# **ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM** PEACE REGION (PEACE RIVER DISTRICT) **2022 INSPECTION**



Site Number	Location	Name	Hwy	km
SH004-1	South of Little Smoky River	Little Smoky River (South of Bridge)	49:12	2.3-2.6
Legal Description	1	UTM Co-ordinates		
W33-074-21-W5M		11U E 489,224	N 6,145,1	128

	Date	PF	CF	Total
Previous Inspection:	28-Jun-2021	13	6	78
Current Inspection:	31-May-2022	13	6	78
Road AADT:	1450		Year:	2022
Inspected By:	Rishi Adhikari, TRANS Ed Szmata, TRANS Max Shannon, TRANS		Ken Froese, Thurber Mark Gallego, Thurber	
Report Attachments:	<ul><li>✓ Photographs</li><li>✓ Plans</li><li>✓ Maintenance Items</li></ul>		ems	

Primary Site Issue:	Highway traverses deep-seated (about 35 m), retrogressive landslide with ongoing creep movement over the entire valley slope due partly to erosion at toe by the Little Smoky River.	
Dimensions:	300 m length of highway affected by several intersecting scarps resulting in uneven riding surface. Approx. 1.3 km of the highway crosses this unstable west valley slope.	
Date of Remediation:	1987: Subdrain installed in backslope ditch. 1998: Overlay including building up downslope shoulder with gravel fill buttress and raising of the guardrail. 2006: Repair of erosion damage and installation of surface drains to nearby sag pond.	
Maintenance:	Routine annual or bi-annual asphalt patching and milling 2016: Patch placed in August, milled in late September 2017: Patch and milled in August/September 2018: Milling removed up to about 60 mm of asphalt Fall 2019: Milling both sides of valley for about 172,000 m <sup>3</sup> 2020: Pavement overlay and guardrail replacement	

Observations:	Description	Worsened?
✓ Pavement Distress	Cracking and uneven roadway surface requires ongoing patching and milling especially at the landslide scarp cracks and the south end of the bridge.	>
✓ Slope Movement	Overall slope movement continues resulting in cracks at scarps and sags in the graben blocks.	>
✓ Erosion	Rilling between riprap channel and beyond P01-6. Lesser riling near north end of patch.	
✓ Seepage	Evidence of seepage observed near culvert outlet at north end of site.	
☑ Bridge/Culvert Distress	Separation of plastic culvert.	
□ Other		

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Instrumentation (Spring 2022):		
VW07-2, VW07-2A, VW07-3, VW07-3A	Relatively stable and slightly decreasing over the last two years.	
Destroyed/Lost	VW07-2B and -3B (damaged Fall 2014); SI4A, SI01-6, SI01-7, SI01-2, SI07-3B, SI01-9, SI07-2B, PZ01-6, PZ01-2 (discontinued)	

# **Assessment:**

The overall valley slope is moving as several separate slide blocks resulting in numerous scarps, sag ponds, and differential movement zones and the highway is intersected at several locations by these features resulting in an uneven highway surface. The driving mechanism appears to be toe erosion by the Little Smoky River although stability analyses undertaken by others indicate that a high ground water table may also be contributing. Based on GPS survey of the InSAR points conducted by Alberta Geological Survey (AGS Open Report 2013-14), this portion of the highway is located on landslide blocks moving relatively slower (5 mm to 40 mm per year) as compared to the north side (SH003) or the bridge site (SH016). Drawing 32121-SH004-1-1 shows some of the local slide features that have been interpreted from the 2008 LiDAR imagery. Regrading of the slopes at this site has obscured the scarps of the larger slide blocks that are likely present beneath the highway alignment.

The ongoing movement of the valley slope results in continued deformation of the highway surface that requires frequent patching. An overlay was placed in the summer of 2020 to maintain the smoothness of the highway. Reportedly, the crack pattern in the highway started reflecting through the overlay in less than 24 hours. As the crack pattern became fully established, the increasing differential across the cracks required additional milling in 2021 and 2022. As shown on the Drawings, the crack pattern is similar to previous years.

The rilling in the bare area on the downslope side of the highway to the north of the riprap apron appears to in a state similar to the previous inspection. These rill and gully areas should be repaired to prevent further degradation. There is a second area of rilling toward the north which is not quite as deep also appears to be in a state similar to the previous inspection.

