ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION (PEACE RIVER DISTRICT) **2021 INSPECTION**



Site Number	Location	Name	Hwy	km
SH004-1	South of Little Smoky River	Little Smoky River (South of Bridge)	49:12	2.3-2.6
Legal Description		UTM Co-ordinates		
W33-074-21-W5N	1	11U E 489,224	N 6,145	5,128

	Date	PF	CF	Total
Previous Inspection:	2-Jun-2020	13	6	78
Current Inspection:	28-Jun-2021	13	6	78
Road AADT:	1290		Year:	2020
Inspected By:	Rocky Wang, TRANS Ed Szmata, TRANS Max Shannon, TRANS Chase Milligen, TRANS		Barry Meays, Thurber Mark Gallego, Thurber	
Report Attachments: Photographs Plans			☐ Maintenance	Items

Primary Site Issue:	Highway traverses deep-seated (about 35 m), retrogressive landslide with ongoing creep movement over the entire valley slope due partly to erosion at toe by the Little Smoky River.		
Dimensions:	300 m length of highway affected by several intersecting scarps resulting in uneven riding surface. Approx. 1.3 km of the highway crosses this unstable west valley slope.		
Date of Remediation:	1987: Subdrain installed in backslope ditch. 1998: Overlay including building up downslope shoulder with gravel fill buttress and raising of the guardrail. 2006: Repair of erosion damage and installation of surface drains to nearby sag pond. Fall 2020: Pavement overlay and guardrail replacement		
Maintenance:	Routine annual or bi-annual asphalt patching and milling 2016: Patch placed in August, milled in late September 2017: Patch and milled in August/September 2018: Milling removed up to about 60 mm of asphalt Fall 2019: Milling both sides of valley for about 172,000 m ³		

Observations:	Description	Worsened?
Pavement Distress	Cracking and uneven roadway surface requires ongoing patching and milling especially at the landslide scarp cracks and the south end of the bridge.	V
☐ Slope Movement	Overall slope movement continues resulting in cracks at scarps and sags in the graben blocks.	<
☑ Erosion	Riling between riprap channel and beyond P01-6. Lesser riling near north end of patch.	
✓ Seepage	Evidence of seepage observed at culvert outlet at north end of site.	

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☑ Bridge/Culvert Distress		Separation of plastic culvert was 0.6 m wide in 2016 and decreased to 0.3 m in 2017 and 2018.	
□ Other			
Instrumentation (as of Spring 2020):			
VW07-2, VW07-2A, VW07-3, VW07-3A	Relatively stable with a slight increasing trend at VW07-3, VW07 3A over the last four years.		
Destroyed/Lost	VW07-2B and -3B (damaged Fall 2014); SI4A, SI01-6, SI01-7, SI01-2, SI07-3B, SI01-9, SI07-2B, PZ01-6, PZ01-2 (discontinued)		

Assessment:

The overall valley slope is moving as several separate slide blocks resulting in numerous scarps, sag ponds, and differential movement zones and the highway is intersected at several locations by these features resulting in an uneven highway surface. The driving mechanism appears to be toe erosion by the Little Smoky River although stability analyses undertaken by others indicate that a high ground water table may also be contributing. Based on GPS survey of the InSAR points conducted by Alberta Geological Survey (AGS Open Report 2013-14), this portion of the highway is located on landslide blocks moving relatively slower (5 mm to 40 mm per year) as compared to the north side (SH003) or the bridge site (SH016). Drawing 32121-SH004-1-1 shows some of the local slide features that have been interpreted from the 2008 LiDAR imagery. Regrading of the slopes at this site has obscured the scarps of the larger slide blocks that are likely present beneath the highway alignment.

The ongoing movement of the valley slope results in continued deformation of the highway surface that requires frequent patching and required an overlay in the summer of 2020 to maintain the smoothness of the highway.

