# ALBERTA TRANSPORTATION GEOHAZARD ASSESSMENT PROGRAM PEACE REGION – SWAN HILLS DISTRICT 2020 CALL OUT INSPECTION



| Site Number   | Location |  |   | Name                           |       |       |        | Hwy                 | km     |  |
|---|----------|--|---|--------------------------------|-------|-------|--------|---------------------|--------|--|
| Call Out South of Hi  |          | ligh Prairie   |   | West Prairie River Oxbow Slide |       |       | 749:02 | 20.2                |        |  |
| Legal Description   |          | UTM Co-ordinates (NAD 83)                                      |   |                                |       |       |        |                     |        |  |
| NW36-73-17-W5   |          |  |   | 11U N 6,136,115 E 532,715      |       |       |        |                     |        |  |
|   |          |  | Date  |                                | PE CE |       |        | Total               |        |  |
| Previous Inspection:  |          | Dute   |   |                                |       |       |        |                     |        |  |
| Current Inspection:   |          | June 18, 2020  |   |                                | 13    | 4     | 52     |                     |        |  |
| Road AADT:  |          | 480  |   | )                              |       | Year: | 2019   |                     |        |  |
| Inspected By:   |          | Don Proudfoot, Nicole Wilder (Thurber)<br>Rodney Johnston (AT) |   |                                |       |       |        |                     |        |  |
| Report Attachments:   |          | •  | Photograph  | าร                             | Plans |       |        | □ Maintenance Items |        |  |
| Landslide with a 1.3 m high backscarp in embankment fill on west side |          |  |   |                                |       |       |        |                     |        |  |
| Primary Site Issue:   |          |  | of highway 26 cm from pavement edge.  |                                |       |       |        |                     |        |  |
| Dimensions:   |          |  | About ~35 m long by 25 m wide   |                                |       |       |        |                     |        |  |
| Date of any remediation:  |          |  |   |                                |       |       |        |                     |        |  |
| Maintenance:  |          |  |   |                                |       |       |        |                     | 1      |  |
| Observations:   |          |  | Description   |                                |       |       |        |                     | Worse? |  |
| Pavement Distress   |          |  | There are s appear to be  |                                |       |       |        |                     |        |  |
| Slope Movement  |          |  | The landslid<br>and is retro<br>appears to b  |                                |       |       |        |                     |        |  |
| Erosion   |          |  |   |                                |       |       |        |                     |        |  |
| ✓ Seepage   |          |  | Water was found ponded in several locations within the landslide mass within several medial cracks.   |                                |       |       |        |                     |        |  |
| Bridge/Culvert Distress   |          |  |   |                                |       |       |        |                     |        |  |
| Other Instrumentation: None   |          |  | The embankment slopes on either side of the landslide<br>are relatively steep; however, currently no signs of<br>cracks but the flanks of the landslide may retrogress<br>laterally over time. A Telus communications cable that<br>runs through the landslide broke on June 8 due to the<br>slide movements. |                                |       |       |        |                     |        |  |
| Instrumentation: None   |          |  |   |                                |       |       |        |                     |        |  |

# Assessment:

The slide is approximately 35 m long by 25 m wide and located in a 12 m high embankment fill side slope. The soils exposed in the slide scarp appeared to consist of brown high plastic clay. The slide mass has slid down to the oxbow at the toe of the slope.

There was no pavement distress observed during the call-out visit, but there was relatively fresh sloughing and moist soil within the slide mass, which was marked with many small secondary scarps and tears further downslope, along with free ponding of water throughout.

It is anticipated that the slide is a relatively shallow, retrogressive slide that was triggered by water seepage and weathering leading to loss of cohesion in the embankment fill. Weak high plastic clay embankment materials and a relatively steep slope may also have contributed to causing the slide. The main scarp of the active landslide is retrogressing further back towards the highway and was measured

to be 26 cm from the pavement during our inspection; however, on July 2, 2020 a photograph was taken by the MCI and the slide has now retrogressed into the highway pavement and is expected to continue which may require partial closure of the highway.

The backslope slumping appears to be shallow based and is not anticipated to be connected to a deepseated slide below the highway.

The pavement structure in the exposed edge of road was measured to be 230 mm of ACP over 200 mm of soil cement at this location.

# **Recommendations:**

#### Investigation:

Drill one test hole above the main scarp on the west side of the highway to a depth of about 20 m. The test hole should be completed with a piezometer. This would provide information on the subsurface soil and groundwater conditions for inputs to a slope stability assessment to assess potential slope stabilization design measures.

# Short Term:

In the short term, the slide should be regularly monitored for regression of the slide scarp. The southbound lane may need to be closed if retrogression continues and more signage/barricades placed.

### Medium to Long Term:

The recommended repair for this project is to sub-excavate the failed slide mass down to intact foundation soil and rebuild the slope with imported 6-80 gravel to a slightly flatter 3H:1V inclination. The excavation for the gravel wedge would likely need to extend to about the centreline of the highway, however this should be confirmed by the slope stability assessment. The new fill material should be placed and compacted in thin horizontal lifts, benched into the intact slope surface, utilizing a gravel shear key (if required) to stabilize the slide area. Some of the more suitable excavated material could be used to provide a covering layer overtop the gravel as the finished slope surface to shed runoff, with any excess removed from site. A subdrain should be installed along the base of the slide excavation to drain any subsurface water that may enter the new fill zone.

An alternate option would be to construct a toe berm within the oxbow/marsh area, flatten the slope and rebuild the slope with granular fill. However, this may not be a feasible option if the area is considered a wetland.

Ballpark Cost ~\$0.5 Million





**LEGEND** 





























Looking south at landslide scarp that has retrogressed further cutting into pavement taken after call out