There is a break in the plastic culvert that handles downslope ditch flow from the south that creating a sinkhole up to the ground surface. The sinkhole has not changed in size significantly in the last few years. A second gap at a culvert joint was observed in 2019 about 25 m further north.

The slope below approach at the south end of the site has started to deteriorate since about 2019. There is increasing displacement and extension of the tension and scarp cracks on this slope. The gully in the centre of the feature continues to deepen.

### Recommendations:

### **Short-term:**

- Road maintenance should continue as necessary to maintain an even, safe roadway surface and may consist of milling, patching, and crack sealing of the pavement. An asphalt overlay was placed through the site in Summer 2020, but rapid slide movements have already cracked and distorted it requiring additional milling.
- The bare areas on the downslope side of the highway to the north of the riprap apron have the potential to deteriorate further. This area should be regraded and covered with topsoil and seed secured with an erosion control blanket or seed-impregnated compost blanket. The second rilling area near the north end of the site could be repaired at the same time with the same methodology.
- The break in the culvert should be excavated and repaired to limit infiltration into the slope which could potentially result in slope movement or significant erosion.

# Long-Term:

The two alternatives for this location are: to realign the highway either using the existing bridge crossing or constructing a new one on more stable ground; or to control river erosion at the toe of the

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slope such that remedial measures above will have a longer effectiveness. It is understood that AMEC prepared a report under the High Water Related Mitigation Works program providing recommendations for erosion control at the toe and drainage measures on the slope for the sag ponds. In a previous study, Thurber identified a more-stable road alignment option going up a deep cut in the valley slope straight west off the end of the bridge.

# Ongoing Investigation:

- It is recommended that the annual Geohazard inspection and twice-annual instrumentation readings should continue as scheduled.
- Since there are no longer any operable SI's on site, consideration should be given to adding two to three slope inclinometers if there are other investigation projects in the area such that drilling costs could be shared so that the rate of creep can be monitored.
- Consideration should also be given to re-surveying the InSAR (interferometric synthetic aperture radar) targets, perhaps annually, to supplement the work done by the AGS as this will provide an overall view of ground movements.
- A GPS real-time ground movement system (Geocube), that is less expensive than the current systems, may be an option worth considering at this site particularly for identifying lower-movement rate zones for potential realignment.

# Closure

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Renato Clementino, Ph.D., P.Eng. Principal | Senior Geotechnical Engineer