The riling in the bare area on the downslope side of the highway to the north of the riprap apron appears to in a state similar to the previous inspection. These rill and gully areas should be repaired to prevent further degradation. There is a second area of rilling toward the north which is not quite as deep also appears to be in a state similar to the previous inspection.

There is a break in the plastic culvert that handles downslope ditch flow from the south that was about 0.6 m wide in 2016 and only 0.3 m in 2017 which may indicate some lateral shifting of the pipe. There is a small sinkhole below the break. Similar conditions were observed in 2018, 2019 and 2021. A second gap at a culvert joint was observed in 2019 about 10 m further north.

Recommendations:

Short-term:

- Road maintenance should continue as necessary to maintain an even, safe roadway surface and may consist of milling, patching, and crack sealing of the pavement. An asphalt overlay was placed through the site in Summer 2020, but rapid slide movements have already cracked and distorted it.
- The bare areas on the downslope side of the highway to the north of the riprap apron have the potential to deteriorate further. This area should be regraded and covered with topsoil and seed secured with an erosion control blanket or seed-impregnated compost blanket. The second riling area near the north end of the site could be repaired at the same time with the same methodology.
- The break in the culvert should be excavated and repaired to limit infiltration into the slope which could potentially result in slope movement or significant erosion.

Long-Term:

The two alternatives for this location are: to realign the highway either using the existing bridge crossing or constructing a new one on more stable ground; or to control river erosion at the toe of the slope such that remedial measures above will have a longer effectiveness. It is understood that AMEC prepared a report under the High Water Related Mitigation Works program providing recommendations for erosion control at the toe and drainage measures on the slope for the sag

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ponds. In a previous study, Thurber identified a more-stable road alignment option going up a deep cut in the valley slope straight west off the end of the bridge.

Ongoing Investigation:

- It is recommended that the annual GeoHazard inspection and twice-annual instrumentation readings should continue as scheduled.
- Since there are no longer any operable Sl's on site, consideration should be given to adding two to three slope inclinometers if there are other investigation projects in the area such that drilling costs could be shared so that the rate of creep can be monitored.
- Consideration should also be given to re-surveying the InSAR (interferometric synthetic aperture radar) targets, perhaps annually, to supplement the work done by the AGS as this will provide an overall view of ground movements.
- A GPS real-time ground movement system (Geocube), that is less expensive than the current systems, may be an option worth considering at this site particularly for identifying lower-movement rate zones for potential realignment.

Closure

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Don Proudfoot, P.Eng. Principal | Senior Geotechnical Engineer

Mark Gallego, P.Eng. Geotechnical Engineer

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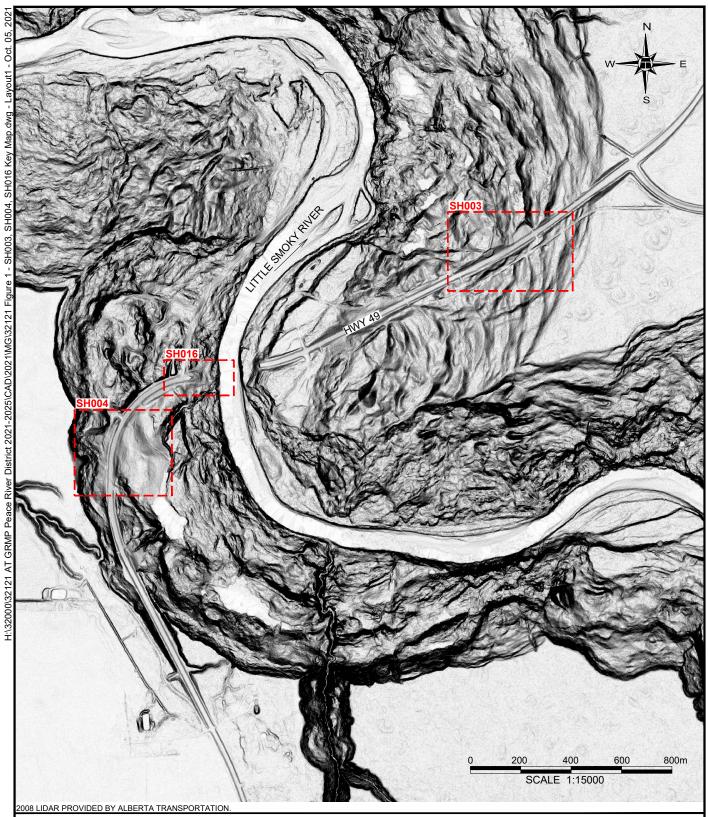
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- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

