ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – SWAN HILLS 2022 INSPECTION



Site Number	Location			Name		Hwy	km	
SH023-11 Little Smoky R		kv River		Little Smoky River Valley,		744:02	2 20.46-20.79	
				North Hill – Site #11		7 1 1.02	20.10 20.70	
Legal Description NE21-76-22-W5M		UTM Co-ordinates 11U E 478,317 N			N	6 162 199		
INE21-70-22-005101		11U E 478,317 N 6,162,188						
		Date	-	PF	CF		Total	
Previous Inspection:		2-Jun-2020		10	4		40	
Current Inspection: Road AADT:		<u>1-Jun-2022</u> 23		10	4 Year:		40 2021	
Inspected By:		Rishi Adhi						
			shi Adhikari, TRANS Ken Froese, Thurber ax Shannon, TRANS Mark Gallego, Thurber					
Report Attachments:		Photog	Photographs					
		0			— • • • •			
		Plans			Mainten	ance Ite	ems	
Primary Site Issue:		Site), partly result nume unsta	Highway traverses deep-seated (likely 35 m to 45 m deep at this Site), retrogressive landslides with ongoing creep movements due partly to erosion at toe by the Little Smoky River and Peavine Creek resulting in cracking and sagging of the pavement surface at numerous locations. Approx. 4 km of the highway crosses this unstable north valley slope. Site #11 is 60 m above and 260 m away from the Peavine Creek.					
Dimensions:		330 m	330 m length of highway affected by cracking and distortion					
Date of Remedia	20+60 2000: to Site and d	 1988: 6 m deep subdrain installed in upslope ditch from Sta. 20+600 to 20+860. 2000: Toe berm, gravel drainage blanket, and subdrain pipe (drains to Site #10) installed (by AGRA/AMEC). Patching of the highway and ditch cleaning done at the same time. 2005: West ditch lined with ECP and GeoRidge (20 m spacing). 						
Maintenance:		Routii 2017: remov Fall 2 2019: 2020:	Routine ACP crack sealing, milling, and patching, when required. 2017: ACP patch placed over south portion of Site #11. Guardrail removed and sideslopes regraded (1,200 m ³ of pitrun). Fall 2017: Milled and patched. 2019: Milling over most of the Site. 2020: Line painting 2021: Highway overlay (50 mm)					
Observations:		Description				Worsened?		
Pavement Distress		longit throug	Site was recently overlaid. Some of the previous longitudinal and traverse cracks have reflected through. Rutting was observed in a patch located in the outer wheel path of the SBL.				<u><</u>	
Slope Movement		Site lands There	Site is located on an active deep-seated landslide moving toward the Peavine Creek. There is significant vertical deformation of the pavement.					
Erosion		Upslope ditch was regraded and erosion control measures installed.						
Seepage								
Bridge/Culve	s							

C Other						
Instrumentation:						
Destroyed: (year lost)	SP99-3 (2006), SF Installed in 2000 b	<i>by AGRA:</i> SI99-1 (2000, sheared about 24m), 299-4 (2006), SP99-5 (unknown), SP99-6 (2005), <i>y AGRA:</i> SI00-1 (2002), SP00-1 <i>by Thurber:</i> S01-1 (2002, sheared at 5.5m), SP- S)				

Assessment:

The overall valley slope is moving as several separate slide blocks in response to the toe erosion and downcutting of two different rivers resulting in numerous scarps, sag ponds, and differential movement zones going in slightly different directions. The highway intersects the scarps of these blocks at several locations resulting in an uneven highway surface and cracking. There is approximately 55 m to 60 m elevation difference between the highway and the Peavine Creek located about 250 m to the southeast with two significant scarps identified from LiDAR at 110 m and 205 m from the highway.

Site #11 is located on an active scarp with significant vertical deformation observed to be affecting the highway. Two significant scarp cracks were identified crossing the highway surface and could also be traced in the adjacent ditches (although obscured by regrading done in 2019 and 2021). The ditches were regraded and removed previously observed erosion gullies. Erosion control measures including matting and GeoRidges were installed in a portion of the upslope ditch. Since the highway overlay in 2021, the two main scarp cracks have become re-established. The south of the two sets of scarp cracks has greater deformation since the observed cracks were wider and deeper. There was less deformation at the north scarp crack; however, there is a significant 4 m high scarp located downslope in the trees (identified from LiDAR topography).

Historically, there has also been shallow movement of the embankment which was remediated in 2000 with the construction of a toe berm and blanket drain. Without instrumentation, it is difficult to determine the present effectiveness of the toe berm; however, there did not appear to be signs of toe berm instability (such as cracking or bulging); however, the crack pattern in the highway above the berm continues to expand which is likely indicative of deeper-seated movement below the berm.