Ken Froese, P.Eng. Associate | Senior Geotechnical Engineer

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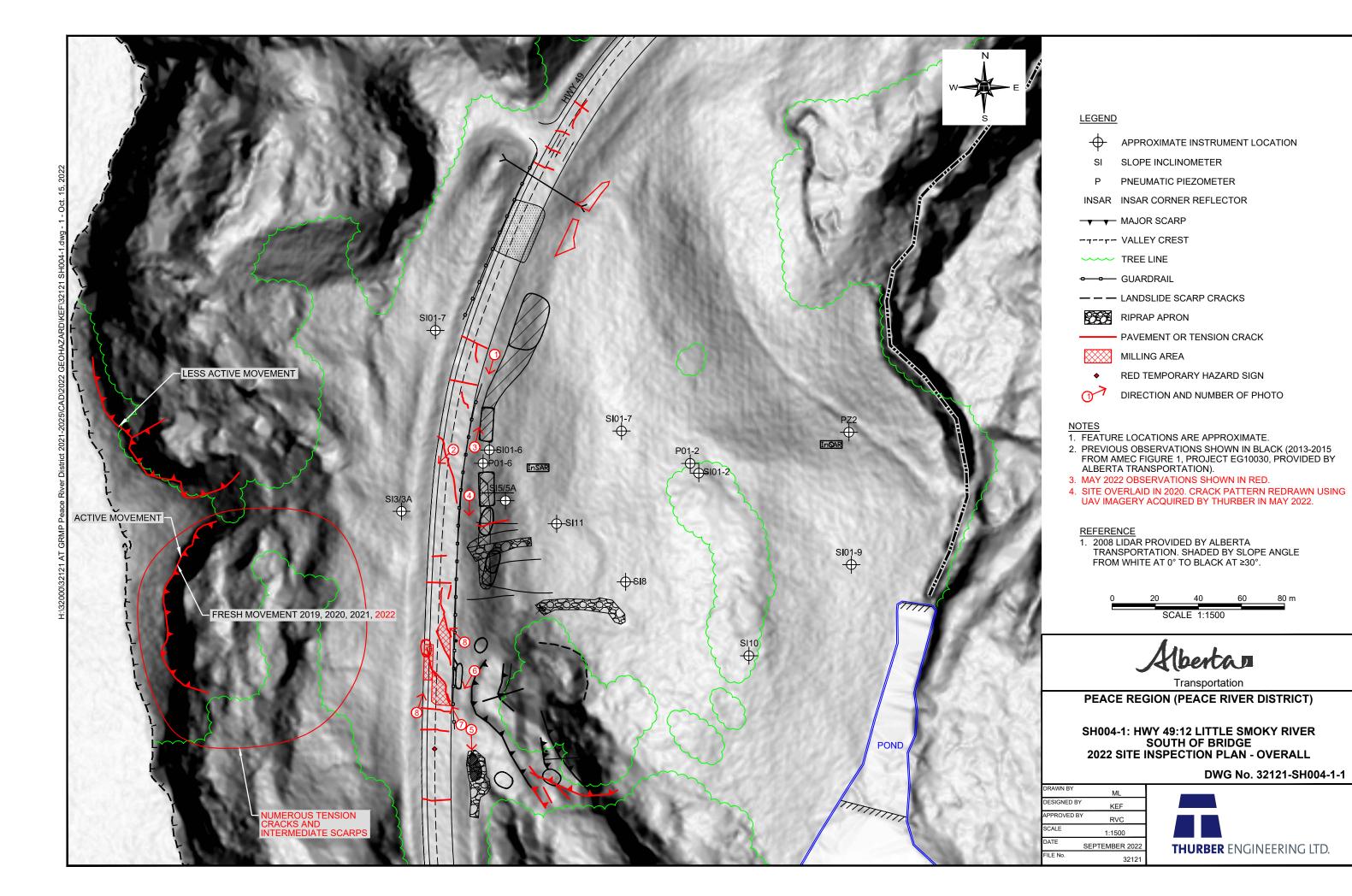
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- d) Construction Services: During construction Thurber should be retained to provide field reviews. Field reviews consist of performing sufficient and timely observations of encountered conditions in order to confirm and document that the site conditions do not materially differ from those interpreted conditions considered in the preparation of the report. Adequate field reviews are necessary for Thurber to provide letters of assurance, in accordance with the requirements of many regulatory authorities.