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Geotechnical engineering and environmental consulting projects often have the potential to encounter pollutants or hazardous substances and the potential to cause the escape, release or dispersal of those substances. Thurber shall have no liability to the Client under any circumstances, for the escape, release or dispersal of pollutants or hazardous substances, unless such pollutants or hazardous substances have been specifically and accurately identified to Thurber by the Client prior to the commencement of Thurber's professional services.

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PEACE REGION (PEACE RIVER DISTRICT)

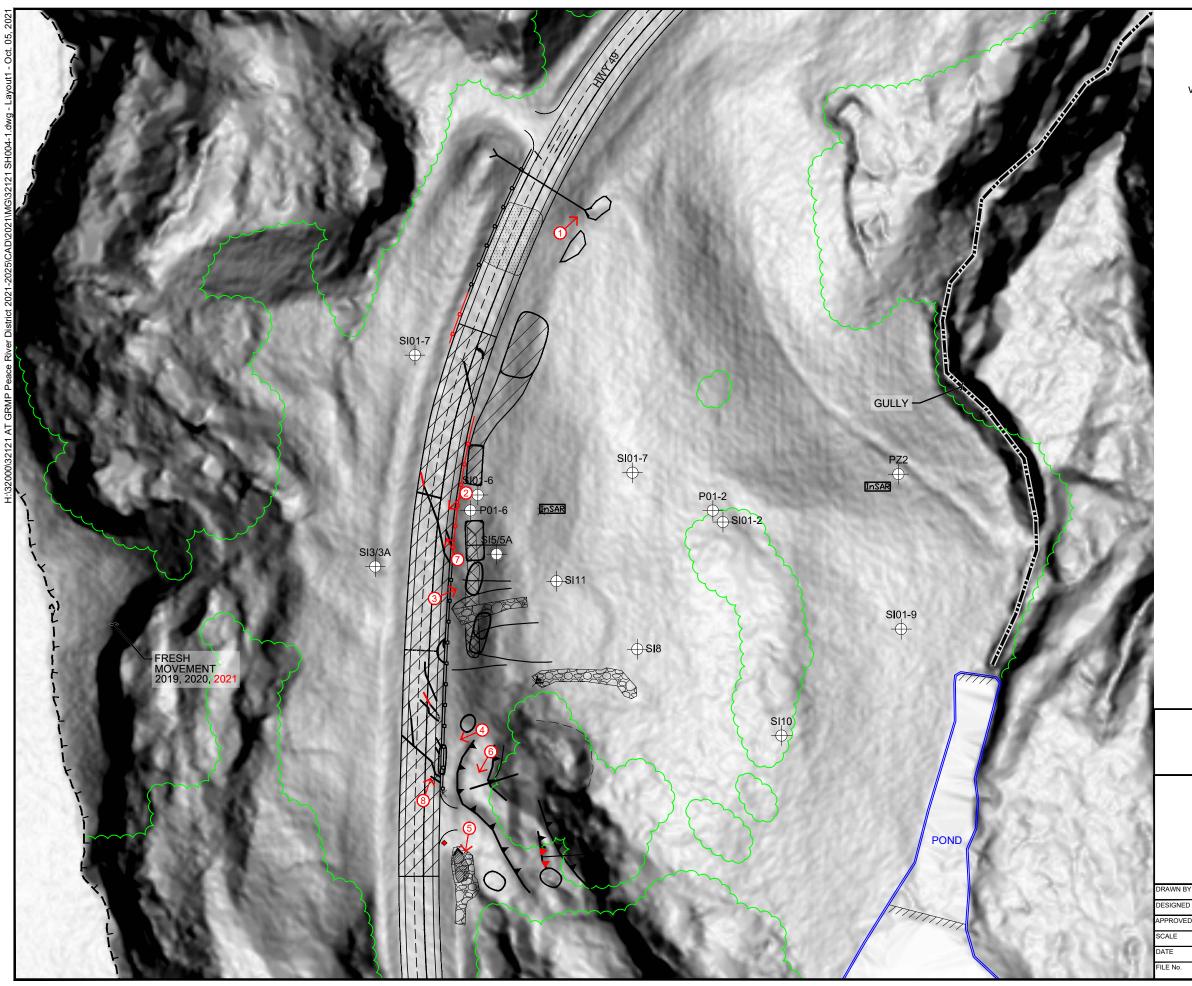
SH003-1, SH004-1, SH016-1 KEY MAP

FIGURE 1



DRA	WN BY	KLW