Recommendations:

Short-Term:

 Road maintenance should continue as necessary to maintain as safe roadway surface and may consist of milling, patching, and crack sealing of the ACP.

Long-Term:

- It is understood that, at this time, the only long-term remediation option under consideration is realignment of the entire north hill section of Highway 744. However, given the high cost of this option and as it is a low volume highway, it is unlikely that a full realignment will be undertaken in the near-future. Consideration is also being given to a shorter realignment which would include both of the SH023 sites as they currently require frequent maintenance.
- However, given the significant vertical distortion, vertical realignment of the highway at this Site #11 should be considered. Lowering of the highway grade, or subcut and replacement with light-weight fill, would reduce the driving weight at the top of this slide block and might decrease the rate of maintenance. Alternatively, a horizontal shift of at least 20 m into the slope could be considered to move the highway off this active slide block. The Maintenance Contractor Inspector estimates a \$2M cost to realign the highway around the current Sites #10 and #11.

Ongoing Investigation:

• It is recommended that the annual Geohazard inspection should continue as scheduled.

As this is one of the more-active Sites along this north valley slope, consideration should be given to installing two or three slope inclinometers to evaluate the ongoing performance of the toe berm and assessing current slope movement rates particularly if vertical or horizontal realignment is being considered.

Closure

It is a condition of this letter report that Thurber's performance of its professional services will be subject to the attached Statement of Limitations and Conditions.