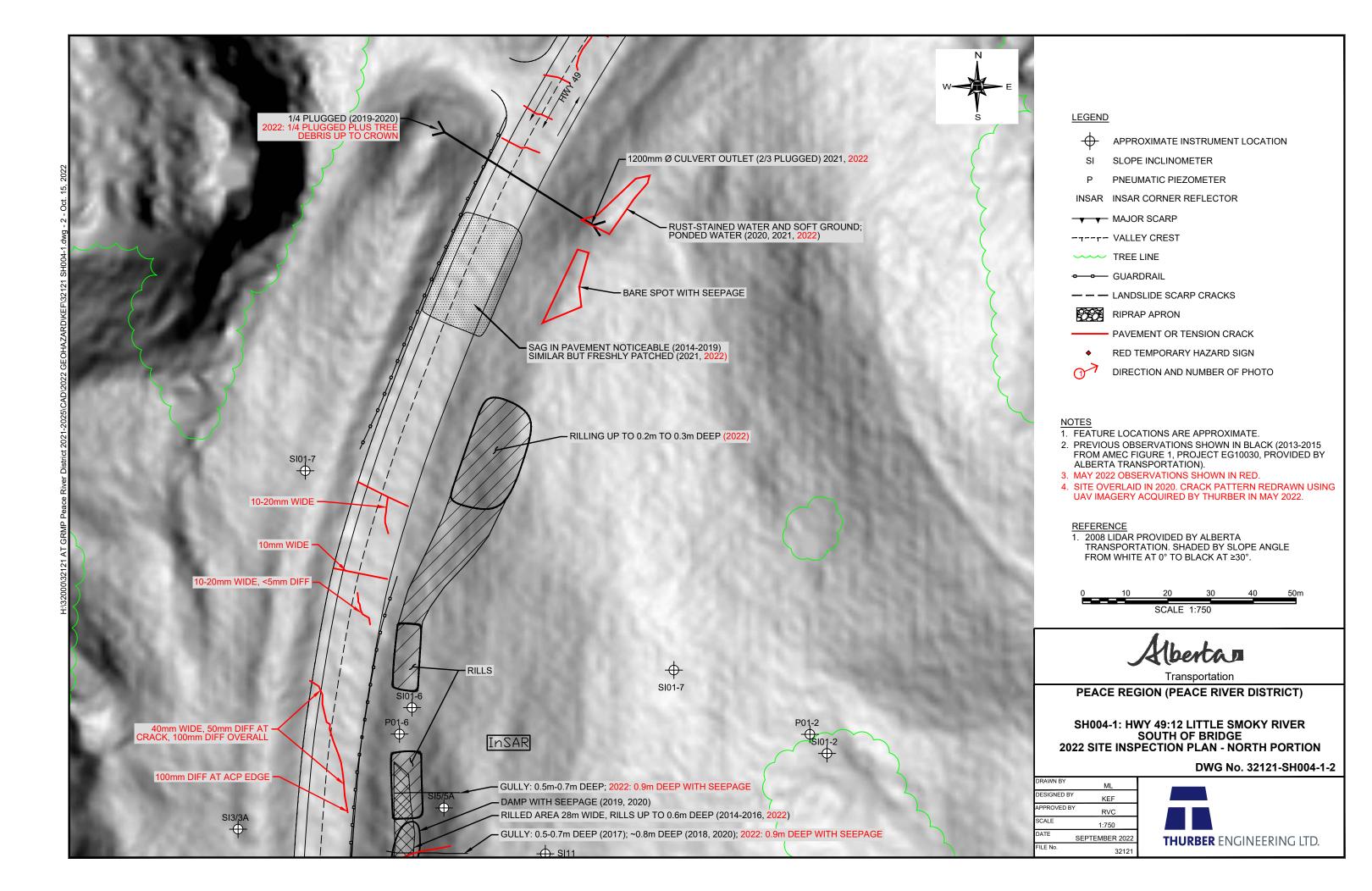
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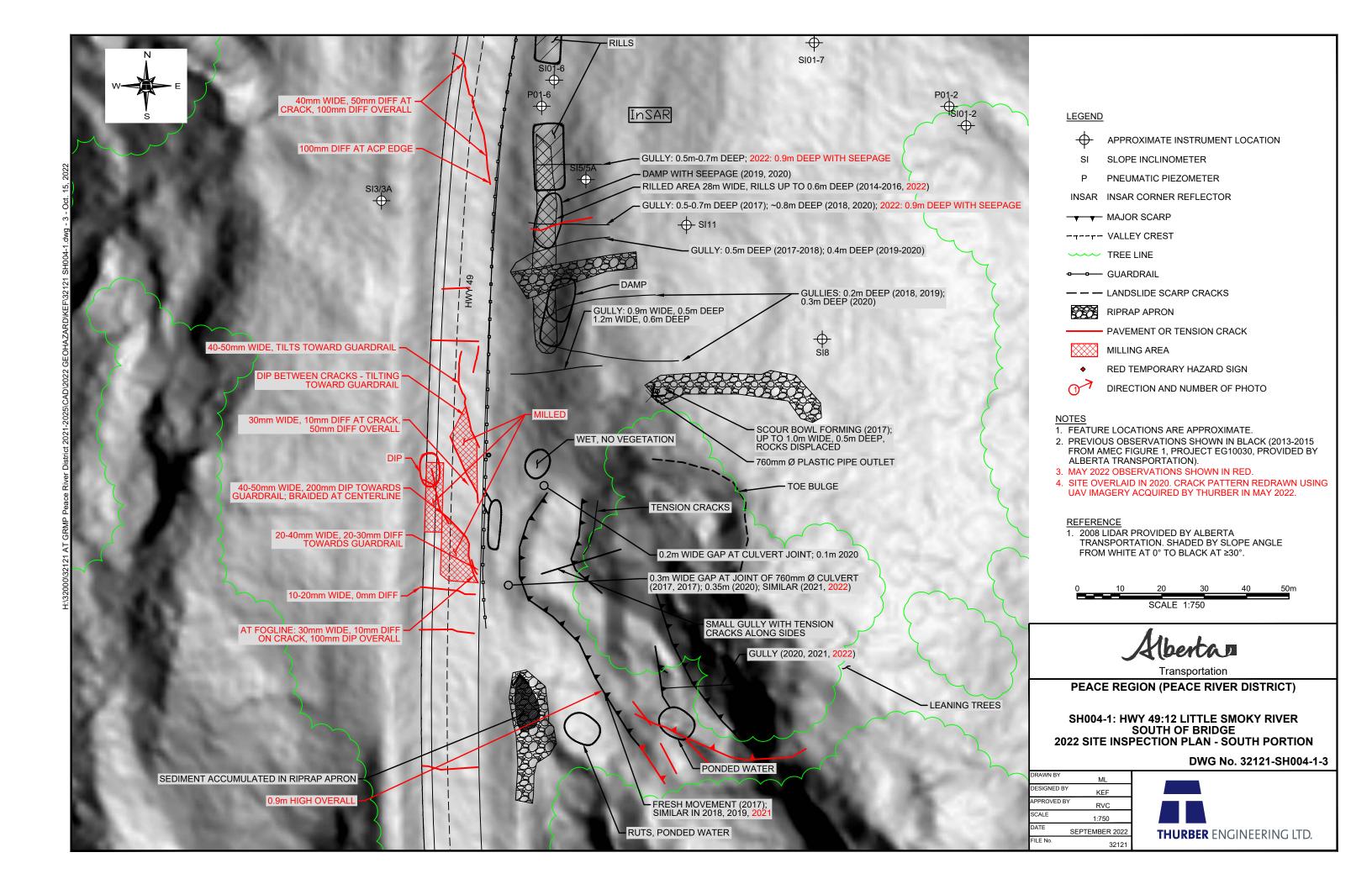
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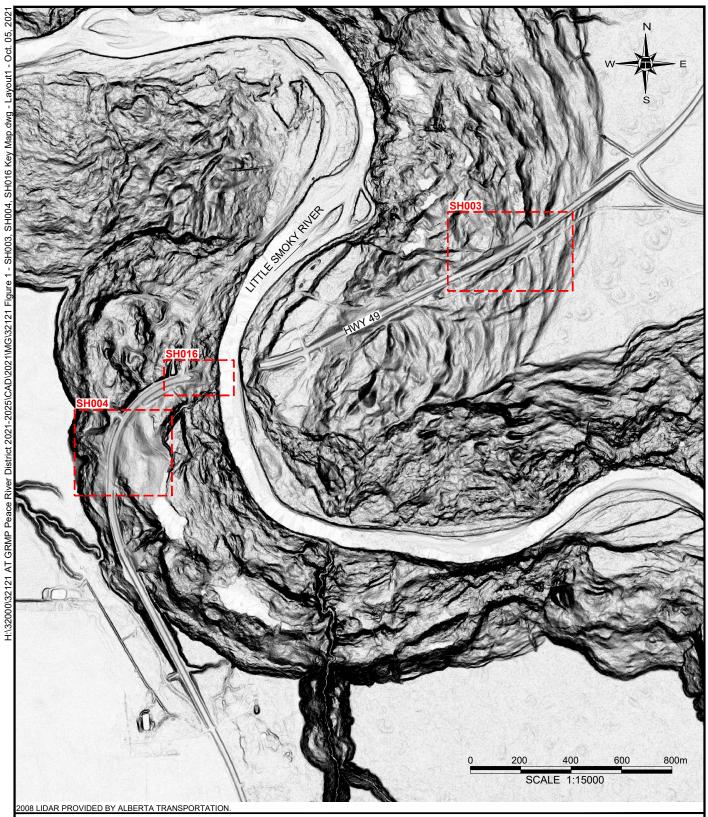
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PEACE REGION (PEACE RIVER DISTRICT)

SH003-1, SH004-1, SH016-1 KEY MAP

FIGURE 1



DRA	WN BY	KLW
DES	GNED BY	MG
APPI	ROVED BY	DWP
SCA	.E	1:15000
DATI	Ē	OCTOBER 2021
FILE	No.	32121





Photo 1 – Looking south at downslope side of highway at the north end of the site.



Photo 2 – Looking southwest slide movement in the backslope above the highway.

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Photo 3 – Looking northeast at rilling below the guardrail.



Photo 4 – Looking southwest at guardrail at south end of site. There was a significant sag at this location before the work in 2020.

Photo Date: May 31, 2022

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File No.:



Photo 5 – Looking south along downslope ditch at sedimentation at inlet of 760 mm plastic culvert.



Photo 6 – Looking at 760 mm culvert joint separation.

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Photo 7 – Looking north at crack just north of the downslope guardrail.



Photo 8 – Cracks across highway at south end of site.

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