DES	GNED BY	MG
APPI	ROVED BY	DWP
SCA	.E	1:15000
DATI	Ē	OCTOBER 2021
FILE	No.	32121







LEGEND

APPROXIMATE INSTRUMENT LOCATION

SLOPE INCLINOMETER

PNEUMATIC PIEZOMETER

→ ▼ MAJOR SCARP

-T-T- VALLEY CREST

TREE LINE

----- GUARDRAIL

— — LANDSLIDE SCARP CRACKS



RIPRAP APRON

RED TEMPORARY HAZARD SIGN



DIRECTION AND NUMBER OF PHOTO

NOTES

- 1. FEATURE LOCATIONS ARE APPROXIMATE.
- PREVIOUS OBSERVATIONS SHOWN IN BLACK
 (2013-2015 FROM AMEC FIGURE 1, PROJECT EG10030, PROVIDED BY ALBERTA TRANSPORTATION).
- 3. JUNE 2021 OBSERVATIONS SHOWN IN RED.
- 4. 2018 AND 2019 MILLING EXPOSED CRACKS
 PREVIOUSLY COVERED BY PATCHING. MILLING
 MATERIAL SPREAD ON SIDESLOPES.

REFERENCE

1. 2008 LIDAR PROVIDED BY ALBERTA
TRANSPORTATION. SHADED BY SLOPE ANGLE
FROM WHITE AT 0° TO BLACK AT ≥30°.





