottom rip-rap to minimize downcutting and erosion of the recently-graded faces of the slide toe rolls. Excavated material from the regrading in the spring of 2019 should be removed to reduce the potential for local instability.
- Establish a line of communication with the pipeline owners to determine risk tolerance and minimum setback distance.

Long-Term:

 Develop remediation options such as: flattening of the backslope with a buried culvert along the ditch (so that the toe of the slope can be moved to the north), reconstruct slope with gravel material or select clay with French drains and subdrains, install a groundwater cut-off trench at the top of the slope.

Ongoing Investigation:

- It is recommended that the annual GeoHazard inspection should continue as scheduled.
- A geotechnical drilling program may be required depending on the remediation option(s) considered.



	MARKER DISTANCES (M)								SCARP HEIGHT (M)				
MARKER	2020	2019	2018	2017	2016	2015	2014	2013	2020	2019	2018	2017	
B1 - B2			4.9	-	-	-	-	5.7					
B2 - SCARP			-	-	-	-	-	5.2					
B1 - SCARP	3.2	3.2	3.3	3.3	3.3	3.4	3.4	-	1.6****	1.7	1.9	1.9	
B1 - B3* [,] **				5.0	-	-	-	-					
A1 - A2***			5.0	5.4	5.4	6.6	6.6	5.4					
A2 - SCARP	0.8	0.8	0.8	0.9	0.9	1.2	1.2	2.1	1.3	1.3	1.3	1.3	
C1 - C2			7.8	7.8	7.8	7.8	7.8	7.9					
C2 - C3			-	-	-	-	-	5.1					
C2 - SCARP	2.1	2.1	2.1	2.2	2.2	2.3	2.3	2.3	0.6****	1.1	1.0	0.7	
C5 - C6			2.0	2.0	2.0	2.0	2.0	-					
C5 - SCARP	5.7	5.7	5.9	5.9	5.9	5.9	6.0	-	1.0	0.7	0.4	0.3	
*B2 REBAR PIN SALVAGED AND INSTALLED 5m BEHIND B1 AND RENAMED B3 **B3 LOCATED 0.1m NORTH OF TCPL PIPELINE ***A1 LOCATED 3.9m NORTH OF TCPL PIPELINE													





Photo 1 – Looking east at toes of Slumps B and A which were excavated in 2019 to improve ditch flow with the material placed back onto the slide masses.



Photo 2 – Looking southeast at the top of Slump B. Disturbances from 2019 regrading becoming vegetated.





Photo 3 – Looking west at crest of Slump D



Photo 4 – Looking west at the toe of Slump B.





Photo 5: Recent grading (2019) of lower portion of the slope at toe of Slump B becoming vegetated.



Photo 6: Looking southwest at Slump D (by R. Wang, TRANS).





Photo 7: Looking south at recent movement on the east flank of Slump A.



Photo 8: Looking west at top of Slump B.