Don Proudfoot, P.Eng. Principal | Senior Geotechnical Engineer

Mark Gallego, P.Eng. Geotechnical Engineer



STATEMENT OF LIMITATIONS AND CONDITIONS

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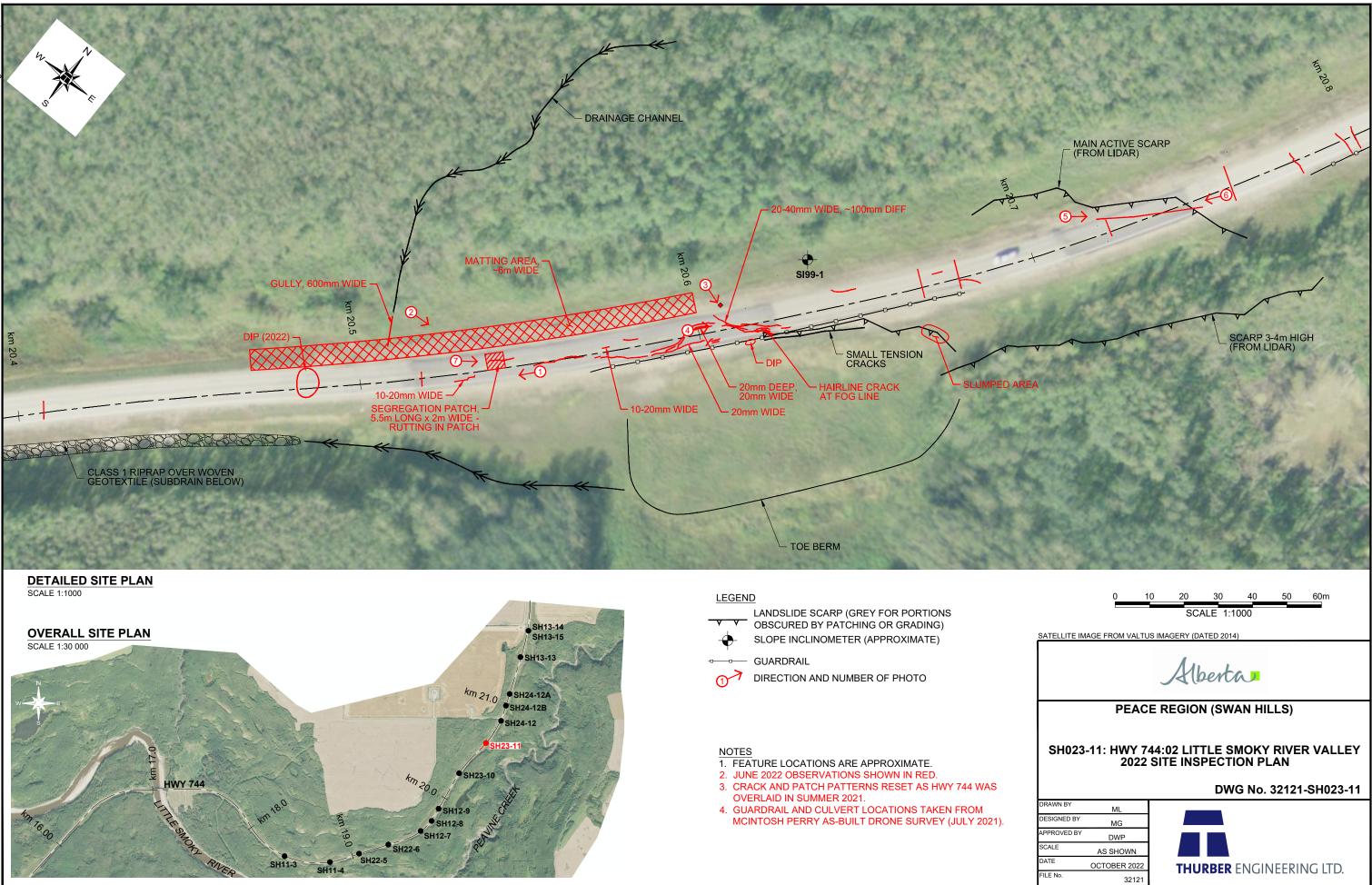
- a) Nature and Exactness of Soil and Contaminant Description: Classification and identification of soils, rocks, geological units, contaminant materials and quantities have been based on investigations performed in accordance with the standards set out in Paragraph 1. Classification and identification of these factors are judgmental in nature. Comprehensive sampling and testing programs implemented with the appropriate equipment by experienced personnel may fail to locate some conditions. All investigations utilizing the standards of Paragraph 1 will involve an inherent risk that some conditions will not be detected and all documents or records summarizing such investigations will be based on assumptions of what exists between the actual points sampled. Actual conditions may vary significantly between the points investigated and the Client and all other persons making use of such documents or records with our express written consent should be aware of this risk and the Report is delivered subject to the express condition that such risk is accepted by the Client and such other persons. Some conditions are subject to change over time and those making use of the Report should be aware of this possibility and understand that the Report only presents the conditions at the sampled points at the time of sampling. If special concerns exist, or the Client has special considerations or requirements, the Client should disclose them so that additional or special investigations may be undertaken which would not otherwise be within the scope of investigations made for the purposes of the Report.
- b) Reliance on Provided Information: The evaluation and conclusions contained in the Report have been prepared on the basis of conditions in evidence at the time of site inspections and on the basis of information provided to Thurber. Thurber has relied in good faith upon representations, information and instructions provided by the Client and others concerning the site. Accordingly, Thurber does not accept responsibility for any deficiency, misstatement or inaccuracy contained in the Report as a result of misstatements, omissions, misrepresentations, or fraudulent acts of the Client or other persons providing information relied on by Thurber. Thurber is entitled to rely on such representations, information and instructions and is not required to carry out investigations to determine the truth or accuracy of such representations, information and instructions.
- c) Design Services: The Report may form part of design and construction documents for information purposes even though it may have been issued prior to final design being completed. Thurber should be retained to review final design, project plans and related documents prior to construction to confirm that they are consistent with the intent of the Report. Any differences that may exist between the Report's recommendations and the final design detailed in the contract documents should be reported to Thurber immediately so that Thurber can address potential conflicts.
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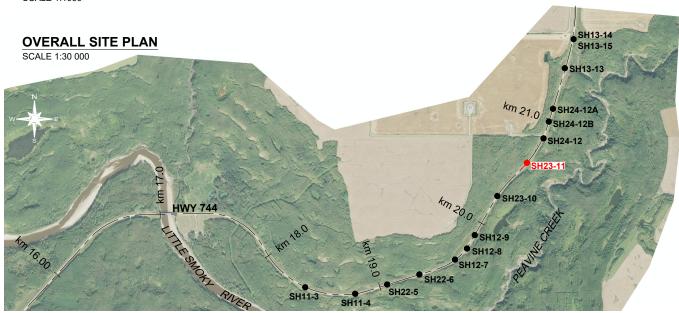
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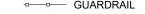














Photo 1 – Looking southwest at the southwest end of Site 11.



Photo 2 – Looking east highway at south end of site where previous significant gully was regraded and erosion control measures (matting and GeoRidges) were installed.





Photo 3 – Looking east at main scarp crack crossing highway (see Photo #4).



Photo 4: Looking northeast where main scarp crack crosses the highway at the central portion of Site 11.





Photo 5 – Looking northeast at north scarp crack.



Photo 6 – Looking southwest at north scarp crack.





Photo 7: Scarp crack and rutting in a patch located in the SBL.