Transportation

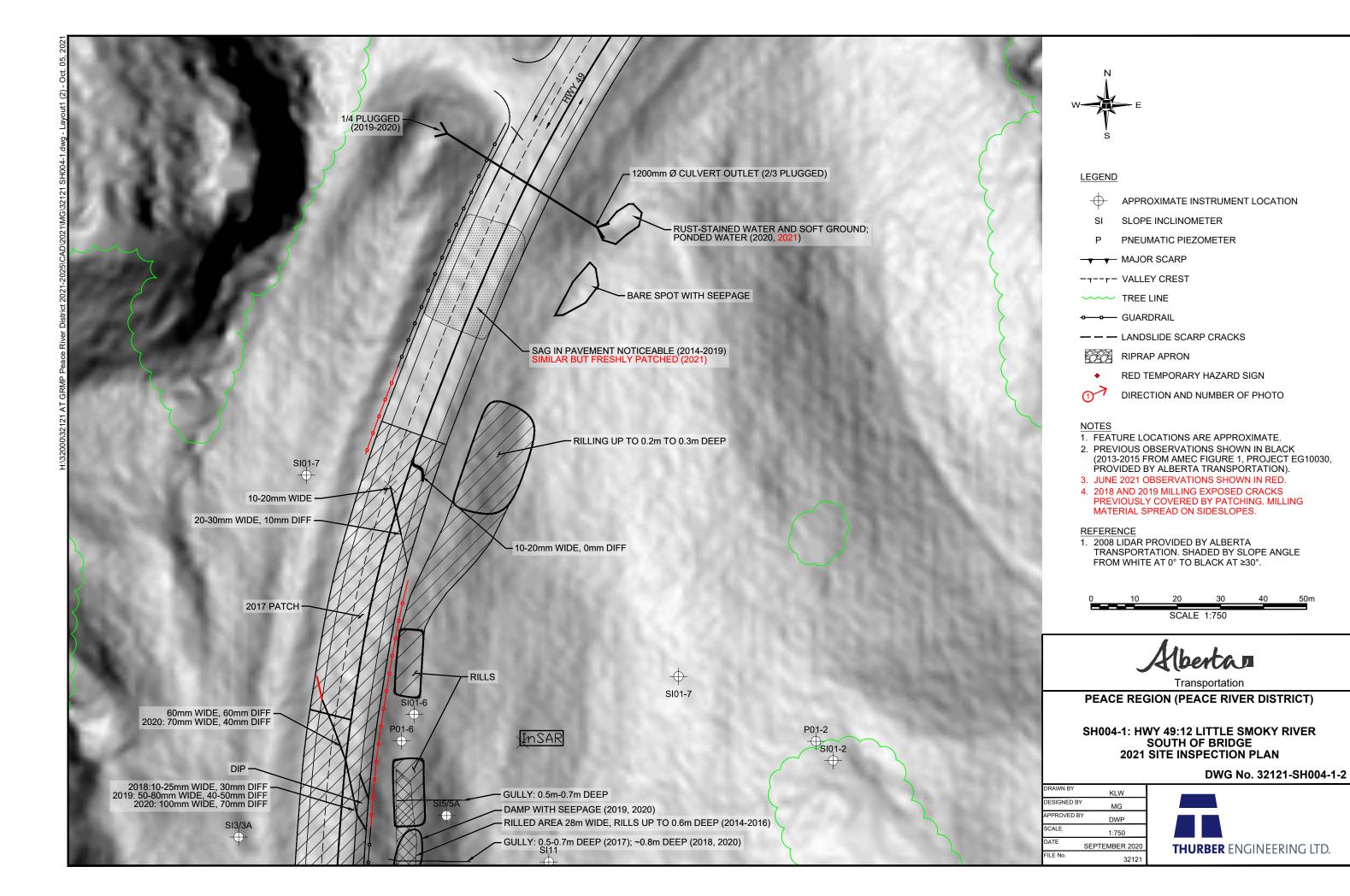
PEACE REGION (PEACE RIVER DISTRICT)

