ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION (PEACE RIVER DISTRICT) 2021 INSPECTION



Site Number	Location		Name		Hwy	km			
SH008-1	2 km E of	Watino Bridge	Watino East Hill		49:08	18.80-19.05			
Legal Description	า	UTM Co-ordinates							
NE26-77-24-W5M			11U E 462,30)7	Ν	6,173,153			
		_							
		Date	PF	CF		Total			
Previous Inspection:		3-Jun-2020	12	2		24			
Current Inspection:		28-Jun-2021	10	2		20			
Road AADT:		79	Year:		2020				
Inspected By:		Rocky Wang, TR	ANS	Barry Meays, Thurber					
		Ed Szmata, TRA	NS	Mark Gallego, Thurber					
		Max Shannon, TF	RANS		-				
		Chase Millegen,	TRANS						
-		Photographs							
Report Attachme	ents:	Plans		🗌 Maintena	nance Items				

Primary Site Issue:	Rotational, retrogressive failures in 10 m high backslope.						
Dimensions:	250 m length of backslope slumping on south side of highway.						
Date of Remediation:	 1993: Highway re-routed to this current alignment. 1994: Assessment of subdrains in north slope found them to be working well. 1999: Design undertaken for French drains in south backslope slump area but not constructed. 2001: Gravel placed to buttress upper portion of Slump A and lower portion of Slump C and placed 3 m wide riprap lining in ditch. 2003: Rip-rap-lined channel constructed on lower half of Slump B. 2020: Pavement overlay placed on valley hill section of highway through this site 						
Maintenance:	 2004: Additional stone added to south ditch east of Slump C. 2006: Rip-rap placed in north ditch. 2015: Slumped material in the south ditch removed. 2019: Site regraded to open up south ditch; north sideslope a regraded 						
Observations:	Description	Worsened?					
Pavement Distress							
Slope Movement	Four separate slump blocks in the south backslope.						
Erosion	Slumped material removed from the ditch, leaving bare soil exposed.						
✓ Seepage	Seepage noted in the backslope and accumulating in grabens.						
Bridge/Culvert Distress	Outlet of drainage pipe partially obstructed.						
✓ Other	Two pipelines are located immediately south of the backslope.						

Instrumentation:

None.

Assessment:

This site is located at the crest of the Smoky River valley; however, the backslope slumping appears to be the result of seepage from exposed sandy/silty layers in the backslope rather than deep-seated instability due to valley movements. At the present time, there is no impact to the highway surface as the depth of failure is contained within the height of the backslope. South slopes to the west and east of this localized area are apparently stable although not quite as high; the north slope is of a similar height and also appears stable. This may indicate that the direction of horizontal groundwater flow is a contributing factor, as well as a shaded north facing slope that stays wetter than the south facing one.

In the spring of 2019, the maintenance contractor excavated the toe rolls to improve ditch drainage. The material was wasted higher up on the slopes. The contractor also added two swales on the slope to assist with drainage from the sag ponds further up the slope. The grading work obscured some of the slide features and may also contribute to local instability at the toe due to the overall steepening of the lower portion of the backslope. However, it solved the ditch drainage issue. In 2020, there was deterioration of the toe of this regrading with some sloughing and erosion observed, however the site hadn't gotten worse in 2021 The upper portions of the fill are starting to revegetate.

An underground utility locate was undertaken in 2018 to identify the locations of the pipelines at the top of the slope. The TransCanada pipeline (TCPL) was closest at an offset 3.9 m south of Pin A1 and 0.1 m south of Pin B3. The East Peace Gas Co-op natural gas line is located further south of the TCPL line.

Overall, there does not appear to be significant changes since the 2020 inspection perhaps due to the overall drier weather this year.