SH004-1: HWY 49:12 LITTLE SMOKY RIVER SOUTH OF BRIDGE **2021 SITE INSPECTION PLAN**

DWG No. 32121-SH004-1-1

DRAWN BY	KLW
DESIGNED E	MG MG
APPROVED I	BY DWP
SCALE	1:1500
DATE	SEPTEMBER 202
FILE No.	3212





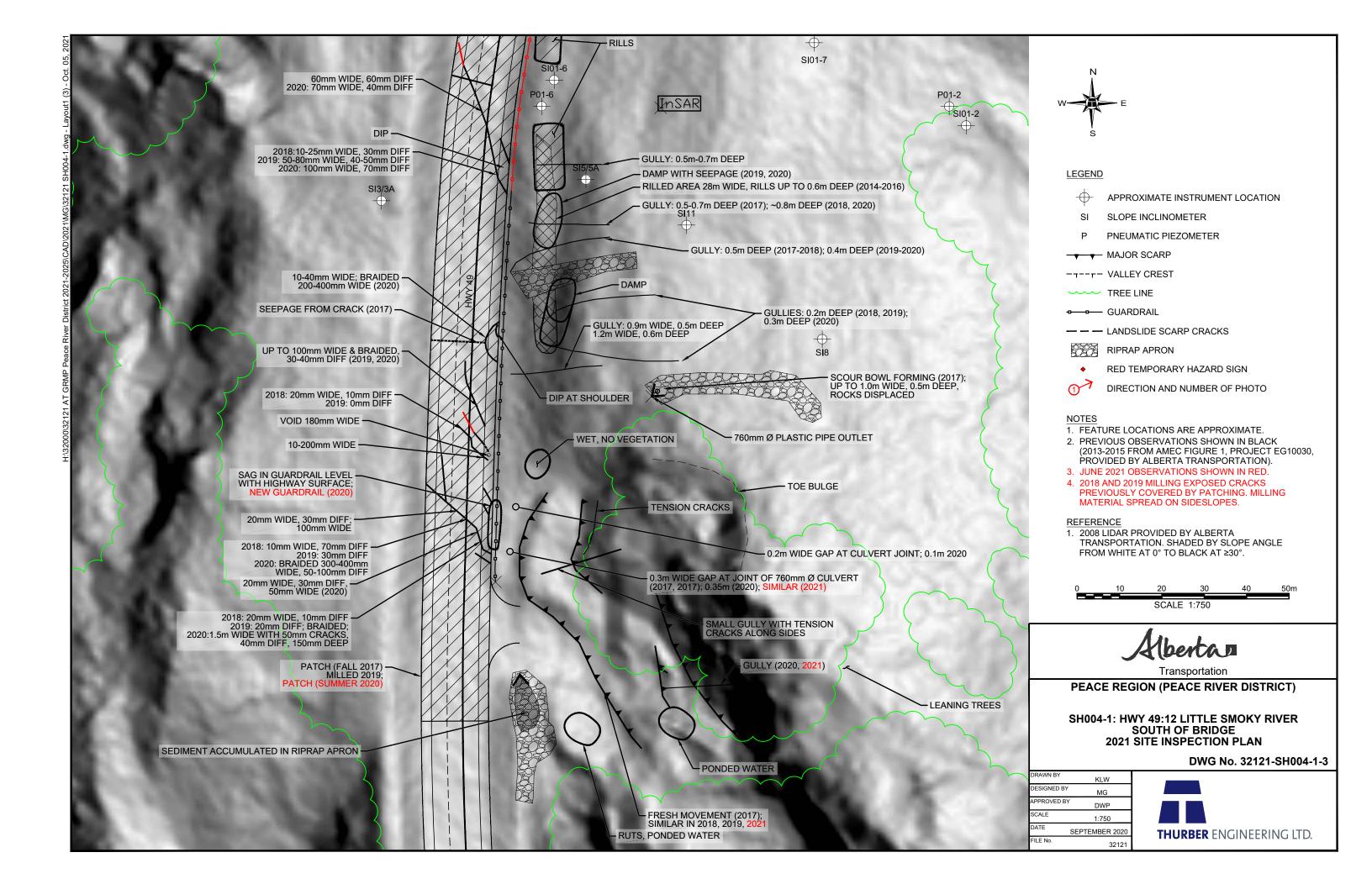






Photo 1 – Looking north at seepage on downslope side of highway at the north end of the site.





Photo 2 – Looking southwest at slide movement in the backslope above the highway.





Photo 3 – Looking northeast at rilling on either side of riprap apron.





Photo 4 – Looking southwest at guardrail at south end of site.





Photo 5 – Looking south along downslope ditch at sedimentation at inlet of 760 mm plastic culvert.

Client: Alberta Transportation File No.: 32121 Photo Date: June 28, 2021





Photo 6 – Looking south at 760 mm culvert joint separation.





Photo 7 – Looking north at crack adjacent to the north end of the downslope guardrail.





Photo 8 – Overlay and guardrail added in Fall 2020; cracking and differential becoming reestablished through the ACP overlay.