Recommendations:

Short-Term:

- Remove material from the ditch when required to maintain flow but do not place the excavated material back onto the slide mass as it will load the slide. Augment the ditch bottom rip-rap to minimize downcutting and erosion of the recently-graded faces of the slide toe rolls.
- Establish a line of communication with the pipeline owners to determine risk tolerance and minimum setback distance.

Long-Term:

 Develop remediation options such as: flattening of the backslope with a buried culvert along the ditch (so that the toe of the slope can be moved to the north), reconstruct the slope with gravel material or select clay with French drains and subdrains, install a groundwater cut-off trench at the top of the slope.

Ongoing Investigation:

- This site has been slated for formal inspections twice under the current contract. This seems
 reasonable as not much change has been observed since the 2020 inspection. However, an extra
 inspection should be carried out if the MCI notes any significant changes prior to the next
 scheduled inspection.
- A geotechnical drilling program is recommended if the long-term remediation option(s) are considered.

Closure

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Don Proudfoot, P.Eng. Principal | Senior Geotechnical Engineer

Mark Gallego, P.Eng. Geotechnical Engineer



STATEMENT OF LIMITATIONS AND CONDITIONS

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This Report has been prepared in accordance with generally accepted engineering or environmental consulting practices in the applicable jurisdiction. No other warranty, expressed or implied, is intended or made.

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All documents, records, data and files, whether electronic or otherwise, generated as part of this assignment are a part of the Report, which is of a summary nature and is not intended to stand alone without reference to the instructions given to Thurber by the Client, communications between Thurber and the Client, and any other reports, proposals or documents prepared by Thurber for the Client relative to the specific site described herein, all of which together constitute the Report.

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The Report has been prepared for the specific site, development, design objectives and purposes that were described to Thurber by the Client. The applicability and reliability of any of the findings, recommendations, suggestions, or opinions expressed in the Report, subject to the limitations provided herein, are only valid to the extent that the Report expressly addresses proposed development, design objectives and purposes, and then only to the extent that there has been no material alteration to or variation from any of the said descriptions provided to Thurber, unless Thurber is specifically requested by the Client to review and revise the Report in light of such alteration or variation.

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- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

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	MARKER DISTANCES (m)								SCARP HEIGHT (m)					
MARKER	2021	2020	2019	2018	2017	2016	2015	2014	2013	2021	2020	2019	2018	2017
B1 - B2				4.9	-	-	-	-	5.7					
B2 - SCARP				-	-	-	-	-	5.2					
B1 - SCARP	3.2	3.2	3.2	3.3	3.3	3.3	3.4	3.4	-	1.6****	1.6****	1.7	1.9	1.9
B1 - B3* [,] **					5.0	-	-	-	-					
A1 - A2***				5.0	5.4	5.4	6.6	6.6	5.4					
A2 - SCARP	0.8	0.8	0.8	0.8	0.9	0.9	1.2	1.2	2.1	1.3	1.3	1.3	1.3	1.3
C1 - C2				7.8	7.8	7.8	7.8	7.8	7.9					
C2 - C3				-	-	-	-	-	5.1					
C2 - SCARP		2.1	2.1	2.1	2.2	2.2	2.3	2.3	2.3		0.6****	1.1	1.0	0.7
C3 - SCARP	0.5													
C5 - C6				2.0	2.0	2.0	2.0	2.0	-					
C5 - SCARP		5.7	5.7	5.9	5.9	5.9	5.9	6.0	-		1.0	0.7	0.4	0.3



Photo 1 – Looking east at toes of Slumps B and A which were excavated in 2019 to improve ditch flow with the material placed back onto the slide masses.



Photo 2 – Looking southeast at the top of Slump B. Disturbances from 2019 regrading becoming vegetated.



Photo 3 – Looking west at crest of Slump D



Photo 4 – Looking west at the toe of Slump B.



Photo 5: Recent grading (2019) of lower portion of the slope at toe of Slump B becoming vegetated.



Photo 6: Looking southwest at Slump D.



Photo 7: Looking west at top of Slump B.